





# **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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**BLOEMFONTEIN**

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## DECLARATION

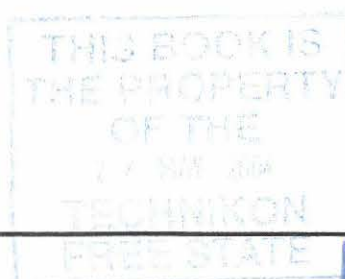
I, OCKERT, JACOBUS GERICKE, hereby declare that this research project submitted for the degree MAGISTER TECHNOLOGIAE: ENGINEERING: CIVIL, is my own independent work that has not been submitted before to any institution by me or anyone else as part of any qualification.



SIGNATURE OF STUDENT

2003-04-11

DATE





*This study is dedicated to my late father, George Sebastiaan Gericke, whose support, love and faith in me will remain me always, as a lasting legacy.*



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## EXPRESSION OF THANKS

I wish to express my gratitude to:

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## SUMMARY

Hydrological modelling, simulating surface runoff in a catchment, was a relatively new approach in South Africa in the mid seventies. The advent of hydrological modelling on a practical level in South Africa lagged that of other countries by several years at that stage. This delay could be ascribed to the fact that the awareness concerning surface water, draining concepts and hydrological modelling was only then becoming more prevalent in South Africa.

Today, following international trends, there is a growing number of hydrological modelling systems for integrated water resource management on a catchment basis in South Africa. Such systems are likely to be installed for operational use in ongoing learning, research, strategic planning and consensus building amongst role players in the Catchment Management Agencies (CMAs). These installed systems are poised to fundamentally change the way modelling is approached in South Africa. Hydrological models are the logical and irreversible response to the enormous forces, which have led to the revision of the National Water Act (NWA), 1998 (Act 36 of 1998) and the water resource management paradigms.

The *primary objective* of this modelling process is the application of the streamflow generation component of the Hydrological Simulation Program – Fortran (HSPF) model in order to provide hydrological information essential to those responsible for planning, development and management of the Msunduzi River Catchment. The *focus* of this modelling process is on the development and implementation of all input data and the testing of HSPF's continuous modelling system to correctly represent the hydrological components of the hydrological cycle.

The Msunduzi River Catchment forms an integral part of the southern portion of the Mgeni River Catchment. By 1985, the Mgeni River Catchment was already supplying water to 3.6 million people, industry and agriculture, which contribute 20% of South Africa's Gross National Product.

The margin of error in the simulated annual water balance varied between two and 11% during calibration, while it varied between five and 16% during the period of verification. There is a slight bias in the seasonal calibration for under-estimating the streamflow in the upper sub-catchments during the wet period and over-estimating the baseflow during autumn and follow-on dry periods. In the lower sub-catchments, the wet periods were over-simulated and the baseflow was correctly simulated during the dry period. The response time and interflow recession rates of the hydrographs were accurately simulated, except for some cases during autumn where the interflow recession rate was incorrectly simulated.



The over-simulation of the various single storm events (higher streamflow peaks) was the most consistent error that occurred during the period of calibration. This can probably be ascribed to the poor representation and areal distribution of precipitation data, which did not account accurately for the spatial variation in precipitation- and storm distributions.

Hydrological simulations are a prerequisite for further expansion of the model for water quality simulations. Therefore an accurate hydrological simulation forms an essential first step in developing a full HSPF application for a catchment. The fact that the Msunduzi River Catchment constitutes a substantial portion of the Mgeni River Catchment emphasises the importance of this project's results to help manage the hydrology and water quality of the larger Mgeni River Catchment of approximately 4000 km<sup>2</sup>. HSPF can also be used to develop model parameters to test scenarios of future development within the catchment such as land-use changes, which may affect runoff and streamflow.

HSPF, together with other installed modelling systems, will enable CMAs to address the contradictory calls for affordable modelling at the same time as recognising greatly increased complexity, scalability, accessibility and integration. The successful implementation of the HSPF model will address the timeliness issues of today's fast moving world in which contentious water resource issues are becoming much more commonplace.



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## OPSOMMING

Hidrologiese modelering om oppervlakafloop te simuleer was in die sewentigerjare 'n betreklike nuwe benadering in Suid-Afrika. Suid-Afrika het toe 'n agterstand van 'n paar jaar beleef waar dit by die praktiese toepassing en implementering van hidrologiese modelering gekom het. Hierdie vertraging kan toegeskryf word aan die feit dat bewustheid rakende die beginsels rondom oppervlakwater, dreinerings en hidrologiese modelering toe eers posgevat het in Suid-Afrika.

Volgens internasionale neigings, vandag, word hidrologiese modeleringsisteme vir geïntegreerde waterhulpbronbestuur toenemend gebruik in opvangsgebiede in Suid-Afrika. Sulke sisteme sal waarskynlik gevestig word vir operasionele gebruik in voortgesette onderrig, navorsing, strategiese beplanning en om eenstemmigheid tussen die belangrikste rolspelers van Opvanggebiedbestuursowerhede te bevorder. Die reeds geïmplementeerde sisteme is slag gereed om die huidige modeleringstegnieke in Suid-Afrika, wesenlik te verander. Hidrologiese modelle is die logiese en onomkeerbare gevolg van die oortuigingskrag wat die hersiening van die Nasionale Water Wet, 1998 (Wet 36 van 1998) en klemverskuiwing rondom waterhulpbronbestuur paradigmas, tot gevolg gehad het.

Die *primêre doelwit* van hierdie modeleringspoging is die toepassing van die hidrologiese komponent van die "Hydrological Simulation Program- Fortran (HSPF)" model om sodoende 'n sisteem daar te stel wat noodsaaklike hidrologiese inligting aan al die rolspelers van die Msunduzi Opvangsgebied betreffende beplanning, ontwikkeling en bestuur beskikbaar te stel. Die *fokus* van hierdie modeleringsproses is op die ontwikkeling en implementering van al die invoerdata en die toetsing van HSPF se kontinue modeleringsisteem om al die hidrologiese komponente van die hidrologiese siklus korrek weer te gee.

Die Msunduzi Opvangsgebied vorm 'n integrale komponent in die suidelike gedeelte van die Mgeni Opvangsgebied. In 1985 het die Mgeni Opvangsgebied alreeds water aan 3.6 miljoen mense, industrieë en die landbou, wat 20% bydra tot Suid Afrika se Bruto Nasionale Produk, voorsien.

Die foutgrens in die gesimuleerde jaarlikse waterbalans het gevarieer tussen twee en 11% tydens kalibrasie, terwyl dit tussen vyf en 16% gevarieer het tydens verifikasie. Die model se seisoenale kalibrasie van die hoërliggende subopvangsgebiede is soms geneig om die piek stroomvloei tydens die nat periode te laag te simuleer en die basisvloei tydens herfs en die opeenvolgende droë periodes te hoog te simuleer. In die laerliggende subopvangsgebiede is die piek stroomvloei te hoog gesimuleer tydens die nat periode, terwyl die basisvloei in die droë periode korrek gesimuleer was. Die reaksietyd en tempo van intervloei van die hidrograwe is akkuraat gesimuleer; behalwe in sommige gevalle in die herfsmaande waar die intervloei-tempo verkeerd gesimuleer was.

Die oor-simulering van die verskeie eenmalige stormgebeurtenisse (hoër stroomvloei pieke) was die mees algemene fout tydens kalibrasieperiode. Dit kan moontlik toegeskryf word aan die swak verteenwoordigende oppervlakverspreiding van reënvaldata wat nie die onreëlmatige verandering en verspreiding van die reënval en donderstorms akkuraat kon verklaar nie.

Hidrologiese simulasies is 'n voorvereiste indien die model verder ontwikkel gaan word om waterkwaliteit te simuleer. Derhalwe vorm akkurate hidrologiese modelering die eerste noodsaaklike stap in die ontwikkeling van 'n volledige HSPF-toepassing in 'n opvangsgebied. Die feit dat die Msunduzi Opvangsgebied 'n groot gedeelte van die groter Mgeni Opvangsgebied uitmaak, beklemtoon die belangrikheid dat die resultate van hierdie projek sal bydra tot die effektiewe bestuur van die hidrologie en waterkwaliteit van die groter Mgeni Opvangsgebied van bykans 4000 km<sup>2</sup>. HSPF kan ook gebruik word om model-parameters wat die impak van toekomstige ontwikkelinge soos veranderinge in grondbenutting wat moontlik afloop en riviervloei sal beïnvloed, te ontwikkel.

HSPF, tesame met ander geïmplementeerde modeleringsisteme sal Opvangsgebiedbestuursowerhede in staat stel om die uiteenlopende behoefte vir bekostigbare modelering, sowel as die omvang, toeganklikheid en integreerbaarheid daarvan, te beseef en aan te spreek. Die suksesvolle implementering van die HSPF model sal alle toepaslike aspekte van belang in vandag se vinnige samelewing, waarvan kontroversiële waterhulpbronaangeleenthede so deel geword het, aanspreek.



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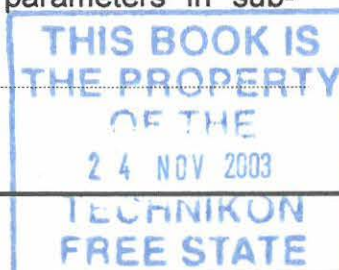
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## LIST OF ABBREVIATIONS

ACRU	Agricultural Catchment Research Unit
AGWETP	Evapotranspiration from Active Groundwater
AGWRC	Base Groundwater Recession Rate (/day)
AGWS	Active Groundwater Storage (mm)
ANNIE	Interactive Hydrological Analyses and Data Management
ARM	Agricultural Runoff Management Model
ARS	Agriculture Research Service
ASAE	American Society of Agricultural Engineers
BASINS	Better Assessment Integrating Point and Non-point Sources
BASETP	Evapotranspiration from Baseflow
BOD	Biochemical Oxygen Demand
CBD	Central Business District
CCWR	Computing Centre for Water Research
CEPS	Interception Storage (mm)
CEPSC	Interception Storage Capacity (mm)
CMA	Catchment Management Agencies
CREAMS	Chemicals, Runoff and Erosion from Agricultural Management Systems
CRRAT	Ratio of maximum to mean velocity
CSIR	Council for Scientific and Industrial Research
DEEPR	Fraction of infiltrating water, which is lost to deep aquifers
DELTH	Stream reach length change in elevation (m)
DO	Dissolved Oxygen
DSS	Data Storage System

---

EIA	Effective Impervious Area
EPA	Environmental Protection Agency
EPIC	Erosion Productivity Impact Calculator
ET	Evapotranspiration
FTABLEs	Function Tables to document the functional relationship between two or more variables (volume-discharge relationship for RCHRES operations)
GENSCN	Generalised Scenario Generator Software
GIS	Geographical Information System
GLEAMS	Groundwater Loading Effects on Agricultural Management Systems
GWVS	Initial Index to Groundwater Slope
HSP	Hydrological Simulation Program
HSPF	Hydrological Simulation Program – Fortran
HYDR	Flood routing, reservoir behaviour and analysis of dissolved constituents
IFWS	Initial Interflow Storage
IGWI	Inactive Groundwater
ILLUDAS	Illinois Urban Drainage Area Simulator
IMPLND	Impervious Land Segment
INFEXP	Exponent in Infiltration Equation
INFILD	Ratio of maximum or mean Infiltration Capacities
INFILT	Index of Mean Soil Infiltration Rate (mm/h)
INFLO	Inflow of water and constituents into RCHRES through a single gate
INTFW	Interflow Inflow Parameter
IOWDM	Input and Output for Watershed Data Management File

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IRC	Interflow Recession Constant
IVOL	Volume of inflow
IWATER	Water Budget for impervious land segments
KINE 2	Two-dimensional Kinematic Model
KVARY	Non-linear groundwater recession rate (/mm)
LEN	Stream reach length (km)
LSUR	Length of assumed overland flow plane (m)
LZETP	Lower Zone Evapotranspiration Parameter
LZS	Lower Zone Storage (mm)
LZSN	Lower Zone Nominal Storage (mm)
MAP	Mean Annual Precipitation (mm)
MUTSIN	Multiple Time Series Sequential Input
NCH	Manning's $n$ value for river channels
NFP	Manning's $n$ value for flood planes
NPS	Non-point Source Runoff Model
NSUR	Manning's $n$ for overland flow plane
NWA	National Water Act
NWRS	National Water Resource Strategy
OFLO	Outflow of water and constituents from RCHRES through multiple gates
PERLND	Pervious Land Segment
PEST	Parameter Estimation Program (non-linear)
PET	Potential Evapotranspiration
PETMAX	Temperature below, when evapotranspiration reduces to half (°C)
PETMIN	Temperature below where evapotranspiration is set to zero (°C)
PLS	Pervious Land Segment

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PRSUPY	Precipitation on the RCHRES surface
PWATER	Water Budget of pervious land segments
PQUAL	Water Quality Constituents of pervious land segment
QUAL2E	Enhanced Stream Water Quality Model
RCHRES	Runoff Simulator in a Single Reach
RETS	Retention Storage (mm)
RETSC	Retention Storage Capacity (mm)
RMSE	Root-mean-square-error
ROUTE	Hydraulic Routing
ROVOL	Volume of total outflow
RUN	Interpreter or group of sub-programs, which read and interprets the User's Control Input (UCI)
SAWB	South African Weather Bureau
SEDMNT	Land Surface Erosion
SLSUR	Average slope of assumed overland flow plane (m/m)
SNOW	Snow accumulation and melt
SURS	Surface Detention Storage
SWAT	Soil and Water Assessment Tool
SWMM	Storm Water Management Model
SWRRB	Simulator for Water Resources in Rural Basins
TDS	Total Dissolved Solids
TEAMS	Time Series Extraction and Manipulation
UCI	User's Control Input
URBCELL	Cell Model applicable to Urbanised Areas
USDA	United States Department of Agriculture

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USA	United States of America
USEPA	United States Environmental Protection Agency
USGS	United States Geological Survey
UZS	Upper Zone Storage (mm)
UZSN	Upper Zone Nominal Storage (mm)
VOL	Volume at the end of the interval or Initial stream channel water volume (M.m <sup>3</sup> )
VOLEV	Evaporation
WDM	Watershed Data Management
WITSKM	Witwatersrand Stormwater Kinematic Model
WITWAT	Witwatersrand Wave Approximation Theory Model
XSECT	Utility or program used in the conversion of river geometry data to volume-discharge relationships in FTABLEs

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# INTRODUCTION

## HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT



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# 1. INTRODUCTION

## 1.1 BACKGROUND

The Msunduzi River Catchment forms the southern portion of the Mgeni River Catchment. By 1985, the Mgeni River Catchment was already supplying water to 3.6 million people, industry and agriculture, which contribute 20% of South Africa's Gross National Product. In Water Plan 2025 (Horne Glasson Partners, 1989) it was predicted that, depending on future growth, the population in the whole area presently supplied by Umgeni Water could increase to between nine and twelve million by the year 2025 (Tarboton & Schulze, 1992: 1).

Concomitant with the population increase, anticipated future rural-, urban- and industrial development would increase the water demand in excess of available water resources, making effective water resource management within the Msunduzi River Catchment vital. This increase in water demand is likely to be accompanied by a decrease in water quality (Tarboton & Schulze, 1992: 1-2).

The above-mentioned factors that influence the hydrological response of a catchment as well as the different hydrological processes that occur are taken into consideration during hydrological modelling of the Msunduzi River Catchment. This specific hydrological modelling study is important, because it can be used to develop model parameters to test scenarios of future development within the catchment such as land-use changes, which may affect runoff and streamflow.

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surface runoff, infiltration rates and subsurface pathways, mechanisms for water storage in the catchment and evapotranspiration (ET) rates.

- The model should be able to investigate a wide variety of conditions in the river, including the extremes of floods and low-flow periods.
- Realistic and objective simulation of daily streamflow in the Msunduzi River Catchment, over the past eight years from 1992 to 2000.
- The development of a Geographical Information System (GIS) containing the relevant spatial information for the Msunduzi River Catchment.

### **1.3 TYPICAL MODELS USED IN SOUTH AFRICA**

Hydrological modelling simulating surface runoff in a catchment was a relatively new approach in South Africa in the mid seventies (Green & Stephenson, 1986: ii). The advent of hydrological modelling on a practical level in South Africa lagged that of other countries by several years at that stage. This delay could be ascribed to the fact that the awareness concerning surface water, draining concepts and hydrological modelling was only then becoming more prevalent in South Africa.

During the last two decades several models and their applications became more prevalent. Most of the models were developed in the early seventies and their application expanded during the last two decades to a level where they were verified against observed data. Although, due to the above-mentioned slow adoption, the demand is still limited to a few models and therefore the model availability is also restricted locally. Some of the most prominent models in use in South Africa together with their relative merits and applications are given in

Consulting engineers, water resource planners and hydrologists would use more complex hydrological simulation techniques if the appropriate technology has already been applied (Green & Stephenson, 1986: ii). These more complex computational techniques are used to determine viable surface water management policies, especially if the higher levels of urbanisation and population densities are taken into consideration (Coleman & Stephenson, 1990: 1).

## **1.4 HSPF: HISTORY AND PREVIOUS APPLICATIONS**

### **1.4.1 History**

The hydrological component of the HSPF model was developed in the early 1960's as the Stanford Watershed Model. In the 1970's, water-quality processes were added (USGS, 2002). A Fortran version incorporating several related models using software engineering design concepts was then developed. The Research Laboratories of the United States Environmental Protection Agency (USEPA) in Athens, Georgia funded these developments in the late 1970's.

In the 1980's, pre-processing and post-processing software, algorithm enhancements and use of the United States Geological Survey Watershed Data Management (USGS WDM) system were developed by the United States Geological Survey (USGS) and USEPA. An interactive version was developed by the USGS in the 1990's (USGS, 2002).



HSPF is currently in its 12<sup>th</sup> version (2001) and is widely used by consultants and government agencies in the United States of America (USA) and elsewhere, most notably Australia. The model is currently maintained by Aqua Terra Consultants and is supported by USGS and EPA. HSPF is an extension and improvement of three previously developed models:

- The EPA Agricultural Runoff Management Model (ARM) (Donigian & Davis, 1978);
- The EPA Non-point Source Runoff Model (NPS) (Donigian & Crawford, 1979) and;
- The Hydrological Simulation Program (HSP, including HSP Quality), a privately developed proprietary program (Aqua Terra Consultants, 2002).

The continuous simulation approach contained in these models is valuable in solving many complex water resource problems.

#### 1.4.2 Previous Applications

According to Russo (Bicknell, Imhoff, Kittle & Donigian, 1996) HSPF is currently the most comprehensive and flexible model of catchment hydrology and water quality available. This model can simulate the continuous, dynamic event or steady-state behaviour of either hydrological or hydraulic and water quality processes in a catchment (Bicknell *et al.*, 1996: 2). The potential applications and uses of the model are comparatively large, including:

- Flood control planning and operations;
- Hydropower studies;

- 
- ❑ Catchment planning;
  - ❑ Storm drainage analyses;
  - ❑ Water quality planning and management;
  - ❑ Point and non-point source pollution analyses;
  - ❑ Soil erosion and sediment transport studies;
  - ❑ Evaluation of urban and agricultural best management practices;
  - ❑ Fate, transport, exposure assessment, control of pesticides, nutrients and toxic substances and;
  - ❑ Time-series data storage, analysis and display (Bicknell *et al.*, 1996).

HSPF is designed so that it can be applied to most catchments using existing meteorological- and hydrological data, soil- and topographical information, land-use, drainage-, physical- and man-made characteristics. Long, rather than short time-series records are preferred. Typical long time-series records include precipitation, waste discharges and calibration data such as streamflow and constituent concentration (Bicknell *et al.*, 1996).

There have been hundreds of applications of HSPF all over the world. The applications range from the 160580 km<sup>2</sup> tributary area of the Chesapeake Bay to experimental plots of a few hectares near Watkinsville, Georgia. Some specific examples of HSPF applications are summarised below:

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*Truckee-Carson River, California and Nevada:*

The ability of HSPF to manage different categories of water ownership or -rights was used to simulate flow routing and reservoir- and river options. The aim was to provide simulations that allow comparison of the effects of alternative management practices or allocations on streamflow- or reservoir storages over long periods of time (Berris, Hess & Bohman, 2001).

*Waterford River Catchment, Newfoundland, Canada:*

Streamflow was simulated in this small watershed in Newfoundland, Canada. The purpose of the study was to determine the impact of increased urbanisation on streamflow. The model was calibrated against a scenario of future land-use change (primarily conversion of pervious to impervious land). The results verified that, by doubling the amount of impervious land could increase peak flows by about 20% (Ng & Marsalek, 1989: 117-124).

A later application investigated the effects of climate change on streamflow. Input temperature values were increased by up to 4% and precipitation was varied between 90% and 110% of its current value. The precipitation changes had a larger effect on the streamflow. Annual and seasonal streamflow fluctuations were directly proportional to precipitation changes (Ng & Marsalek, 1992: 257-272).

*North Reelfoot Creek, Tennessee:*

HSPF was used to simulate streamflow and sediment in a small catchment in Tennessee. The catchment is primarily agricultural. There are significant soil



erosion and sedimentation problems in the catchment (Chew, Moore & Smith, 1991: 10-16).

*Experimental Agricultural Catchment, Quebec:*

Streamflow and atrazine concentrations in this small experimental catchment in Quebec were simulated (Laroche, Gallichand, Lagacé & Pesant, 1996).

*Dade County, Florida:*

Concentrations of non-point source pollutants (sediment, nutrients and pesticides) in surface runoff and groundwater were simulated. All simulations were performed using the PERLND (Pervious Land Segment) module only, because no instream processes were simulated (Tshirintzis, Fuentes & Gadipudi, 1996).

*Dominican Republic: Hydropower Study:*

An early application of HSPF in a hydropower study of the Rio Yaque del Norte Catchment for the Dominican Republic undertaken by Hydrocomp, Incorporated in 1980 (Donigian, Imhoff, Bicknell & Kittle, 1984: 131).

*Clinton River Stormwater Management Study:*

An early version of HSPF was used by the Macomb County (Michigan) Public Works Department to evaluate alternative stormwater management practices in the Clinton River Catchment (Donigian *et al.*, 1984: 133).

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### 1.4.3 Applications in South Africa

The use of HSPF is a relatively recent occurrence in South Africa, the first applications taking place in the late 1980's (Johanson, 1989 & De Vos, 2002). Consulting engineers, water resource planners and hydrologists tend to use other models, because these models have been researched and experimented with during the last two decades.

The availability of resources and references with regard to HSPF water management is limited. Consultants undertook some of these studies and these references are only available from one of the parties involved. Therefore, most previous HSPF applications in South Africa lie outside the traditional scientific literature and are difficult or impossible to obtain.

The information obtained was either via personal communication or conversation and preliminary reports. The following applications occurred in South Africa:

*Hlobane Colliery, Vryheid Coalfields, South Africa:*

The purpose of this study was to determine the long-term water quality risks after the mine closure and to evaluate the effects of various water management actions. An integrated assessment approach was adopted that incorporated hydrological-, hydrogeological-, mineralogical- and geochemical assessment and modelling techniques to predict the volumes and qualities of water discharging from various points on the mine for the base case situation where no water management options were implemented (Hattingh, Pulles, Krantz, Pretorius & Swart, 2001: 2).

HSPF was used to obtain a water balance for the Hlobane 1 system in terms of the apportionment of mean annual precipitation to ET, streamflow and loss to the underground mine workings. A lack of reliable flow data (high flows) for the streams on top of the mountains as well as the difficult access to the area complicated this exercise. These data allowed for satisfactory calibration of parameters associated with base flow conditions such as the interflow recession rate and the active groundwater recession constant.

In the area of Hlobane II, HSPF could not be directly calibrated with observed surface flow, due to the fact that there were no defined streambeds, or mining induced fractures and subsidence areas existed. Therefore the same parameter set obtained at the Hlobane I system was used, and compared to the resultant ingress of water into the associated mine workings with that derived from flow measurements taken at the discharge from the workings.

Furthermore HSPF was used to model the catchments on the plains around Hlobane. The hydrology as well as water quality constituents were modelled. The impacts of mining induced water contamination were simulated by modelling total dissolved solids (TDS), sulphate ( $\text{SO}_4$ ) and sodium (Na) concentrations as typical substances in the Manzana-, Tshoba- and Sithebe river catchments. After a successful iterative calibration process, the model was used to assess the benefit of various proposed management strategies and for the generation of discharge hydrographs in weekly time steps.



The results showed that model derived ingress rates correlated well with the difference in land-use characteristics observed between the two areas. In all instances the predicted water qualities improved over time, with the result that the current monitoring results for the key parameters involved (sodium, TDS and sulphate) represent the worst case in terms of future scenarios.

*Rural Catchments of the Berg River Basin, Western Cape:*

This study was undertaken to compare different non-point source water quality models, which are capable of simulating phosphorous production in rural catchments with a variety of land-use types. These four rural catchments were the Doring-, Kompanjies-, Leeu- and Twenty-Four River catchments in the Berg River Basin, Western Cape. (Matji & Görgens, 1999: 2-4).

The emphasis was on the link between management decisions and non-point source assessment techniques. HSPF was successfully applied to simulate the hydrology of these catchments and, at the time of writing, the simulation of phosphorous production was still under investigation.

*Urban Catchment, Pinetown, Kwa-Zulu Natal, South Africa:*

The purpose of this study was to simulate the hydrology, sediment and phosphorous in this highly urbanised catchment. This 90-hectare catchment is situated in the Central Business District (CBD) of Pinetown (Johanson, 1989: 104).

The Council for Scientific and Industrial Research (CSIR) was responsible for the collection of precipitation- and streamflow data from 1982 to 1987. Only three years of data were used. Daily evaporation data were also used.

A good fit between the daily observed- and simulated streamflow was obtained during the hydrological calibration. The simulation of the accumulated flow over time was 7% lower than observed streamflow overall. This could be ascribed to the fact that some of the observed baseflow was non-natural in origin, typical examples being leaking water mains and wash water from industries. These non-natural components were not included in the modelling attempt. Due to the fact that data from only two precipitation stations were used or available as input to the model, the overall model performance and agreement were considered accurate.

Sediment calibration is dependent on the hydrological simulation. The observed data for sediment were in the form of event loads. During the three-year simulation period there were about 240 precipitation events. The sediment simulation was calibrated until there was a good fit between observed data and simulated values. The simulation indicated that almost all the sediment came from impervious land, but it was not always the case in severe events.

Adsorbed phosphorous and dissolved phosphorous are dependent on the sediment calibration. The mechanisms involved in these two forms of phosphorous are different and therefore they were simulated separately. A close fit between the simulated accumulated quantities of the soluble- and total

phosphorous and observed data were obtained, but the agreement for individual events was unacceptable.

*Rural Catchment (Midmar) of the Mgeni River Catchment, Kwa-Zulu Natal, South Africa:*

The purpose of this study was to simulate the hydrology and water quality of the Midmar Catchment, an important rural sub-catchment of the larger Mgeni River Catchment, which covers an area of 300 km<sup>2</sup>. The Midmar Dam is also a very important water source and it is eutrophically sensitive (Johanson, 1989: 102, 106).

The Midmar Catchment consists of two sub-catchments. The Lions River in the north drains the one sub-catchment, while the Mgeni River drains the other sub-catchment in the south. Due to time constraints, only the Mgeni River sub-catchment was simulated. The Mgeni River sub-catchment, which covers an area of 299 km<sup>2</sup>, was delineated into six pervious sub-catchments, taking all the land-use characteristics and regional variations in mean annual precipitation into consideration.

The period of 1980 through to 1985 was modelled. Daily precipitation data from five precipitation stations were selected. These daily precipitation data were disaggregated into hourly values by using data from three nearby precipitation stations at the Cedara Research Station. Daily evaporation- and streamflow data were also used (Johanson, 1989: 107).



The period of 1980 through to 1982 was used for hydrological calibration, while the period of 1983 through to 1985 was used for verification. The agreement between the simulated- and observed accumulated flow over time was considered accurate.

Comprehensive water quality data were only available in the Lions River sub-catchment. Due to time limitations, the water quality data were treated as if they are applicable to the Mgeni River sub-catchment. This assumption's error was not severe, due to the similarity in the land-use mix and precipitation data of these two sub-catchments.

A good fit between the observed and simulated accumulated suspended solids was obtained during the water quality calibration, except for the periods of 27 March 1982 and 14 February 1985. Poor streamflow simulations were also associated with these periods, thus resulting in the inaccurate modelling of individual events.

#### *Maputo Iron Steel Project, Southern Africa:*

This was a water demand study undertaken by Coleman and Van Niekerk (1997) as part of a draft report by Wates, Meiring & Barnard (1997) for Gibb Africa, Midrand, South Africa. Hydrology and water quality were simulated.

The Msunduzi River Catchment extends over a large area with numerous river reaches and relatively lower quality meteorological data, when compared to the above-mentioned applications in South Africa. Thus, it was necessary to spend

much more effort on data collection (catchment-, hydrological- and meteorological data), interpretation of the data, model calibration and verification. Furthermore, the fact that the Msunduzi River Catchment constitutes a substantial portion of the Mgeni River Catchment emphasises the importance of this project's results to help manage the hydrology and water quality of the larger Mgeni River Catchment of approximately 4000 km<sup>2</sup>.

### **1.5 AN OVERVIEW AND SUMMARY OF THE STUDY AREA**

The Msunduzi River Catchment covers an area of 901 km<sup>2</sup>. It is located in the province of Kwa-Zulu Natal, South Africa. Rising in the northwestern extremity of the catchment at an altitude of  $\pm 1520$  m above sea level near Ka-Nzakane and at an altitude of  $\pm 1390$  m above sea level near Mafunze in the southwest, the perennial Msunduzi flows 150 km (excluding tributaries) to its confluence with the Mgeni River at an altitude of  $\pm 285$  m.

Urban- and informal areas within the catchment include Pietermaritzburg and Cato Ridge on the southern catchment border. Vulindlela is the most important informal settlement in the Msunduzi River Catchment, just west of Pietermaritzburg (Tarboton & Schulze, 1992: 2). Precipitation over the catchment is highly variable with the mean annual precipitation ranging from in excess of 1186 mm to 684 mm.

The Department of Water Affairs and Forestry (DWAF) divides the Msunduzi River Catchment into the management sub-catchments of Henley, Inanda, Pietermaritzburg and Table Mountain as listed in Table 1.2.

**Table 1.2:** Management sub-catchments of the Msunduzi River Catchment

Management sub-catchments	Area (km <sup>2</sup> )	Percentage-distribution (%)
Henley	220	25
Inanda	19	2
Pietermaritzburg	319	35
Table Mountain	343	38
<b>Total:</b>	<b>901</b>	<b>100</b>

The catchment data such as specific location, topography, geological data, land-use and vegetation, as well as the hydrological- and meteorological data are discussed in detail in Chapter 5.

## 1.6 THESIS SCOPE

This thesis describes the application of the streamflow generation component of HSPF and the development of the input data of the HSPF Msunduzi River Model. Chapter 1 describes the background of the study area, the objectives and scope of this modelling attempt and the history and previous applications of HSPF. Chapter 2 provides a literature review of hydrological modelling in general, the role of hydrological modelling in water resource management and the comparison between some of the different hydrological models available or in use in South Africa.

The development and structure of HSPF is discussed in Chapter 3. The parameters and calibration of the HSPF model, as well as the applicable array of model- performance statistics are discussed in Chapter 4.



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Chapter 5 describes the preparation and development of input data (catchment-, hydrological- and meteorological data) for HSPF. Extensive analyses of catchment-, hydrological- and meteorological data were performed to ensure the best possible simulation was achieved. Chapter 6 presents the results concerning data collection, hydrological simulations, calibration, verification and analysis of model performance. Chapter 7 presents the conclusions and recommendations on the results discussed in Chapter 6.

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# **LITERATURE REVIEW: HYDROLOGICAL MODELLING IN GENERAL**

**HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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## 2. LITERATURE REVIEW: HYDROLOGICAL MODELLING IN GENERAL

### 2.1 HYDROLOGICAL MODELLING

#### 2.1.1 Introduction

It is important to have a good understanding of hydrological principles before attempting to use any computer models to design stormwater systems and structures. All models have limitations and are designed for specific applications. Computer modelling can be one of the more effective and efficient methods for predicting the quantity and nature of runoff and the effectiveness of best management practices (Brach, 2000: 8.00-1).

Snyder and Stall (1965) defined a “model” as follows: “A model is simply the symbolic form in which a physical principle is expressed. It is an equation or formula, but with the extremely important distinction that it was built by consideration of the pertinent physical principles, operated on by logic and modified by experimental judgement and plain intuition”.

Schulze (1995) expanded this definition to hydrological models by stating that a hydrological model can be defined as a mathematical model representing one or more of the hydrological processes resulting from precipitation and culminating in catchment runoff. A model provides a way of transferring knowledge from a study area to an area where hydrological information and decisions are needed, thus it represents a quantitative expression of the observation, analysis and prediction of hydrological responsiveness.



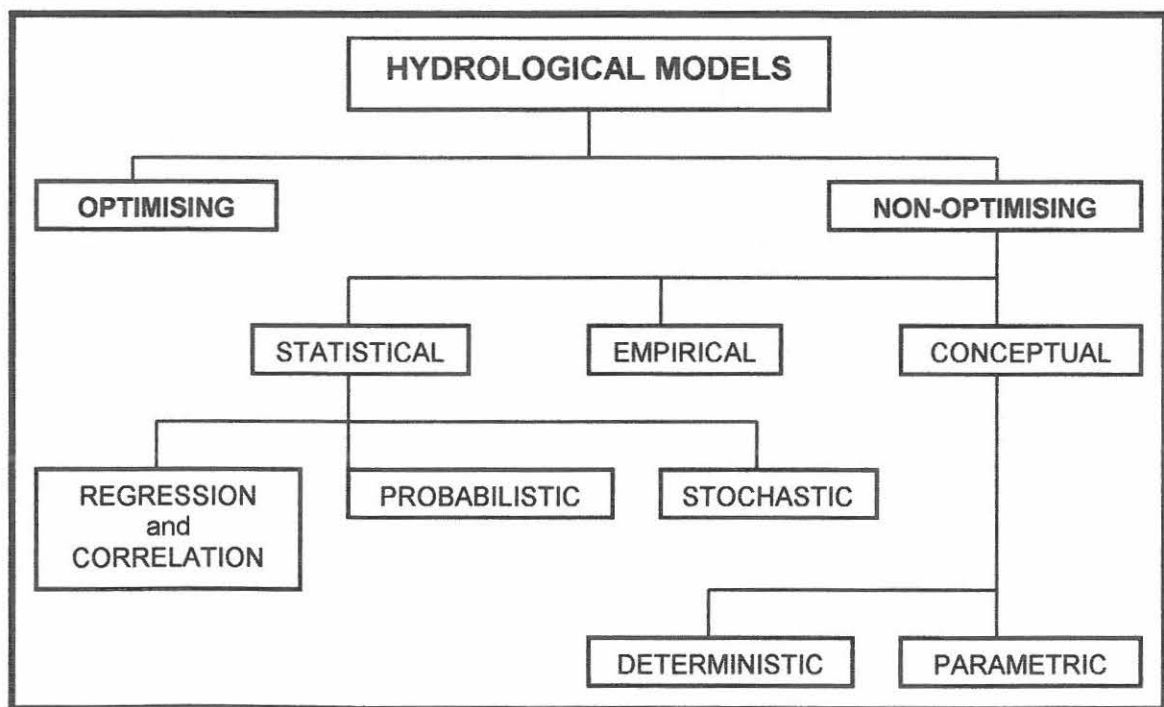
Furthermore, hydrological models are planning tools that ask “what if” questions. The model user asks the questions. The model developer anticipates the question and develops the capability to answer them through the model. Soundly structured hydrological models may therefore provide answers to “what if” scenarios where observed catchment data are not available, or studies have not yet been carried out (Dent, 2002). The model could provide information on the effect of land-management practices on quantity and quality of runoff, infiltration, lateral flow, both saturated and unsaturated subsurface flow, and deep percolation (De Coursey, 1991).

A model is most accurate when it is used with actual data and measurements. Observed data to varying degrees are used as part of individual components of the model. Errors associated with observed data will also impact the ability of the model to reliably produce the desired simulation (Brach, 2000: 8.20-1). The number of actual measurements applied to calibrate a model is important, because it will determine how accurate the model will predict future scenarios when no data are available (Dent, 2002).

Thus, the traditional, older water-resource analyses of historical records are clearly inappropriate planning tools in any changing technological environment. Historical patterns and trends should therefore be interpreted, not as predictors of the future, but rather as baselines against which the effects of changes can be compared. This is essential for effective planning and resource management (Brach, 2000: 8.10-1).

### 2.1.2 Types of Hydrological Models

Hydrological models can be classified as either optimising (decision theory, system analysis and operations research techniques) or non-optimising models (statistical-, empirical- and conceptual models). Statistical models are then divided into three categories, viz. regression and correlation-, probabilistic- and stochastic methods. Conceptual models may further be sub-divided into deterministic and parametric models. A parametric model is also deterministic in the sense that it includes some deterministic components, although the model parameters are not necessarily defined as measurable physical quantities (Green & Stephenson, 1986: 4). Figure 2.1 illustrates the classification of the various modelling approaches.



**Figure 2.1:** Classification of hydrological models (Green & Stephenson, 1986)

**Stochastic:** The random variables in these models use probability distributions and take the chance of occurrence or probability distribution of hydrological variables into consideration (Schmitz & De Villiers, 1997: 14). The sequence of events are treated as time-dependent and applied in extending hydrological forecasts, either as random or non-random data or the combination thereof (Green & Stephenson, 1986: 2).

**Empirical:** These models are formulated purely on observations. The output is generated from input based on experiments and observations, therefore it does not rely much on the physical processes involved (Green & Stephenson, 1986: 4).

**Deterministic:** The theoretical structure of these models are physically based, that is when all the governing physical laws are known and could be described by equations of mathematical physics (Green & Stephenson, 1986: 2). All the input variables in these models are considered to be free from random variation. In some cases it may include stochastic processes to add the dimension of spatial and temporal variability to some of the sub-processes (Hydrocomp, 2003: 195). Deterministic models can further be sub-divided as follows by making use of a set of criteria as presented by the ASAE (American Society of Agricultural Engineers):

- A *single-event model* represents a single runoff event occurring over a period of time ranging from about an hour to several days. The initial conditions in the catchment for each event must be assumed or determined and supplied as input data. The accuracy of the model output may depend on the reliability



of these initial conditions. A single-event model may exclude one or both of the subsurface components, as well as evapotranspiration (Hydrocomp, 2003: 195 & Dent, 2002).

- A *continuous model* operates over an extended period of time, determining flow rates and conditions during both runoff periods and periods of no surface runoff. Thus the model keeps a continuous account of the basin moisture condition and therefore determines the initial conditions applicable to runoff events. These models utilise three runoff components: surface runoff, interflow and groundwater flow (Hydrocomp, 2003: 195).
- Precipitation and other meteorological data are the primary input for *complete* or *comprehensive models* and the output is simulated hydrographs. The model represents the hydrological processes, which significantly affect the runoff, while maintaining the water balance. This is achieved by solving the continuity equation (water balance) of precipitation, evapotranspiration (ET) and runoff, thus increasing the accuracy of the model. This represents one of the most important advantages of complete models over partial models.
- A *partial model* represents only a part of the hydrological processes concerned, and it does not take the heterogeneity of the catchment into consideration. The input- as well as the output variables is spatially averaged (Kienzle, Lorentz & Schulze, 1997: 5).
- A *calibrated parameter model* has one or more parameters that can be evaluated only by fitting simulated hydrographs to the observed hydrographs. Calibrated parameters are usually necessary if the catchment component has any conceptual component models. These models are data demanding and

the transfer of ungauged catchments is problematic and speculative (Kienzle *et al.*, 1997: 5).

- In a *measured parameter model* all the parameters can be determined satisfactorily from known catchment characteristics, either by measurement or estimation (Hydrocomp, 2003: 196).
- *Lumped models* are structured to utilise average values of the catchment characteristics affecting runoff volume, thus the spatial variability of inputs, outputs or parameters are not explicitly taken into account. Averaging certain parameters also implicitly averages the process that being simulated. Non-linearity and threshold values can also lead to significant error (Hydrocomp, 2003: 196).
- *Distributed models* include spatial variation in inputs, outputs, and parameters. Normally, the catchment area is delineated into a number of reaches and runoff volumes are calculated separately for each reach. Models can be mistakenly classified as lumped, even though these models are able to represent spatial variability by subdividing the catchment into segments with representative "lumped" parameters for each segment (Hydrocomp, 2003: 196).
- *General models* are used without modifications to catchments of various types and sizes. These models have parameters, either observed or calibrated, which can adequately represent the effects of a wide variety of catchment characteristics. Conceptual models (with calibration parameters) are normally used to achieve this (Hydrocomp, 2003: 196).
- *Special purpose models* are applicable to a particular type of catchment in terms of topography, geology or land-use. Usually these models can be

applied to catchments of different sizes, as long as the characteristics of the catchments are the same (Hydrocomp, 2003: 196).

Any catchment can be divided into a number of sub-catchments. The unique hydrological and water quality characteristics of some of the homogeneous parts of these sub-catchments, as well as factors such as the lagging and attenuation of floods through river reaches and reservoirs should be taken into account (Kienzle *et al.*, 1997: 6 & Schulze, 1995: AT 2.9).

In order to represent hydrological processes realistically, the time step-interval of the model is important. Precipitation events normally occur over short periods, but it is the driving force on which the system's long-term streamflow is based (Kienzle *et al.*, 1997: 6).

#### 2.1.2.1 Advantages of Hydrological Models

The advantages of using hydrological models are as follows:

- The model provides a frame of reference for a problem and it can point out information gaps and thus suggests needed research (Brach, 2000: 8.10-1).
- The model brings out the problem of abstraction in complex systems and uncovers questions that might not otherwise be raised. It promotes understanding and added insights into the different processes in the hydrological system.
- The model, once expressed, provides relatively easy manipulation of components and a basis for comparison (Schulze, 1995: AT 1.11).



- Time scales are significantly compressed. Observations made over many years in the physical system are reproduced in a much shorter period of time. This is also an advantage in certain circumstances; for example, one cannot build a dam and wait for 30 years or longer to see what happens (Dent, 2002).

#### 2.1.2.2 Disadvantages of Hydrological Models

The disadvantages of using hydrological models are as follows:

- Most models are only effective under certain specified conditions.
- Models involve gross simplifications when representing the true physical system.
- Models use a number of mathematical or graphical representations (“lumped” model parameters) to describe various hydrological and hydraulic concepts (spatial heterogeneity of catchment), each of which is considered to be relevant to the overall hydrological response of the catchment (Brach, 2000: 8.10-2).

#### 2.1.2.3 Model Selection

Catchment- and human-impact related considerations are the primary criteria in model selection. Additional to these criteria, management and administrative considerations (availability of technical support, costs, user-friendliness, version control and training) and technical capabilities (data set requirements and model capabilities) inform the process of model selection (Görgens, Tanner, Viljoen, Sami, Dennis *et al.*, 2002: 17).

Even though there are no clear rules for the process of model selection, the following simple guidelines can be stated:

- The simplest method that can provide the answer to your questions must be used.
- The simplest model that will yield adequate accuracy must be used.
- Model selection must be appropriate for the problem, thus one should not try to fit the problem to a specific model (Hydrocomp, 2003: 196).

During the process of model selection, the following questions can also be asked:

- Which model is best for solving a particular problem in a particular location?
- What are the data requirements for both model and problem?
- What computer hardware and staff are required?
- What documentation is available?
- How much will it cost to apply the model?
- How accurate will the model be in representing the “real world”?
- Are there enough data available?
- What different kinds of models are available?
- Will it be applicable to land-use activities?
- Will it be applicable to broad geographic areas?
- How accurate will the prediction be?

(Görgens *et al.*, 2002: 18 & Brach, 2000: 8.10-3)

#### 2.1.2.4 Model Verification

A model should generally be verified by running the model against known conditions to compare simulated versus observed results. During verification the

data set used must not have been used before, during calibration. Monitoring error or interpretation of observed data can be one of the main sources of error in model verification. Precipitation can vary significantly within short distances, even fractions of a kilometre. If monitoring stations do not cover the entire catchment, assumptions must be made about the distribution of precipitation in the catchment (Brach, 2000: 8.10-4).

Grab samples do not show variability of concentration with flow unless flow data are correlated with concentration and a significant number of samples are taken. Any averaged or composite sample methods tend to disguise the extreme events.

Flow-weighted mean samples depend on good sample values for each representative unit of flow. Small flow-measurement errors or unrepresentative sample values can significantly affect any loading estimate. Systematic errors, such as using the highest possible values, may be desirable for “worst-case scenarios” but these values also tend to multiply in an unrealistic manner if not properly utilised (Brach, 2000: 8.10-4).

#### 2.1.2.5 Sensitivity Analysis in Hydrological Modelling

The key task in any modelling study is the sensitivity analysis and interpretation of the model outputs. Since models are simply tools for a quantitative, systematic analysis of specific environmental problems or issues, they do not provide simple “yes” or “no” answers to managers, regulators or decision-makers. Rather, they usually provide detailed information about the expected response of the system



to a given perturbation in order that a more informed objective decision could be made (Brach, 2000: 8.10-5).

The accuracy associated with the results of modelling studies depends on the model used, the accuracy of the input data, the characterisation of the environmental system being simulated, the expertise or experience and resources available to the model user. Most models are often more accurate in a relative sense, than in an absolute sense. That is, when models are used to compare alternatives (such as management- or control options), the relative differences predicted between alternatives are sometimes more reliable than an absolute value predicted for any one alternative (Brach, 2000: 8.10-5).

## **2.2 ROLE OF HYDROLOGICAL MODELLING IN WATER RESOURCE MANAGEMENT IN SOUTH AFRICA**

The hydrological world is already interrelated and it is therefore not necessary to create its interrelatedness: the challenge is to enable organisations and scientific disciplines to achieve a measure of interrelatedness so as to better understand and hence manage within the hydrological world. This interrelatedness is explicitly recognised in the National Water Act (NWA), 1998 (Act 36 of 1998) and has resulted in institutional and management changes (Dent, 2001).

The National Water Policy and NWA (1998) form the foundation of the National Water Resource Strategy (NWRS). The objectives of water resource management of the NWRS in South Africa were revised and include the following changes:

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- ❑ Ensuring security of supply for basic human needs;
  - ❑ Protecting water resources;
  - ❑ Ensuring equitable access to resources;
  - ❑ Improving the efficiency of water use;
  - ❑ Ensuring sufficient future water supplies to sustain health and economy and to promote a prosperous society;
  - ❑ Ensuring equity in payment for water and;
  - ❑ Honouring obligations to neighbouring countries (DWAF, 2002).

Another change is that, following international trends, there is a growing number of modelling systems for integrated water resource management on a catchment basis in Southern Africa. Such systems are likely to be installed for operational use in ongoing learning, research, strategic planning and consensus building amongst role players in the CMAs. These installed systems are poised to fundamentally change the way modelling is approached in Southern Africa. They are a logical and irreversible response to the enormous forces, which have led to the revision of the NWA and the water resource management paradigms (Dent, 2001).

Installed modelling systems will enable CMAs to address the contradictory calls for affordable modelling at the same time as recognising greatly increased complexity, scalability, accessibility and integration. Modelling systems are operationally successful because they address the timeliness issues which are important in today's fast moving world in which contentious water resource issues are becoming much more commonplace (Dent, 2001).

Water resource modelling systems perform processes on sequences of time-dependent data. Such systems must be flexible in terms of the temporal and spatial resolution of the processes being modelled. A key functional element of an installed modelling system is therefore a highly efficient time series management system (Dent, 2001).

The system should be able to model both quantity and quality both on the land and in the stream. It should be able to model both point and non-point sources of conservative and non-conservative pollutants. The system should be able to inter-operate with the functional modules of other systems. The networks of flows, abstractions, return flows, pumping and release regimes within a catchment are often complex. The flows in each reach or branch of the network will often be allocated to persons or institutions. It is therefore important that the modelling system keeps track of the water ownership or allocation categories (Dent, 2001).

The primary objective of this modelling process is the application of the streamflow component of the HSPF model. In future studies, the objective will expand to encompass water resources management at a catchment level as part of the greater Mgeni River Catchment, including water quality modelling and water allocation simulation as a management tool, taking the objectives of water resource management of the NWRS into consideration.

The installed modelling system framework must keep track of all the above-mentioned factors in a functional, flexible and robust manner and it should be



capable of modelling the responses to conditional anthropogenic interventions. According to Russo (Bicknell *et al.*, 1996) HSPF is the most comprehensive model in the category of continuous water budgeting models.

Therefore, the HSPF model was used in this hydrological modelling study of the Msunduzi River Catchment. However, this does not mean that HSPF is the best model to use in all circumstances, but compared to other hydrological models locally available it possessed the best combination of features for the present and future investigations.

## **2.3 COMPARISON OF HYDROLOGICAL MODELS IN GENERAL**

According to Hydrocomp (2003) physically based, continuous water budgeting models are the most accurate models currently available. The comparison, relative merits and application of some of these models in the continuous water budgeting category are discussed below.

### **2.3.1 ACRU Model**

The acronym ACRU is derived from the Agricultural Catchment Research Unit within the Department of Agricultural Engineering of the University of Natal in Pietermaritzburg, South Africa. The model has been verified widely on data from Southern Africa and the United States of America (USA) (Schulze, 1995: AT 2.1).

Important features:

- It is a physical conceptual model. The model conceives of a system in which important processes and couplings are idealised. Physical processes are represented explicitly.

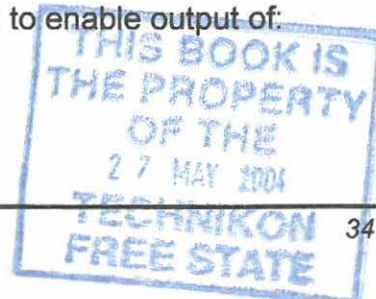
- This multi-purpose model integrates the various water budgeting and runoff-producing components of the hydrological system, taking all the aspects of risk analysis into account. These aspects of risk analysis can be integrated with simulated soil moisture changes antecedent to design events.
- The "heart" of the ACRU model is a daily multi-layer soil water budgeting system. The model simulates the components and processes of the hydrological cycle affecting the soil water budget, like the soil water deficit antecedent to a precipitation event on a daily basis (Kienzle *et al.*, 1997: 10).
- The critical response depth of the soil depends mainly on the dominant runoff-producing mechanism. Stormflow is split into quickflow (same day response) and delayed stormflow with a "lag". This "lag" is a surrogate for simulating interflow and is dependent on soil properties, catchment size and the drainage density. Therefore the model has been developed into a versatile total evaporation model, which takes all the above-mentioned factors into consideration (Schulze, 1995: AT 2.2-2.6).
- ACRU has been designed as a multi-level model. Multiple options or alternative pathways in many of its routines are characteristic. These options are dependent on the users needs or the level of available data or the detail of output required. Various methods, taking the level of data input into account, can be used to determine most of the hydrological components. Typical components are reference potential evaporation, interception losses, values of soil water retention constants, total evaporation, leaf area index, components of the peak discharge estimation, hydrograph routing and reservoir storage area relationships.

- ACRU's operation is either as a point or as a lumped small catchment model. ACRU can also operate as a distributed cell-type model. This distributed cell-type model can also be linked to a Geographic Information System (GIS) in order to integrate geographically spatial information for input into the model and to display output from the model (Schulze, 1995: AT 2.2-2.4).

Important applications:

- The ACRU model can be used for the estimation of design runoff as a consequence of the non-stationarity of catchment responses over time. The ACRU model can also illustrate potential changes in runoff responses to design precipitation under conditions of anthropogenically induced global climate change (Schulze, 1995: AT 2.8- 2.11).
- ACRU can be applied as a designing tool in hydrology, crop yield modelling, reservoir yield simulation and irrigation water demand or -supply, regional water resources assessment and planning optimum water resource utilisation (Kienzle *et al.*, 1997: 8).
- ACRU can be applied on a minute-by-minute grid of latitude and longitude of the area, when regional water resource assessment is of any concern. The total of the simulated streamflow at each grid point is then determined. By doing this, the regional production of runoff in average, wet and drought-stricken years can be estimated (Schulze, 1995: AT 2.8-2.11).

According to Schulze (1995) the components of the water budget are integrated with the different modules available in the ACRU model to enable output of:





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- Reservoir yield analysis.
  - Sediment yield analysis.
  - Determination of the hydrological response of wetlands.
  - The effect of water abstractions from streams on the catchment's water yield.
  - The hydrological impact of afforestation.
  - The effect of gradual or abrupt land-use and management changes.
  - The effects of enhanced atmospheric CO<sub>2</sub> levels on transpiration suppression and hence on crop yield and water resources.
  - The depositing, transport and export of sediment, total phosphorus and *Escherichia coli* from non-point sources into receiving water bodies. However, these routines are a relatively recent addition to the model and have not yet been subjected to extensive verification.

Limitations:

- The model only uses daily time steps, thus sub-hourly simulations is not possible (Görgens *et al.*, 2002: 22). Therefore, it is not suitable for simulating certain water quantity and quality studies in small or complex catchments with sub-daily responses.
- ACRU makes no provision for simulating the implementation of water allocation or ownership categories. This will be an important requirement of the NWA.
- ACRU can normally only be used in catchments with an urban land-use less than 20% (Schmitz & De Villiers, 1997: 2).

- 
- This model is not able to simulate the water quality of the base flow accurately and it does not make any provision for the chemical influence of sediments on the quality (Schmitz & De Villiers, 1997: 3).
  - The model uses a relatively simple but primitive sequential file format for managing the large numbers of time series, which are required as input or produced as output (Smithers & Schulze, 1995: AM 4-1). This impacts negatively on the efficient execution of the model on computer.
  - The model is not very flexible in terms of setting up connections of flows between operating modules.

### 2.3.2 HSPF Model

The modules of the HSPF model are arranged in a hierarchical structure for the continuous simulation of hydrological- and water quality processes in urban- and rural catchments (Bicknell *et al.*, 1996: iv). Detailed descriptions of the modules, which are important in the hydrological simulation are given in Chapter 3.

Extensive and flexible data management and statistical routines are available for analysing simulated or observed time series data. The HSPF software is linked to several important utilities developed under contract to the Environmental Protection Agency (EPA) and available in the public domain. These include the Generalised Scenario Generator Software (GENSCN) (EPA, 1997), Better Assessment Science Integrating Point & Non-point Sources (BASINS) (Lahlou, Shoemaker, Choudhury, Elmer, Hu *et al.*, 1998) software which links ArcView 3.2 and Arc Explorer GIS to the model user control input, and the Watershed Data Management (WDM) system for managing time series

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data. The model code is in the public domain and is freely downloadable from the Internet.

Important applications:

- ❑ Continuous hydrological simulation and water budgeting.
- ❑ Simulation of sediment production and removal.
- ❑ Simulation of nitrogen-phosphorous behaviour.
- ❑ Simulation of water temperature and numerous other water quality constituents using a generalised approach.
- ❑ Simulation of pesticide behaviour and movement of tracer elements.
- ❑ Analyses both point- and non-point sources of pollution.
- ❑ Simulation of separate categories of water allocation or ownership.
- ❑ Performs risk analysis due to the exposure of aquatic organisms to the toxic chemicals present in receiving waters (Bicknell *et al.*, 1996 & Aqua Terra Consultants, 2002).

Limitations:

- ❑ Requires extensive meteorological- and hydrological data.
- ❑ It is a complex model and it takes time to become fully conversant with its input files, its utilities and with the calibration of the model.
- ❑ Cost associated with different best-management practices is not linked to pollutant delivery (Donigian *et al.*, 1984).

Despite these limitations, HSPF has an excellent time series management system based on random access files and optimisation of data movement in

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memory, which results in fast execution when compared to other hydrological models.

### 2.3.3 Mike-SHE Model

The Mike-SHE model is a continuous water budgeting modelling system. It simulates all major hydrological processes in the land phase of the hydrological cycle. Therefore, this integrated model includes the dynamic exchange of water between all major hydrological components, e.g. surface water, interflow and groundwater. It is also physically based and distributed; the basic equations governing the major flow processes are solved and the spatial and temporal variation of meteorological-, hydrological- and geological data is described in a gridded form for the input as well as the output from the model. The modular structure of the model allows the expanding of water quantity simulations to deal with solute transport, particle tracking and geochemical reactions (Refsgaard & Storm, 1995).

Mike-SHE is computationally efficient and easily links regional- and local scale models. The following processes are included in the model:

- Evapotranspiration (ET);
- Two-dimensional overland flow;
- One-dimensional channel flow in river reaches;
- One-dimensional unsaturated flow;
- Saturated groundwater flow and;
- Infiltration, recharge, crop growth and nitrogen cycle analysis (SSG, 2003).

### Important applications:

The model has been applied in a large number of studies worldwide focusing on the conjunctive use of surface water and groundwater for domestic and industrial consumption, as well as irrigation, wetland dynamics and water quality studies (Gupta, Das, Raghuwanshi, Singh, Dutta *et al.*, n.d.). Typical applications are as follows:

- ❑ Surface water impact from groundwater withdrawal.
- ❑ Conjunctive use of groundwater and surface water.
- ❑ Wetland management and restoration.
- ❑ Catchment management and planning.
- ❑ Environmental impact assessment studies.
- ❑ Aquifer vulnerability mapping with dynamic recharge and surface water boundaries.
- ❑ Floodplain studies.
- ❑ Impact studies for changes in land-use, climate and agricultural practices (SSG, 2003).

### Limitations:

- ❑ Numerical simulation results are overly dependent on grid cell sizes larger than one square kilometre due to the tight linkage of surface- and groundwater in this code.



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- ❑ The lack of published examples of applying the model to pesticides.
  - ❑ Aquifer geometry data are required to apply the three-dimensional groundwater sub-model.
  - ❑ Numerical methods to solve the solute transport equations are under construction, thus problems concerning the conservation of mass, can be experienced (DeGloria, Pacenka, Porter, Anderson, Smith *et al.*, 1999).
  - ❑ The source code is not available, and the model is expensive to purchase.

#### 2.3.4 SWAT Model

The acronym SWAT is derived from Soil and Water Assessment Tool. SWAT is a continuous water budgeting model used to predict the impact of land management practices on water, sediment and agricultural chemical yields in large complex catchments over long periods of time (Arnold, Williams, Nicks & Sammons, 1990).

SWAT incorporates features of several Agriculture Research Service (ARS) models and is a direct outgrowth of SWRRB model (Simulator for Water Resources in Rural Basins) (Williams, Nicks & Arnold, 1985; Arnold, *et al.*, 1990). Specific models that contributed significantly to the development of SWAT were CREAMS (Chemicals, Runoff, and Erosion from Agricultural Management Systems) (Knisel, 1980), GLEAMS (Groundwater Loading Effects on Agricultural Management Systems) (Leonard, Knisel & Still, 1987), and EPIC (Erosion-Productivity Impact Calculator) (Williams, Jones & Dyke, 1984).

The model includes the incorporation of the urban build-up and/ or wash-off equations from Stormwater Management Model (SWMM), instream water quality based on the Enhanced Stream Water Quality Model (QUAL2E) kinetics, pond simulation and creation of interfaces with Windows, ArcView and BASINS (Neitsch, Arnold, Kiniry & Williams, 2001).

#### Important features:

The model is physically based. It requires specific information about meteorological data, catchment data and land management practices occurring in the catchment, rather than incorporating regression equations to describe the relationship between input and output variables. The physical processes associated with water movement, sediment transport, crop growth and nutrient cycling are directly simulated by SWAT using this input data (Arnold, *et al.*, 1990). The model is freely downloadable from the Internet.

#### Important applications:

- Unregulated catchments with no observed streamflow data can be modelled.
- The modelling of sediment transport, crop growth and nutrient cycling.
- The relative impact of alternative input data (changes in management practices, climate and land-use) on water quality or other variables of interest can be quantified.
- It is computationally efficient, thus the simulation of very large catchments can be performed without excessive investment of time or money.

- It enables users to study long-term impacts. Some of the problems currently addressed by users involve the gradual build-up of pollutants and the impact on downstream water bodies. Results from runs with output of several decades are needed to study these types of problems (Arnold, *et al.*, 1990).

Limitations:

- The model is not designed to simulate detailed, single-event flood routing.
- The model only uses daily time steps, thus sub-hourly simulations is not possible (Neitsch *et al.*, 2001).
- It has not been widely applied and verified as other models such as HSPF, and its application to studies in South Africa is still very limited.

### 2.2.5 SWMM Model

The acronym SWMM is derived from Stormwater Management Model. This model of the EPA is a comprehensive deterministic model for analysis of quantity and quality problems associated with urban runoff. The model can perform both single-event and continuous storm simulations on catchments (Huber, Heany, Nisi, Dickenson & Poleman, 1982 & Green & Stephenson, 1986: 16). Continuous simulation has not yet been extensively used in South Africa, chiefly due to problems encountered with inputting historical precipitation data (Green & Stephenson, 1986: 17).

Important applications:

- The modeller can simulate all aspects of the urban hydrological and quality cycles, including precipitation, snowmelt, surface and subsurface runoff,



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flow routing through a drainage network, storage and treatment  
(Brach, 2000: 8.40-18).

Limitations:

- Cannot simulate the effect of agricultural land-use.
- The lack of subsurface quality routing (a constant concentration is used).
- No interaction of quality processes (apart from adsorption).
- Difficulty in simulation of wetland quality processes.
- A weak scour-deposition routine in the Transport Block

(Brach, 2000: 8.40-18)

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# **LITERATURE REVIEW: HSPF DEVELOPMENT & STRUCTURE**

## **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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### 3. LITERATURE REVIEW: HSPF DEVELOPMENT AND STRUCTURE

#### 3.1 INTRODUCTION

The Hydrological Simulation Program-Fortran (HSPF) is a mathematical model developed under USEPA sponsorship to simulate hydrological and water quality processes in natural and man-made water systems. It is an analytical tool, which has application in the planning, design and operation of water resource systems. The model enables the use of probabilistic analysis in the fields of hydrology and water quality management (Aqua Terra Consultants, 2002).

HSPF uses historical information such as precipitation, temperature, evaporation and parameters related to land-use patterns, soil characteristics and agricultural practices to simulate the processes that occur in a catchment. The initial result of an HSPF simulation is a time history of the quantity and quality of water transported over the land surface and through various soil zones down to the groundwater aquifers (Aqua Terra Consultants, 2002). HSPF simulates for extended periods of time the hydrological- and associated water quality processes on pervious and impervious land surfaces, streams and well-mixed impoundments. HSPF uses continuous precipitation- and other meteorological data to compute streamflow hydrographs and pollutographs (Bicknell *et al.*, 1996: iv).

HSPF can simulate interception, soil moisture, interflow, groundwater recharge, base flow, surface runoff, evapotranspiration (ET), temperature, snowpack depth and water content and snowmelt. It can also simulate the processes of sediment



detachment and transport, sediment routing by particle size, channel-, reservoir- and constituent routing. Furthermore HSPF can simulate the effects of dissolved oxygen (DO), biochemical oxygen demand (BOD), pesticides, conservatives, faecal coliforms, pH, ammonia, nitrite-nitrate, organic nitrogen, orthophosphate and organic phosphorous (Donigan *et al.*, 1984: 171-177).

There is no limitation on the period that may be simulated (USGS, 2002).

### 3.2 HSPF: DEVELOPMENT

The Environmental Research Laboratory in Athens, Georgia sponsored the original development of HSPF. The USGS Water Resources Division in Reston, Virginia has sponsored recent development of HSPF. Hydrocomp Incorporated performed the initial HSPF and user's manual development work. Anderson-Nichols & Company and Aqua Terra Consultants performed subsequent revisions and extensions to the HSPF code and user's manual (Bicknell *et al.*, 1996: 6).

Taking the above-mentioned into consideration, it is clear that HSPF is not the product of one person or organisation. It has been developed, expanded and crafted over the last 25 years.

### 3.3 MODEL STRUCTURE AND DESIGN

HSPF consists of a set of modules arranged in a hierarchical structure. These modules permit the continuous simulation of hydrological- and water quality processes. In the application of complex models much of the human effort is associated with data management. Sound data management is an important

component in successful comprehensive models, because the user may become so entangled in data manipulation that his progress on the simulation work itself is badly influenced. The HSPF software is planned around a time series management system operating on direct access principles.

Simulation modules can draw input from time series storage files and therefore it can write output to them. Thus, these transfers require very few instructions from the user. The above-mentioned problems are then also minimised (Bicknell *et al.*, 1996: 2).

Various simulation- and utility modules can be invoked conveniently into the system, either in tandem or individually. The emphasis is on structured design and therefore a “top down” approach has been followed (Bicknell *et al.*, 1996: 2).

The first step was to design the overall framework and the time series management system. Thereafter, the work progressed down the structure from the highest, most general level to the lowest, most detailed one. Structured design has made the system easy to extend. Therefore users can add their own modules with little disruption of the existing code (Bicknell *et al.*, 1996: 2).

The “real world” is a continuum of constituents and processes, which are subdivided into “elements”. These elements consist of “nodes” and “zones”. Nodes correspond to a point in space and a particular value of a spatially variable function can be associated with it. Zones correspond to a finite portion of the “real world” and it is associated with the integral of spatially variable quantities.

Zones are the smallest units into which the world is subdivided. The relationship between zonal and nodal values is comparable with that between the definite integral of a function and its values at the limits of integration. When the response of land phases of the hydrological cycle are simulated, these elements are known as "segments." A segment is a portion of land, which is assumed to have uniform properties (Bicknell *et al.*, 1996: 9).

The model builder decides what grouping of these "segments" is reasonable and meaningful when the "real world" is being simulated. This is based on his view of the "real world" processes. Nodes are used to define the boundaries of zones and elements. "Fluxes" are a zone, characterised by storage that receives inflows and disperses outflows (Bicknell *et al.*, 1996: 11).

### **3.3.1 PERLND Module (including section PWATER)**

Water, sediment and water quality constituents leaving the catchment are assumed to move laterally to a downslope segment, reach or reservoir during the modelling process. Pervious Land Segments (PLS) have the capacity to allow enough infiltration to influence the water budget. PERLND is the module that simulates the hydrological- and water quality processes, which occur on a pervious land segment (Bicknell *et al.*, 1996: 37).

The primary module sections and their functions in PERLND are the following:

- ❑ SNOW (Simulates snow accumulation and melt);
- ❑ PWATER (Water budget, total runoff from pervious areas);
- ❑ SEDMNT (Sediment produced by land surface erosion) and;

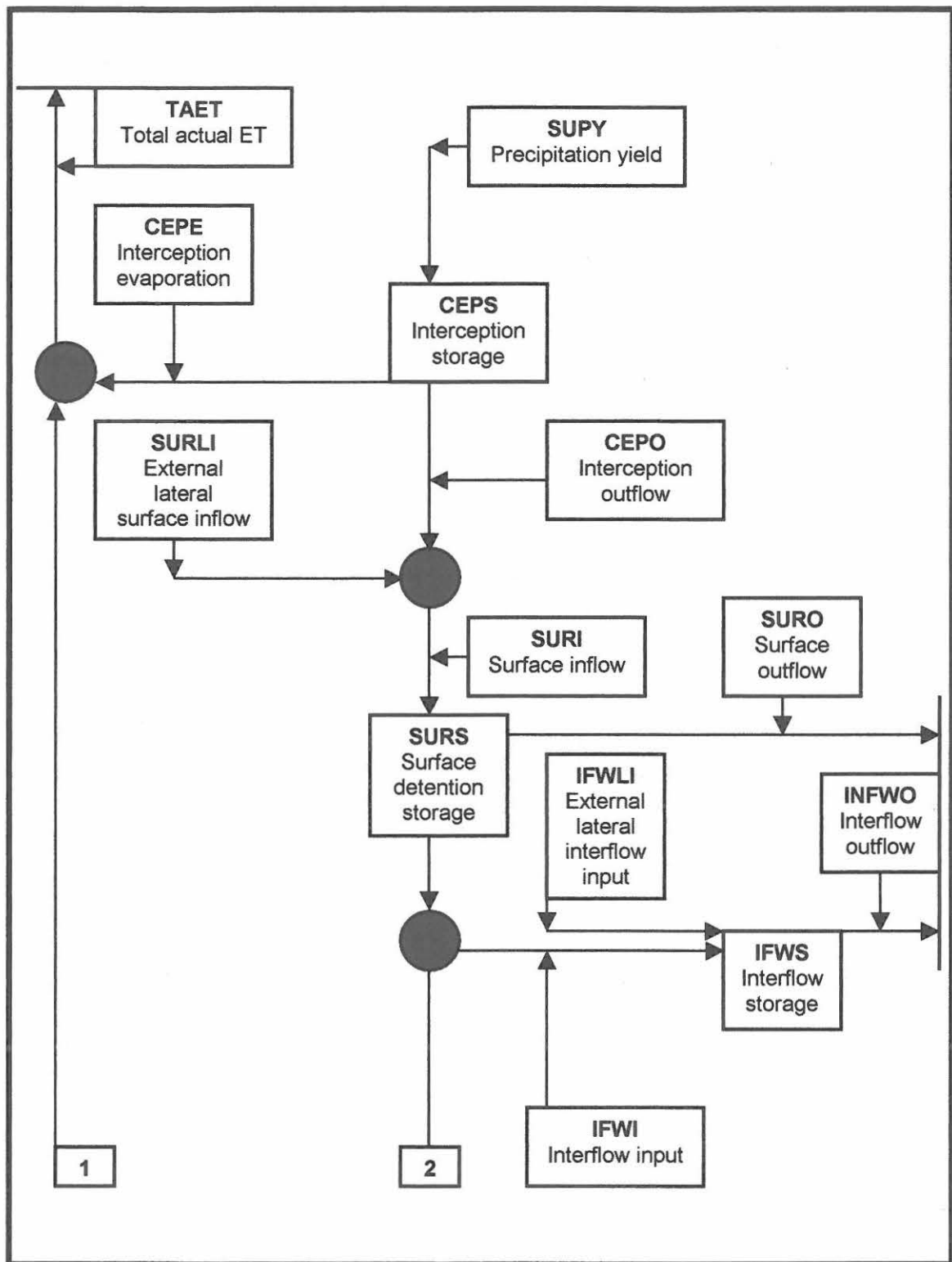


- 
- PQUAL (Water quality constituents by various methods).

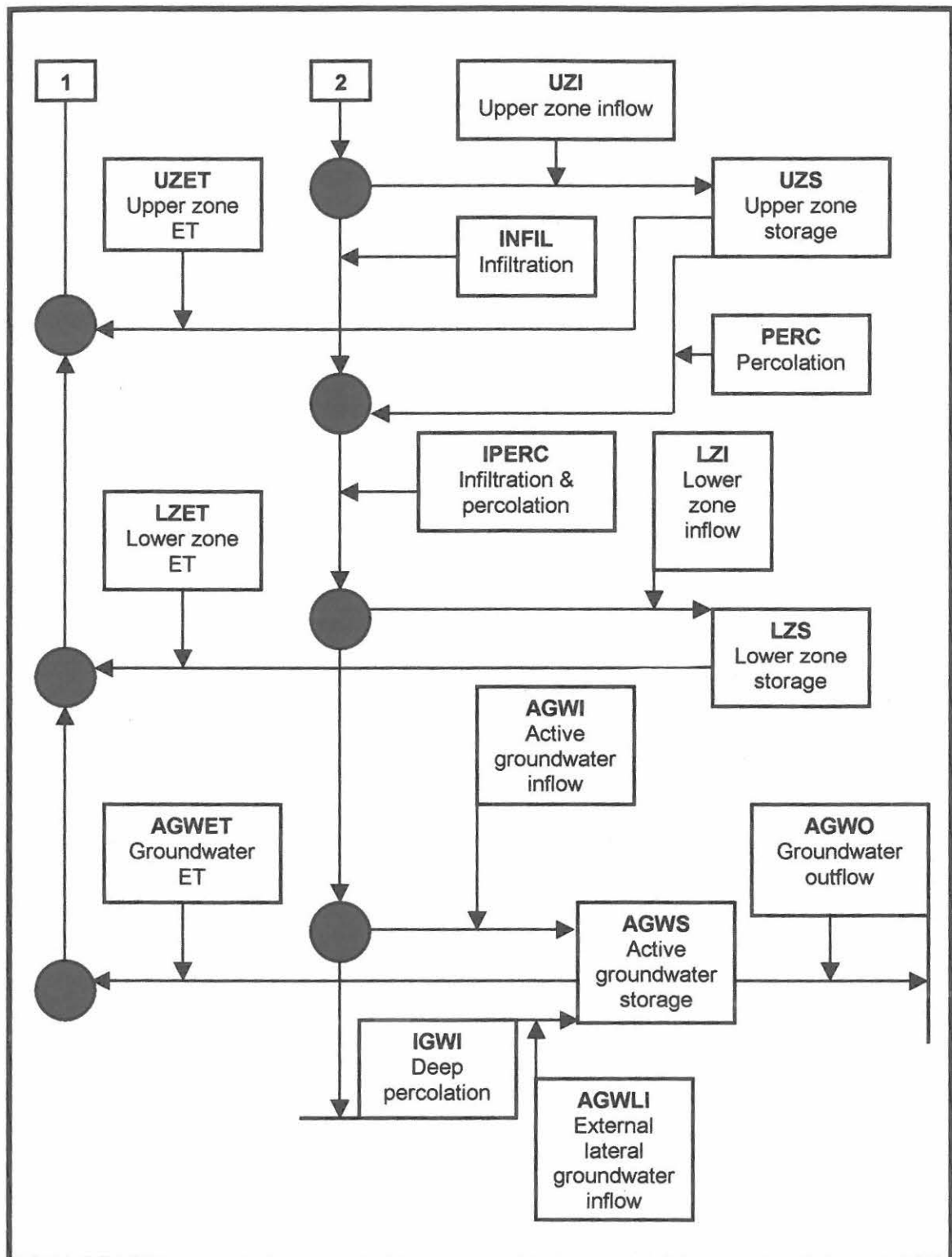
PERLND models the movement of water along three paths: overland flow, interflow and groundwater flow to simulate the above-mentioned processes. The time delay and interactions between water and its various dissolved constituents are different (Bicknell *et al.*, 1996: 37).

PWATER is the key hydrological component of module section PERLND. This module section predicts the total runoff from a pervious area by calculating the components of the water budget. When snow accumulation and -melt are not considered, the module section PWATER requires only potential evapotranspiration (ET) and precipitation as input time series data (Bicknell *et al.*, 1996: 54).

Figure 3.1 represents the water movement and storages simulated in module section PWATER. All these parameters, which play an important role in the complex series of pathways and storages in module section PWATER are discussed in detail in section 3.5 of this chapter, as well as in Chapter 4.



**Figure 3.1:** Flow diagram of water movement and storages simulated in the PWATER section (PERLND module) (Bicknell *et al.*, 1996: 57)

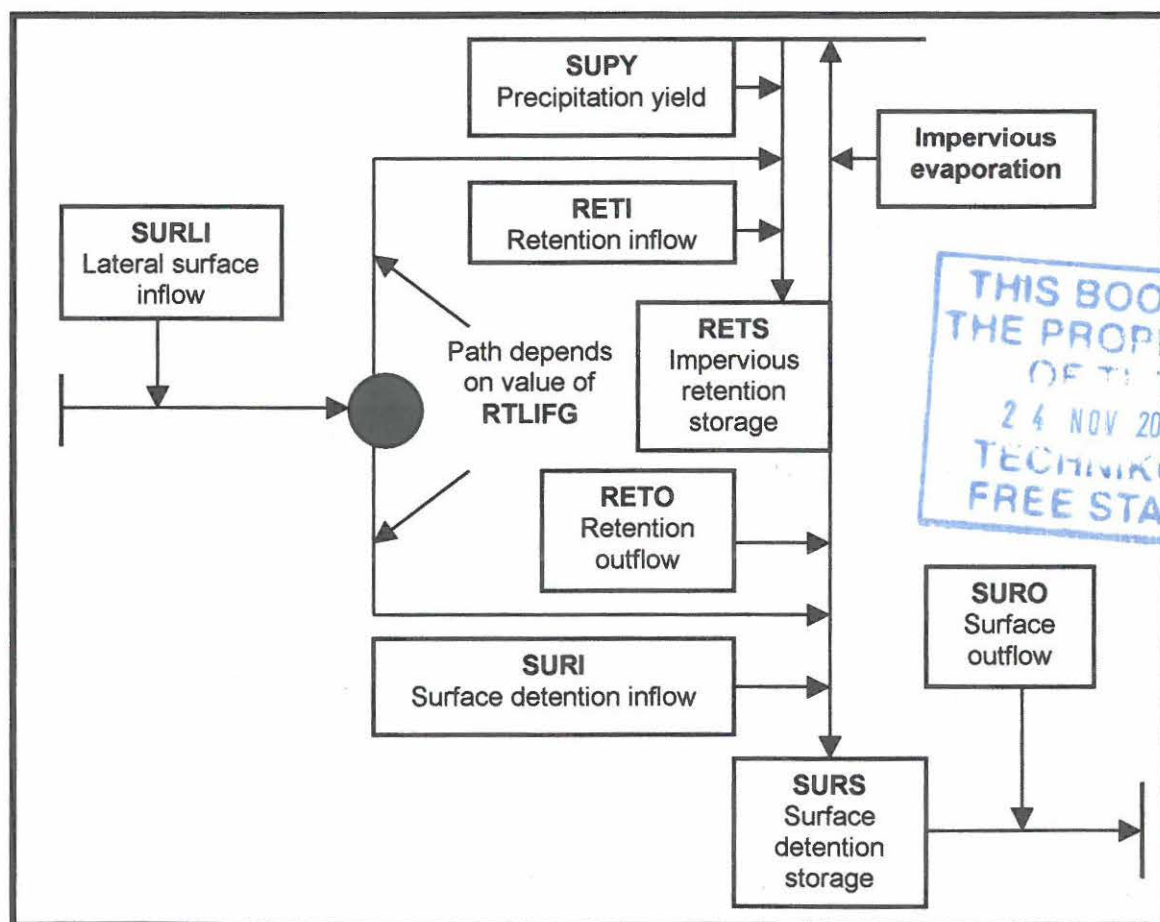


**Figure 3.1 (continued):** Flow diagram of water movement and storages simulated in the PWATER section (PERLND module) (Bicknell *et al.*, 1996: 58)



### 3.3.2 IMPLND Module (including section IWATER)

Impervious Land Segment (IMPLND) is the module that simulates the water quality and quantity processes, which occur on an impervious land segment, where little or no infiltration occurs. IWATER (Water budget for impervious land segments) simulates the retention, routing and evaporation of water from an impervious land segment. Section IWATER is similar to section PWATER of the PERLND module. In IWATER there is no infiltration and consequently no subsurface processes. IWATER has the same time series requirements as section PWATER (Bicknell *et al.*, 1996: 114). Figure 3.2 represents the fluxes and storages simulated in module section IWATER.



**Figure 3.2:** Flow diagram of water movement and storages simulated in the IWATER section (IMPLND module) (Bicknell *et al.*, 1996: 117)

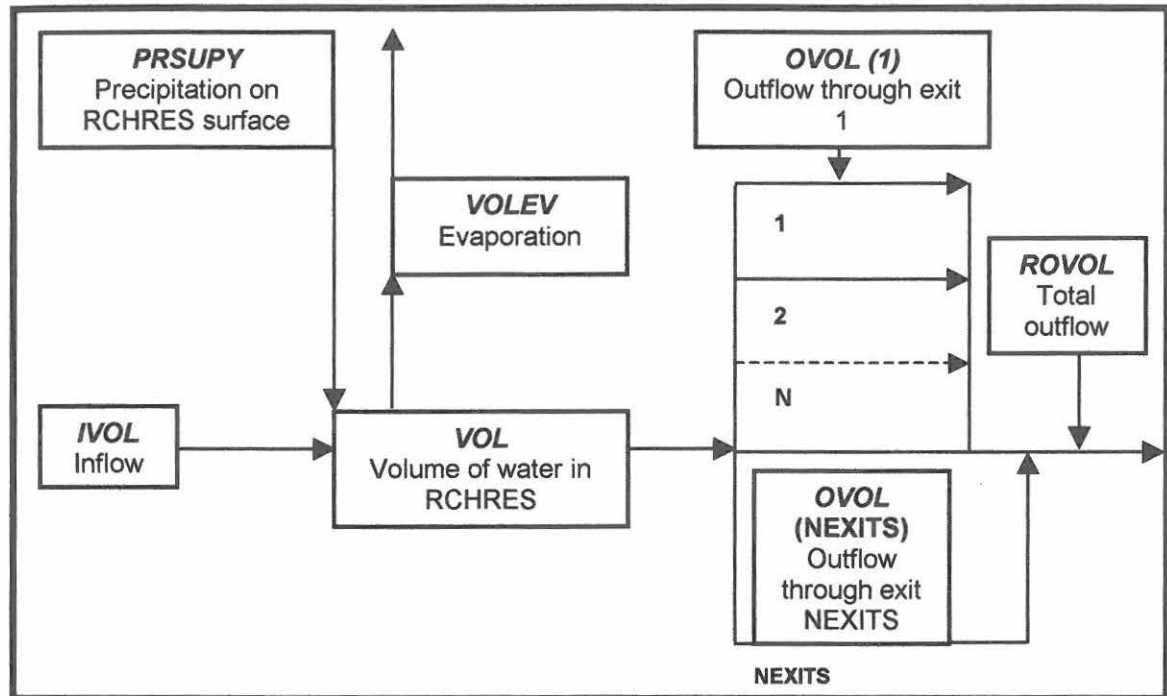
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### 3.3.3 RCHRES Module (including section HYDR)

RCHRES (Runoff Simulator in a Single Reach) routes the movement of runoff water and its associated water quality constituents simulated by PERLND and IMPLND, through river networks and reservoirs. These processes occur in a single reach of an open or closed channel or a completely mixed lake. Due to the assumption of complete mixing, the RCHRES module consists of a single zone situated between two nodes. These nodes are the extremities of the RCHRES (Bicknell *et al.*, 1996: 128).

The flow through a RCHRES is uni-directional. Water and other constituents from other reaches and local sources enter the RCHRES through a single gate (INFLO). The outflow (OFLO) leaves the RCHRES through one of several gates or exits. A RCHRES can have up to five exits. Precipitation, evaporation and other fluxes also influence the processes, which occur in the RCHRES. These processes do not pass through the exits (Bicknell *et al.*, 1996: 128).

The HYDR section simulates the hydraulic processes occurring in a reach or a mixed reservoir. The final goals of the HYDR section is flood routing, study of reservoir behaviour and analysis of dissolved constituents. The input of water from precipitation and the loss of water by evaporation from the surface can also be considered (Bicknell *et al.*, 1996: 132). A flow diagram that illustrates the principal state variable (stored volume) and fluxes of the HYDR section is shown in Figure 3.3.



**Figure 3.3:** Flow diagram for the HYDR section of the RCHRES module (Bicknell *et al.*, 1996: 133)

### 3.4 MODEL CAPABILITIES AND STRATEGY

Some of the many capabilities available in the PERLND module include the simulation of:

- Water budget;
- Snow accumulation and melt;
- Sediment production and removal;
- Nitrogen and phosphorous behaviour;
- Pesticide behaviour and;
- Movement of tracer chemicals (Aqua Terra Consultants, 2002).

In this study, only the water budget capability was used.



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Some of the many processes that can be modelled in the RCHRES module, includes the following:

- ❑ Hydraulic behaviour and water temperature;
- ❑ Inorganic sediment deposition and transport by particle size;
- ❑ Chemical partitioning, hydrolysis, volatilisation, oxidation and biodegradation;
- ❑ DO and BOD balances;
- ❑ Inorganic nitrogen and phosphorous balances;
- ❑ Plankton populations and;
- ❑ Ph, CO<sub>2</sub>, total inorganic carbon and alkalinity (Aqua Terra Consultants, 2002).

In this study, only the hydraulic behaviour capability was used.

The development of a strategy or simulation plan implies the following steps, namely:

- ❑ Identifying all the relevant meteorological data;
- ❑ Developing a catchment discretisation scheme, referring to discretisation of all the homogeneous hydrological responses, soil characteristics and land-uses;
- ❑ Identifying the hydraulic and geometric characteristics of the river network;
- ❑ Examining of all available streamflow- and water quality data and;
- ❑ Devising a modelling strategy, which make use of all the data (Donigian *et al.*, 1984: 17).

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### 3.5 HSPF'S CONCEPTUALISATION OF THE HYDROLOGICAL CYCLE

In the HSPF model the hydrological cycle is simulated by a complex series of storages and pathways. HSPF's conceptualisation of the hydrological cycle and how the model depicts soil water budgeting is shown in Figure 3.4. The cycle is simulated as follows:

- Vegetation intercepts the precipitation. Only a part of the precipitation reaches the land surface, while the vegetation retains the other part. This interception storage (CEPS) is simulated as a reservoir, which must be filled before any precipitation can reach the land surface. The capacity of this CEPS reservoir varies, as the seasonal vegetation cover differs. The water in storage evaporates back into the atmosphere (Bicknell *et al.*, 1996: 56, 59).

- The other percentage of water, which reaches the land surface, is available as surface detention storage (SURS). This SURS is a temporary reservoir.

The water can either enter the upper zone as potential runoff or it can infiltrate to the subsurface. The distribution of potential runoff and infiltration is a function of the infiltration rate and antecedent soil moisture content. A high soil moisture content and low infiltration rate normally results in a higher percentage of runoff (Bicknell *et al.*, 1996: 56, 60).

- Potential direct runoff is the water available in the upper zone. This potential direct runoff can be sub-divided into direct surface-, interflow- and upper zone storage. The quantity of runoff for a specific time interval is a function of the catchment slope, roughness and distance to the main stream (Munson, 1998: 84).

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- Interflow runoff is also stored in a reservoir. This reservoir empties according to an active groundwater recession rate parameter (AGWRC) (1/day). The upper zone nominal storage (UZSN) represents depressions such as ditches on the catchment surface. Water in this storage can either evaporate or percolate to the subsurface or it may become runoff or interflow during the next time interval (Munson, 1998: 84).
  - The water is either routed to the lower zone storage (LZS), active groundwater storage (AGWS) or deep, inactive groundwater (IGWI) after infiltration. Firstly, the water is trapped in the LZS and it is based on the ratio water currently in storage to the nominal storage capacity. All the water in the lower zone nominal storage (LZSN) evaporates back to the atmosphere. After the LZS is satisfied, the remaining water is then divided between inactive and active groundwater. The groundwater specified as inactive is then lost from the cycle. AGWS is stored in a reservoir and then released to the baseflow (Bicknell *et al.*, 1996: 69-71).
  - In the HSPF model pan evaporation is normally multiplied by a factor ranging between 0.8 and 0.85 to convert it to daily total potential ET, since the pan evaporation data are normally too high (Wilson, 1990: 55). In order to meet the specific amount of ET in HSPF, the sequence of evaporation from the five possible storages is as follows: Baseflow, CEPS, upper zone storage (UZS), AGWRC and LZS. ET processes occur until all reservoirs are empty or the measured ET is satisfied (Munson, 1998: 84).



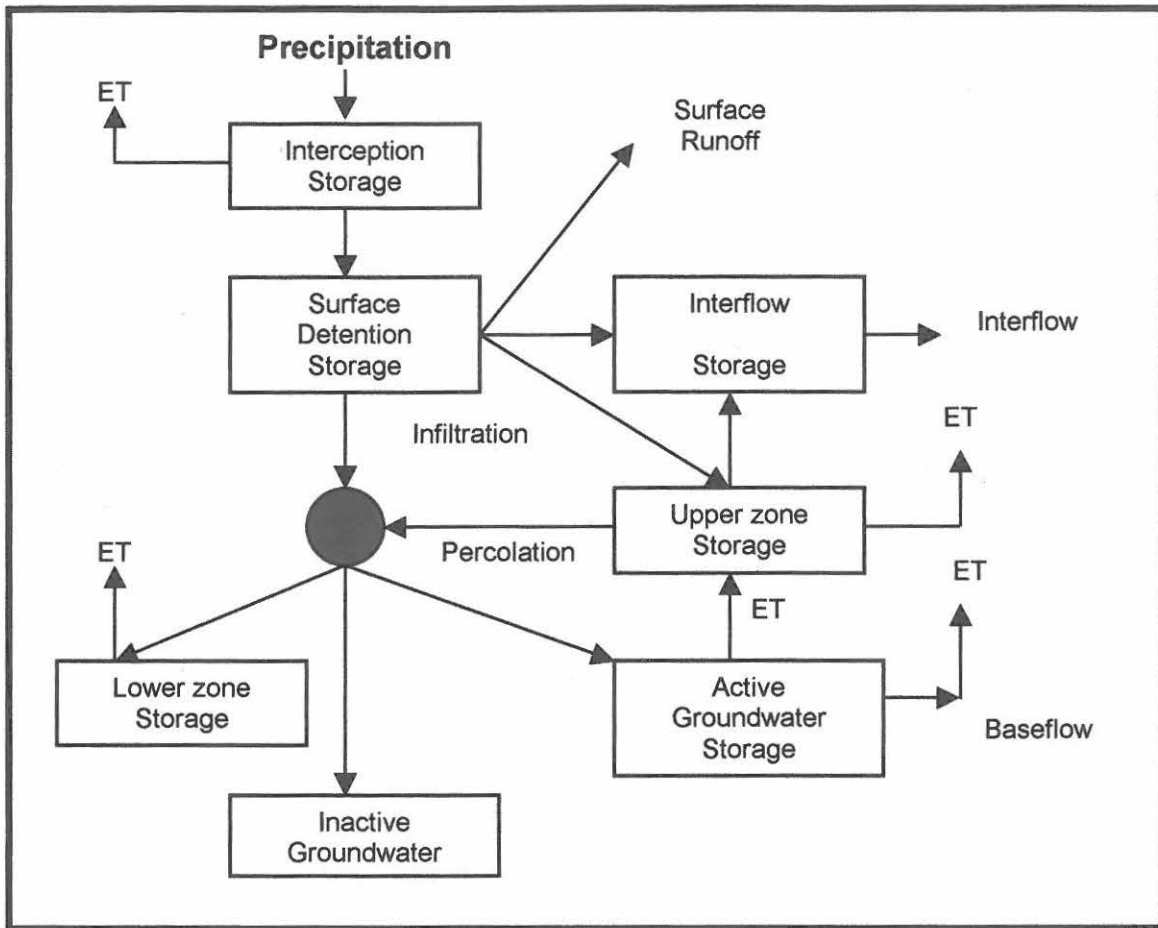


Figure 3.4: HSPF hydrological cycle (Munson, 1998: 83)

### 3.6 DATA REQUIREMENTS

HSPF requires extensive data to accurately simulate hydrological processes in a catchment. The amount and type of data required depends on what is to be simulated. Some input variables can be assumed to be constant, which will reduce the data requirements, but also the functionality of the simulation. The quality of a HSPF simulation will strongly depend on the quality of the input data.

Data requirements are grouped into three broad categories:

- 
- Catchment-specific information is necessary to accurately represent the catchment. These data include altitudes, average slopes and channel geometry, soil types, vegetation types and land-use (Munson, 1998: 35).

For the Msunduzi River Catchment, the impact of human water use must also be considered. Such information can be collected from topographical maps, field observations, GIS databases and historical records.

- HSPF requires meteorological data to drive the hydrological cycle during a simulation (Munson, 1998: 35). Data are collected from weather stations maintained by the South African Weather Service in and near the Msunduzi River Catchment. Meteorological records of precipitation and estimates of potential ET are required for catchment simulation. Surrogates of these meteorological data can be used (Dent, 2002). The monthly mean evaporation used in the Msunduzi River Catchment is a typical example.
- Physical measurements and related parameters are required to describe the land-use, rivers and reservoirs within a catchment (USGS, 2002). The successful application of HSPF in a catchment is influenced by the representation of the catchment's physical characteristics. The larger and more heterogeneous the catchment, the more difficult it becomes to collect a complete data set (Munson, 1998: 36).

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## 4. LITERATURE REVIEW: PARAMETERS, CALIBRATION AND MODEL PERFORMANCE

### 4.1 PARAMETER ESTIMATION

The overarching principle in parameter estimation should be that the estimated values must be realistic; therefore make hydrological sense and reflect the conditions on the catchment.

In the process of hydrological simulation there are two types of parameters, namely control-oriented and process-oriented parameters. Control-oriented parameters specify the period of simulation, the type of constituents to be modelled and the presenting of the simulation results. Process-oriented parameters are used to provide a mechanism to adjust the simulation for specific topographical-, hydrological-, land-use- and hydraulic conditions in a specific catchment (Donigian *et al.*, 1984: 77).

The values of physical parameters vary on a seasonal basis; therefore it is recommended that parameters such as precipitation interception, upper zone storage capacity, land surface roughness and ET be estimated on a monthly basis (Donigian *et al.*, 1984: 83).

#### 4.1.1 PWATER Parameters

Only the most important PWATER parameters are discussed in detail. All the other parameters that form a less important role in this hydrological modelling attempt or are used as default values are summarised in Table 4.5.



LZSN is the lower zone nominal soil moisture storage (mm). It is related to both precipitation patterns and soil characteristics in a catchment. LZSN is firstly an estimated value (normally between 130 and 380 mm) and then calibrated (USEPA, 2000: 9). According to Viessman, Lewis and Knapp (1989) initial estimates for LZSN as used in the Stanford Watershed Model is one-quarter of the mean annual precipitation plus 100 mm for arid and semi-arid regions or one-eighth of the mean annual precipitation plus 100 mm for coastal, humid or sub-humid climates. These formulae tend to give values somewhat higher than the typically final calibrated values. LZSN will be adjusted through an iterative calibration procedure.

INFILT is the index to mean soil infiltration rate (mm/hr). This parameter effectively controls the overall division of the available moisture from precipitation into surface-, subsurface flow and storage components. High values of INFILT will produce more water in the LZS and AGWS. This will result in higher baseflow to the stream. Low values of INFILT will produce more water in the UZS and interflow storage. This will result in greater direct overland flow and interflow. INFILT is firstly estimated, and then followed by an iterative calibration process. INFILT is primarily a function of soil characteristics (USEPA, 2000: 9-10). The value ranges have been related to SCS hydrological soil groups as depicted in Table 4.1.

**Table 4.1:** SCS hydrological INFILT estimations (Donigian & Davis, 1978: 61)

Soil group	Infiltration (mm/hr)	Runoff potential
A	10 - 25	Low
B	2.5 - 10	Moderate
C	1.25 - 2.5	Moderate to high
D	0.25 - 1.25	High

LSUR is the length of the assumed overland flow plane (m). It is an approximation of the average length of travel for water to reach a stream reach or any drainage path that quickly deliver the water to the stream or waterbody. LSUR can be estimated or measured from topographical data by dividing the catchment area by twice the length of all streams, gullies and ditches that move the water to the stream. That is, a representative straight-line reach with length  $L$ , bisecting a representative square areal segment of the catchment. This will produce two overland flow planes of width  $0.5 L$ . LSUR values derived from topographical data are often over-estimated when the data are of insufficient resolution to display the many small streams (USEPA, 2000: 11).

SLSUR is the average slope of assumed overland flow path (m/m). GIS capabilities can be used to estimate average SLSUR values for each land-use being simulated. Graphical techniques whereby a grid pattern is imposed on the catchment and then the slope values for each grid point for each land-use are calculated, can also be implemented (USEPA, 2000: 11).

KVARY is the groundwater recession flow parameter used to describe the non-linear groundwater recession rate (/mm). It is usually one of the last PWATER parameters to be adjusted. It is normally used when the observed groundwater recession demonstrates a seasonal variability with a faster recession during wet periods (USEPA, 2000: 11).

AGWRC is the ratio of current groundwater discharge to that from 24 hours earlier, when KVARY (groundwater recession rate) (1/day) is zero (USEPA, 2000: 12).

In other words, it is the ratio of today's flow divided by yesterday's flow. HSPF assumes the groundwater reservoir is "linear". This ratio should be constant and a semi-logarithmic plot of groundwater flow should be a straight line, because the flow is decreasing exponentially (Johanson, 1989). AGWRC is first estimated and then calibrated (USEPA, 2000: 12).

DEEPFR is the fraction of infiltrating water, which is lost to deep aquifers (inactive groundwater). The remaining fraction (1-DEEPFR) is assigned to active groundwater storage that contributes baseflow to a stream. It also represents any other losses that may not be measured at the hydrological gauging station used for calibration. Flow around or under the gauge site, or underlying dolomitic geology are typical examples. This accounts for one of only three major losses from the PWATER balance. The other two losses are ET and lateral stream outflows. Initially DEEPFR is set to zero or estimated based on groundwater studies. It is then calibrated in conjunction with adjustments to ET parameters to achieve a reasonable annual water balance (USEPA, 2000: 13).

CEPSC is the amount of precipitation (mm), which is retained by vegetation and never reaches the land surface and is eventually evaporated (USEPA, 2000: 14). Typical values for CEPSC for selected land-uses are shown in Table 4.2.

**Table 4.2:** Maximum interception versus land-use (Donigian & Davis, 1978: 54)

Land-use	Maximum interception (mm)/day
Grassland	2.54
Cropland	2.54-6.35
Forest cover, light	3.81
Forest cover, heavy	5.08



As part of an annual water balance, it must be noted that 10-20% of precipitation during the growing season is intercepted. As much as 25% of total annual precipitation is intercepted under dense closed forests. Crops and grassland exhibit a wide range of interception rates, between 7% and 60% of the total precipitation (USEPA, 2000: 15).

UZSN is the nominal upper zone soil moisture storage (mm). It is related to land-use characteristics, topography and LZSN. UZSN may change due to variations in agricultural conditions, tillage and other practices over the course of the growing season. Increasing UZSN values increase the amount of water retained in the upper zone and that available for ET. It thereby decreases the dynamic behaviour of the surface and reduces direct overland flow. Decreasing UZSN has the opposite effect (USEPA, 2000: 15).

Initial estimates for UZSN of 0.06 LZSN (steep slopes, limited vegetation and low depression storage), 0.08 LZSN (moderate slopes, moderate vegetation and moderate depression storage) and 0.14 LZSN (heavy forest cover, soils subject to cracking, high depression storage and very mild slopes) can be used (Donigian & Davis, 1978: 54).

NSUR is Manning's  $n$  for an overland flow plane. Manning's  $n$  values for overland flow are considerably higher than the more common published values (USEPA, 2000: 15). According to Hwang and Hita (1987)  $n$  values for flow through a channel range from a low of about 0.011 for smooth concrete, to as high as 0.050-0.10 for flow through unmaintained channels.

The tabulated values for the following different land-use conditions are shown in Table 4.3.

**Table 4.3:** Manning's  $n$  value versus land-use (Donigian & Davis, 1978: 61)

Land-use	Manning's ( $n$ )
Smooth packed surface	0.05
Normal roads and parking lots	0.10
Disturbed land surfaces	0.15-0.25
Moderate turf/pasture	0.20-0.30
Heavy turf, forest litter	0.30-0.45
Conventional tillage	0.15-0.25
Smooth fallow	0.15-0.20
Rough fallow, cultivated	0.20-0.30
Crop residues	0.25-0.35
Meadow, heavy turf	0.30-0.40

INTFW is the coefficient that determines the amount of water that enters the ground from surface detention storage and becomes interflow, in contrast to direct overland flow and upper zone storage. Interflow can have an important influence on storm hydrographs, particularly when shallow, less permeable soils delay vertical percolation. The timing of runoff is affected by INTFW, due to the division of water between interflow and surface processes (USEPA, 2000: 15). Increasing INTFW increases the amount of interflow and decreases direct overland flow. Therefore peak flows are reduced, while the same volume is maintained. The shape of the hydrograph is then affected, by shifting and delaying the flow to later in time (USEPA, 2000: 16).

IRC is the interflow recession coefficient. It is analogous to the groundwater recession parameter, AGWRC. IRC affects the rate at which interflow is

discharged from storage. It also affects the hydrograph shape in the recession limb of the curve. The maximum value range is from 0.3 to 0.85, with lower values on steeper slopes. High values of IRC will make interflow behave more like baseflow. Low values will make interflow behave more like overland flow (USEPA, 2000: 16). Typical values are between 0.2 and 0.4 (Johanson, 1989).

LZETP is the index to lower zone evapotranspiration. It is a coefficient used to define the ET opportunity. It affects ET from the lower zone, which represents the primary soil moisture storage and root zone of the soil profile. LZETP behaves much like a "crop coefficient" with values mostly in the range of 0.2 to 0.7. It is primarily a function of the different land-uses and vegetation (USEPA, 2000: 17). Typical LZETP values for the different land-uses are listed in Table 4.4.

**Table 4.4:** Typical LZETP values versus land-use

Land-use	LZETP
Forest	0.6-0.8
Grassland	0.4-0.6
Row crops	0.5-0.7
Barren	0.1-0.4
Wetlands	0.6-0.9



**Table 4.5:** HSPF pervious hydrology parameters and value ranges (USEPA, 2000: 30)

Parameter	Unit	Range of values		Comment (Function of)
		Typical	Possible	
PWAT-PARM 2				
LZSN	mm	76-200	50-380	Calibration (Soil and climate)
INFILT	mm/h	0.25-6.4	0.025-2.7	Calibration, divides surface and sub-surface flow (Soil and land-use)
LSUR	m	60-150	30-215	Estimate from high resolution topographical maps or GIS (Topography)
SLSUR	m/m	0.01-0.15	0.001-0.3	Estimate from high resolution topographical maps or GIS (Topography)
KVARY	1/mm	0.0-76	0.0-127	Used when recession rate varies with groundwater levels (Baseflow recession variation)
AGWRC	none	0.92-0.99	0.85-0.99	Calibration (Baseflow recession)
PWAT-PARM 3				
INFEXP	none	2	1-3	Exponent in infiltration equation Usually default of 2.0 (Soil variability)
INFILD	none	2	1-3	Ratios of max/ mean infiltration capacities. Default of 2.0 (Soil variability)
DEEPPFR	none	0-0.2	0-0.5	Accounts for subsurface losses (Geology and groundwater recharge)
BASETP	none	0-0.05	0-0.2	Fraction of remaining ET from baseflow, thus direct ET from vegetation (Riparian vegetation)
AGWETP	none	0-0.05	0-0.2	Fraction of remaining ET from groundwater, thus direct ET from shallow groundwater (Marsh or wetland extent)
CEPSC	mm	0.76-5	0.25-10	Monthly values usually used (Vegetation and land-use)

**Table 4.5 (continued):** HSPF pervious hydrology parameters and value ranges (USEPA, 2000: 30)

Parameter	Unit	Range of values		Comment (Function of)
		Typical	Possible	
PWAT-PARM 4				
UZSN	mm	2.5-25	1.3-50	Accounts for near surface retention (Surface soil conditions and land-use)
NSUR	none	0.15-0.35	0.05-0.5	Monthly values often used for croplands (Surface conditions and residue)
INTFW	none	1-3	1-10	Calibration, based on hydrograph separation (Soil, topography and land-use)
IRC	none	0.5-0.7	0.3-0.85	Often start with a value of 0.7 and then adjust (Soil, topography and land-use)
LZETP	none	0.2-0.7	0.1-0.9	Calibration (Vegetation type, density and root depth)

#### 4.1.2 IWATER Parameters

LSUR is the length of the assumed overland flow plane (m). LSUR reflects the overland flow length on directly connected or effective impervious area. It is usually in the range of 15 – 76 m, although longer lengths may apply in commercial or industrial areas (USEPA, 2000: 20).

RETSC is the retention/ interception storage (mm) of the impervious surface, thus the depth of water that collects on the impervious surface before any runoff occurs. It is the impervious equivalent of the interception storage variable (CEPSC) used for pervious land segments. According to Dinicola (1990) an initial value of 2.5 mm for RETSC is appropriate. During detention storage design of parking lots and rooftops, larger values of up to 13 mm may be reasonable (USEPA, 2000: 21).

**Table 4.6:** HSPF impervious hydrology parameters and value ranges (USEPA, 2000: 31)

Parameter	Unit	Range of values		Comment (Function of)
		Typical	Possible	
IWAT-PARM 2				
LSUR	m	15-46	15-76	Estimate from maps, GIS or field survey (Topography and drainage)
SLSUR	m/m	0.01-0.05	0.001-0.15	Estimate from maps, GIS or field survey (Topography and drainage)
NSUR	none	0.03-0.1	0.01-0.15	Typical range is 0.05-0.10 for roads or parking lots (Impervious surface conditions)
RETSC	mm	0.8-2.5	0.25-8	Typical range is 0.76-2.54 for roads or parking lots (Impervious surface conditions)

#### 4.1.3 HYDR Parameters (Flow routing)

Only the most important HYDR parameters are discussed in detail. All the other parameters and functions are summarised in Table 4.7.

HSPF computes streamflow through a stream reach or reservoir based on two assumptions:

- A fixed relationship exists between depth, volume and discharge and;
- Discharge is a function of volume (USEPA, 2000: 23).

Flow reversals and backwater effects in an upstream reach are therefore not simulated. Routing is computed using the techniques of storage routing or kinematic wave routing. Momentum is not considered in the routing computations (USEPA, 2000: 23).



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LEN is the length of the stream reach (km). It is used in the computation of parameter values (USEPA, 2000: 24).

DELTH is the change in elevation from the upstream end of the river reach to the downstream end (m) (USEPA, 2000: 24).

KS is a weighting factor applied in the computation of the river reach outflow. A KS value of 0.5 is recommended (USEPA, 2000: 24).

CRRAT is the ratio of the maximum velocity to the mean velocity in the stream channel cross-section under typical flow conditions. It must be 1.0 or greater. A value of 1.0 corresponds to completely uniform velocity across the stream channel. It is used to determine the relative volumes of water stored in the stream reach versus that leaving the reach in a given time interval. The outflow is assumed to be in part, made up of water that entered the reach in that same interval, when CRRAT is greater than the volume: outflow ratio. The inflow constituent concentration then alters and influences the outflow constituent concentration (USEPA, 2000: 26).

**Table 4.7:** HSPF hydraulic parameters and value ranges (USEPA, 2000: 32)

Parameter	Unit	Range of values		Comment (Function of)
		Typical	Possible	
HYDR-PARM 2				
FTBDSN	none	none	1-999	Used only if FTABLE is in WDM file (WDM file)
FTABNO	none	none	1-999	Used only if FTABLE is in UCI file (RCHRES block or reach numbering)
LEN	km	0.161-1.61	0.016-161	Used only in computing auxiliary parameters (Topography and river morphology)
DELTH	m	3.05-30.48	0.031-305	Used only for water quality and sediment (Topography and river morphology)
STCOR	m	0-none	0-none	Stage correction factor. Dependent on elevation datum used (Topography)
KS	none	0-0.5	0-0.99	Use KS = 0.5 (River slope and flow obstructions)
DB50	mm	0.25-0.51	0.025-25.4	Bed sediment diameter. Used only in sediment calculations (River bed properties)
ADCALC-DATA				
CRRAT	none	1.5-2	1-3.5	Only used with water quality (Climate and vegetation)
VOL	M.m <sup>3</sup>	0-none	0-none	Initial volume in river reach (Season, river geometry and climate)

## 4.2 HSPF: CALIBRATION

Calibration is a process whereby the parameters in a simulation model are adjusted in a certain range until the differences between the model simulated values and observed data are within selected criteria for performance. The period of simulation must not only be based on the availability of meteorological data, but also on the availability of instream quantity- and quality data (Donigian *et al.*, 1984: 3, 51).

Calibration is an iterative procedure. Parameter evaluation and refinement is part of the comparison between simulated values and observed data. Calibration is applicable to non-deterministic parameters, which are parameters that cannot be evaluated from topographical-, climatic-, physical- and chemical characteristics. Calibration should be based on several years of simulation, preferably three to five years as a minimum. The longer the period of simulation, the better, because then all the changes in climate, soil moisture and water quality conditions can be evaluated. The areal variability of precipitation and ET can cause additional uncertainty in the process of simulation. Calibration must be successful for dry- and wet periods (Donigian *et al.*, 1984: 84).

In the case of hydraulic calibration, particular attention should be given to items such as approximations of river geometry, river roughness coefficients and the interpretation and extrapolation of stage versus discharge data (Donigian *et al.*, 1984: 94).

In general, calibration results for hydrology and hydraulics, must have a percent variation of between 0 – 25%. Percentage variations less than 10% are above good, between 10 – 15% good and between 15 – 25% fair. Verification complements calibration and is an independent test of how well a model represents the important processes occurring in a catchment (Donigian *et al.*, 1984: 3, 51, 114).

#### **4.2.1 Hydrological Calibration (Streamflow)**

A large number of factors control the hydrological cycle. During the process of calibration it is difficult to alter the correct factor in order to improve the



simulation, especially when taken into consideration that these factors are non-linear (Donigian *et al.*, 1984: 89).

The starting conditions for an HSPF simulation are determined as follows: The surface related storages of PERLND (CEPS, SURS and IFWS) and IMPLND (RETS and SURS) are initially set to zero, because individual storms are not being simulated (Donigian *et al.*, 1984: 92). The soil storages, LZS and UZS are the initial moisture storages in PERLND and are set equal to LZSN and UZSN, except when the simulation starts in a very dry- or wet period. In very wet periods, the actual quantities stored in LZS and UZS can go higher than the nominal storage capacities of LZSN and UZSN. In dry periods between storms, these storages can drop well below the nominal storages due to ET (Johanson, 1997). The factors describing the physical characteristics of the catchment, which are considered constant, are identified and suitable parameter values chosen.

After determining the starting conditions, HSPF calibration proceeds in the following sequence:

- An overall water balance over several simulation years is obtained by adjusting the parameters, which affect evaporation and runoff, mainly LZSN and LZETP. The latter is largely determined by land-use, and is set to a constant value at the start of calibration, and only minor adjustments are made to it. LZSN is then adjusted so that simulated runoff, over a long period of time, is approximately equal to the observed runoff.
- The wet versus the dry seasonal water balance is calibrated to ensure the correct distribution of seasonal streamflow. The most important parameter

influencing seasonal distribution of runoff is INFILT. If wet season runoff is too high and dry season baseflow too low, INFILT is increased to allow more precipitation to infiltrate to the groundwater zone. Baseflow recession rates are adjusted with AGWRC.

- Finally, hydrographs for individual storm events are calibrated. Simulated streamflow peaks and slopes of the hydrograph limbs are adjusted to match the observed data. The most important parameters influencing hydrograph shape are AGWRC, INTFW, UZSN and IRC (Donigian *et al.*, 1984: 89-93 & Johanson, 1997).

During the process of HSPF calibration the main aim is to minimise the root-mean-square-errors (*RMSE*) and maximise the coefficient of determination ( $r^2$  values).  $r^2$  Values are the most widely published; therefore the daily, monthly and annual streamflow  $r^2$  values can be compared with those of other catchments or other models. According to several publications on HSPF calibration a good calibration has an  $r^2$  value of 0.9, at the annual level, 0.8 seasonally and 0.6 daily (Munson, 1998: 86).

#### 4.2.1.1 Water Balance (Annual and Seasonal)

The establishment of a water balance on an annual basis is the first step of hydrological calibration. The water balance can be defined as follows:

$$\text{Runoff} = \text{Precipitation} - \text{Actual ET} - \text{Deep percolation} - \Delta \text{ Soil moisture storage} \quad (1)$$

The annual water balance is determined by four parameters, namely INFILT, LZSN, LZETP and to a lesser extent, UZSN.

The input meteorological data series, as well as the parameters such as LZSN, INFILT and LZETP control the water balance. Actual ET must be adjusted to produce a change in the long-term runoff component of the water balance when precipitation is high and percolation to groundwater is low. LZSN and INFILT have a significant impact on percolation and therefore also on the annual water balance (Donigian *et al.*, 1984: 90).

LZSN represents water, which is stored in the subsurface zone and evaporates to the atmosphere. LZSN is the primary parameter when the annual water balance is adjusted. It is also mainly responsible for most of the evaporation from the catchment (Donigian *et al.*, 1984: 90).

Hydrocomp Incorporated recommends an initial value of:

$$LZSN = 0.25 \cdot P + 100 \quad (2)$$

where: P = Total annual precipitation (mm) (Munson, 1998: 95).

LZETP controls the rate of evaporation from the lower zone, which is influenced by the fraction of the catchment area covered by deep-rooted vegetation.

UZSN is the water trapped on the catchment's surface and therefore it is a function of the topography. The type of vegetation, slope and soil types influences the amount of surface storage (Donigian *et al.*, 1984: 90).



#### 4.2.1.2 Hydrograph Calibration

Adjusting the shape of individual storm hydrographs is the final stage of hydrological calibration. HSPF parameters can be used to adjust the rising- and descending limb, as well as the streamflow peak. INTFW and IRC are the most important parameters for adjusting the shape of storm hydrographs. UZSN has significant influence on the magnitude of streamflow peaks, especially on small catchments, as well as on the timing of the ascending limb. Increasing INTFW will reduce peak flows and prolong recession of the hydrograph (Donigian *et al.*, 1984: 92-93).

Munson (1998) found that HSPF simulated the timing of hydrographs correctly, but in some cases it under-estimated the volume, thus the runoff. Streamflow peaks of baseflow were also sometimes under-estimated. The recession rate of large hydrographs was frequently simulated incorrectly. Under-simulation of winter storms, where applicable, lead to the under-estimation of spring recharge and summer baseflow. Human induced hydrological changes may also effect the correlation between simulated- and observed hydrographs.

#### 4.2.2 Non-Calibration Model Parameters

Non-calibration parameters are the physical parameters, which are constant according to the land-use and do not change during hydrological calibration. Typical parameters are latitude, elevation and vegetation type. The parameters NSUR, SLSUR and LSUR are used by the HSPF model to route surface runoff. However, studies have shown that HSPF is largely insensitive to these parameters (Munson, 1998: 90).

Impervious land influences the response time of the catchment. In urban areas where the runoff contributes a significant amount of stormwater and pollutants, impervious land plays an important role. High percentages of impervious land are associated with higher amounts of runoff and increased pollutants (Munson, 1998: 91).

#### 4.3 MODEL PERFORMANCE

The visual comparison of simulation results against observed data can be highly subjective. Statistical model performance measures are therefore used to evaluate a hydrological model's "performance" against certain predetermined statistical criteria of goodness-of-fit in the reproduction of observed data (Schulze, 1995: AT 21.1). According to Roberts (1987) this goodness-of-fit criteria are objective mathematical functions, which express the most desirable characteristics between a model's simulated output and the real world observations.

According to Willmott (1982) and Schulze (1995) the range of goodness-of-fit statistics applicable to hydrological modelling, falls into two major categories, namely:

- Conservation statistics (Conservation of the mean, standard deviation and *RMSE*) and;
- Regression statistics (Slope, *y*-intercept and the coefficient's of determination and efficiency).

The HSPF model's simulated results are evaluated by making use of the above-mentioned conservation- and regression statistics. These results, as well as the scatter plots of the linear regression analyses are discussed in Chapter 6.

In the various examples of statistics of model performance, the following symbols are applicable:

$x_i$	= Observed time series.
$\bar{x}$	= Mean value of the observed time series.
$\hat{y}$	= Estimated value from the regression line of $y$ on $x$ .
$\hat{y}_i$	= $a + bx_i$
$y_i$	= Simulated time series.
$s$	= Standard deviation.
$n$	= Sample size of the time series (For $i$ from 1 to $n$ , depending on the time step of output selected).

#### 4.3.1 Conservation Statistics

*Conservation of the mean:* This is a measure of central tendency of the data.

$$\bar{x} = \frac{1}{n} \sum_{i=1}^n x_i \quad (3)$$

The objective is to minimise the percentage difference between the means of observed data ( $x$ ) and simulated values ( $y$ ).

$$100 \cdot \frac{(\bar{x} - \bar{y})}{\bar{x}} \quad (4)$$



**Standard deviation:** This is a measure of dispersion about the mean in original units. The observed data ( $x$ ) and simulated values ( $y$ ) are defined by the following equations:

$$s_x = \sqrt{V_x} \quad (5)$$

$$s_y = \sqrt{V_y} \quad (6)$$

The objective is to minimise the percentage difference between the standard deviations of observed data ( $x$ ) and simulated values ( $y$ ):

$$100 \cdot \frac{(s_x - s_y)}{s_x} \quad (7)$$

**Root-Mean-Square-Error:** This statistic defines the actual size of error produced by the model. It does not indicate the source and type of error. The objective is to minimise the *RMSE*. It is defined by the following equation:

$$RMSE = \sqrt{\frac{\sum_{i=1}^n (x_i - y_i)^2}{n}} \quad (8)$$

#### 4.3.2 Regression Statistics

**Slope:** It is the slope ( $b$ ) of the least-squares regression line. This line denotes the relative change of simulated ( $y$ )- to observed ( $x$ ) trends.

$$b = \frac{n \sum_{i=1}^n x_i y_i - \sum_{i=1}^n x_i \sum_{i=1}^n y_i}{n \sum_{i=1}^n x_i^2 - \left( \sum_{i=1}^n x_i \right)^2} \quad (9)$$

The objective is to attain a slope as closely as possible to unity (1.0). Slope values greater than unity (>1.0) indicates over-simulation at the upper end of the simulated values. Slope values less than unity (<1.0) indicates under-simulation.

*Base Constant (y-intercept):* This is the point where the line crosses the y-axis. A positive y-intercept indicates over-simulation of low values, while a negative y-intercept indicates under-simulation of low values.

$$a = \frac{\sum_{i=1}^n y_i - b \sum_{i=1}^n x_i}{n} \quad (10)$$

The objective is to minimise the base constant (y-intercept) to zero.

*Coefficient of determination:* It measures the degree of association between the simulated values (y) and the estimated values as predicted by the regression model.

$$r^2 = \frac{\sum_{i=1}^n (y_i - \bar{y})^2 - \sum_{i=1}^n (y_i - \hat{y})^2}{\sum_{i=1}^n (y_i - \bar{y})^2} \quad (11)$$

The objective is to maximise the coefficient of determination to unity (1.0). High  $r^2$  values indicate a good degree of association.

*Coefficient of efficiency:* This coefficient measures the degree of association between observed data (x) and simulated values (y). It can be used to quantify or indicate model bias by determining the difference between the coefficients of

determination and efficiency. It is identical to the coefficient of determination; except that the estimated simulated values replace the observed data.

$$E_c = \frac{\sum_{i=1}^n (y_i - \bar{y})^2 - \sum_{i=1}^n (y_i - x_i)^2}{\sum_{i=1}^n (y_i - \bar{y})^2} \quad (12)$$

The objective is to maximise the coefficient of efficiency to the value of the coefficient of determination.



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## **STUDY AREA & DATA DEVELOPMENT**

### **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

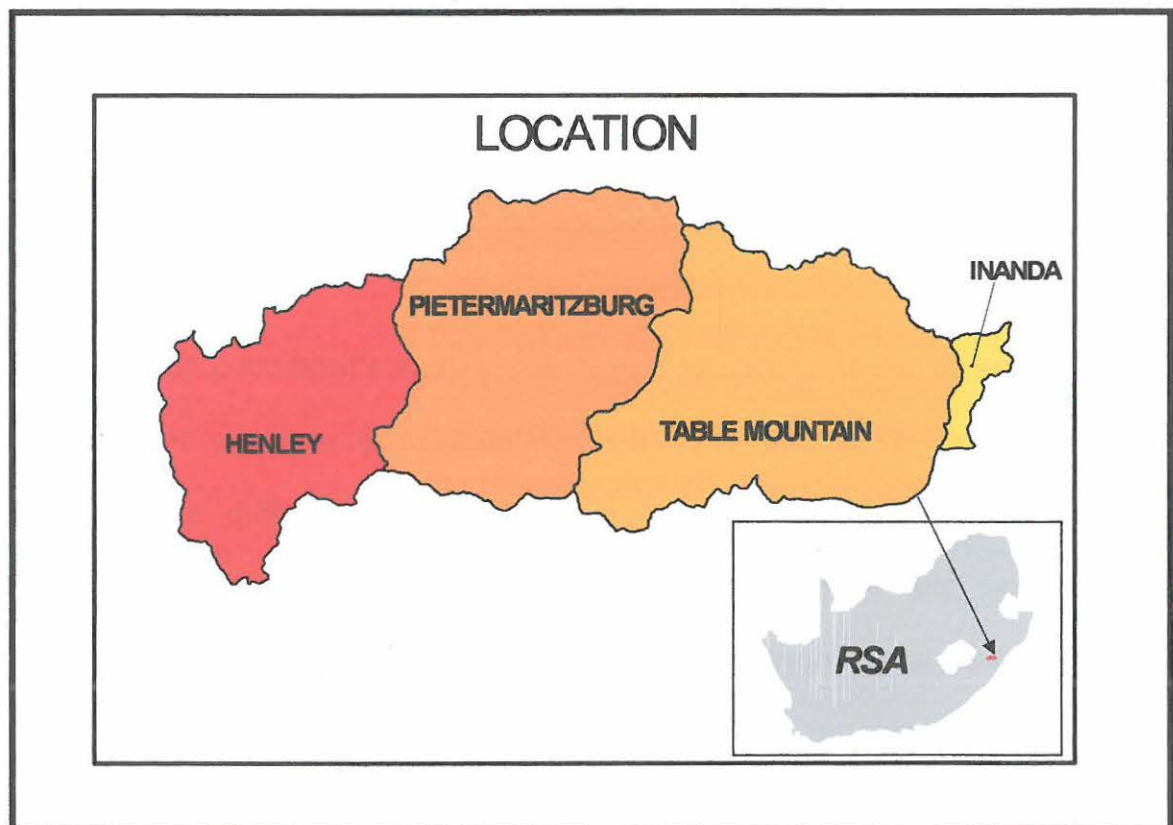
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## 5. STUDY AREA AND DATA DEVELOPMENT

### 5.1 CATCHMENT DATA

#### 5.1.1 Location

The Msunduzi River Catchment covers an area of 901 km<sup>2</sup>. It is located between the latitudes 29°32' and 29°47' south, and longitudes 30°05' and 30°41' east, in the province of Kwa-Zulu Natal near the east coast of the Republic of South Africa (Figure 5.1).



**Figure 5.1:** Location of the Msunduzi River Catchment

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### 5.1.2 Topography

Soil surveys provide a general overview of the catchment's topography. The topography of the Msunduzi River Catchment is mountainous and undulating with numerous hills, as well as lowlands, wetlands, marshes and relatively flatter areas. The mean altitude above sea level in the catchment varies from 285 m to 1520 m. The average slope of the catchment varies from 2.5 to 33%.

### 5.1.3 Geological Data

Soil maps are extremely useful. However, soil types can be highly variable, especially in the rapidly developing urban areas of the Msunduzi River Catchment where soils may be removed or otherwise disturbed by human activity. The disturbed urban areas have highly variably runoff rates in most soil types, even after vegetation has been re-established.

Inspection by pedologist's and infiltration studies may be useful to verify the soil types related to a specific catchment (Brach, 2000: 8.20-3, 4). Pedologists view soils from a different perspective than hydrologists. They are specifically interested in the soil classification, agricultural potential and –constraints. Hydrologists on the other hand, are interested in the hydrological attributes of the soils (Tarboton & Schulze, 1992: 59).

The soil surveys of the Msunduzi River Catchment contain detailed aerial maps with the boundaries of the geological soil groups delineated on them. At least six distinct geological soil groups, which include Adelaide (mud- and sandstone), Dwyka (dyamictite, tillite, shale, mud- and sandstone), Ecca (sandstone, ecca-,



coal- and carbonaceous shale), Natal (quartzitic sandstone, arkose and shale) and Tugela and Mapumulo (amphibolite, gneiss, schist and granulite) soils were identified in the catchment (Tarboton & Schulze, 1992: 61). The distribution of these geological soil groups is listed in Table 5.1.

**Table 5.1:** Distribution of geological soil groups

Geological soil group	Area (km <sup>2</sup> )	Percentage-distribution (%)
Adelaide	31	3
Ecca	545	60
Dwyka	169	19
Natal	6	1
Tugela and Mapumulo	150	17
<b>Total:</b>	<b>901</b>	<b>100</b>

West of Pietermaritzburg the shales of the Ecca- and Beaufort groups predominate. Further inland soils are weakly developed, with lithocutanic subsoil horizons. It consists of either red- and black clays or duplex and plinthic soils. At the higher altitudes of Pietermaritzburg well-drained yellow and red dystrophic or mesotrophic soils prevail. These soils vary widely in texture, drainage and depth (Tarboton & Schulze, 1992: 1).

The composition of the diagnostic sub-soil horizons and descriptive soil forms are summarised in Table 5.2.

**Table 5.2:** Composition of diagnostic sub-soil horizons  
(Tarboton & Schulze, 1992: 61)

Geological soil group	Diagnostic sub-soil horizon	Soil form composition / description
Adelaide	Red-yellow apedal, freely drained soils.	<b>Ac</b> Red and yellow dystrophic and/ or mesotrophic.
Ecca	Red-yellow apedal, freely drained soils.  Plinthic Catena: Upland duplex and Margalitic soils, rare.  Glenrosa and/or Mispah forms.	<b>Ab</b> Red, dystrophic and/ or mesotrophic.  <b>Ah</b> Red and yellow, high base status, usually <15% clay.  <b>Bb</b> Dystrophic and/ or mesotrophic, red soils not widely spread.  <b>Fa/ b</b> Lime rare or absent in landscape or upland soils, but generally present in low-lying soils.
Dwyka	Red-yellow apedal, freely drained soils.  Glenrosa and/or Mispah forms.	<b>Ab</b> Red, dystrophic and/ or mesotrophic.  <b>Ah</b> Red and yellow, high base status, usually <15% clay.  <b>Fa</b> Lime rare or absent in entire landscape.
Natal	Glenrosa and/or Mispah forms.	<b>Fa</b> Lime rare or absent in entire landscape.
Tugela and Mapumulo	Red-yellow apedal, freely drained soils.  Glenrosa and/or Mispah forms.	<b>Ab</b> Red, dystrophic and/ or mesotrophic.  <b>Fa</b> Lime rare or absent in entire landscape.

Soil type is the most important determinant of the infiltration rate. Different soil types have different infiltration characteristics. Fine-textured soils, such as clay, generally produce a higher rate of runoff than coarse-textured soils, such as sand. The higher the infiltration rates; the lower is the quantity of surface runoff.

The infiltration rate, which is controlled by surface conditions, is the rate at which water enters the soil at the soil surface. The transmission rate is the rate at which the water moves within the soil. This rate is controlled by the soil profile.

It is thus important to have a good understanding of the soils present in the Msunduzi River Catchment, as well as their association with the different hydrological groups. Each soil type is assigned to one of the four hydrological groups:

- ❑ **Group A:** These soils have a low runoff potential and high infiltration rates even when completely wetted. They consist mainly of deep, well to excessively drained sands or gravel and have a high rate of water transmission (>7.6 mm per hour).
- ❑ **Group B:** These soils have moderate infiltration rates when completely wetted. They consist mainly of moderately deep to deep, moderately well to well-drained soils with reasonably fine to reasonably coarse textures. These soils have an average rate of water transmission (3.8-7.6 mm per hour).
- ❑ **Group C:** These soils have low infiltration rates when completely wetted. They consist mainly of soils with a layer that impedes downward movement of water and soils with reasonably fine to fine texture. These soils have a low rate of water transmission (1.3-3.8 mm per hour).
- ❑ **Group D:** These soils have a high runoff potential. They have very low infiltration rates when completely wetted and consist mainly of clay soils with a high swelling potential, permanent high water table and claypan or clay layer at or near the surface. These soils have a very low rate of water transmission (0-1.3 mm per hour) (Brach, 2000: 8.20-3).



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#### 5.1.4 Land-use and Vegetation

The land-use within the Msunduzi River Catchment has different hydrological characteristics and therefore it can also be associated with different meteorological data.

Land-use can be determined by satellite images, aerial photographs, topographical maps and field visits. The hydrological properties of each land-use can be determined by a number of variables such as crop coefficients, canopy interception and root system activities in the topsoil. These variables can be calculated for each catchment by superimposing the land-use with maps and area-weighting the relevant values (Kienzle *et al.*, 1997: 15).

The type of land-use and its condition affects runoff volume through its influence on the infiltration rate of the soil. Vegetation maintains the soil's infiltration potential by preventing the sealing of the soil surface by the impact of the precipitation. Some of the raindrops are also retained on the surface of the vegetation. This would then increase the chance of the raindrops being evaporated back to the atmosphere. Some of the intercepted moisture takes so long to drain down to the soil that it is withheld from the initial period of runoff. Foliage also transpires moisture into the atmosphere, thereby creating a moisture deficiency in the soil. This deficiency must be replaced by precipitation before runoff can occur (Brach, 2000: 8.20-4).

The natural vegetation in the Msunduzi River Catchment consists of Valley bushveld and thornveld in the lowlands and Ngongoni veld (Zululand) in the hinterland (Acocks, 1988).

The bioclimate in both these regions can be described as humid to sub-humid (Tarboton & Schulze, 1992: 1). Further inland temperate and transitional forest and scrub of the highland and Dohne sourveld, Southern tall grassveld and Ngongoni veld (Natal mist belt) types are found in a region with a sub-humid to mild sub-arid bioclimate (Acocks, 1988). Natural vegetation has been replaced in many areas by afforestation and agricultural cropping (Tarboton & Schulze, 1992: 1).

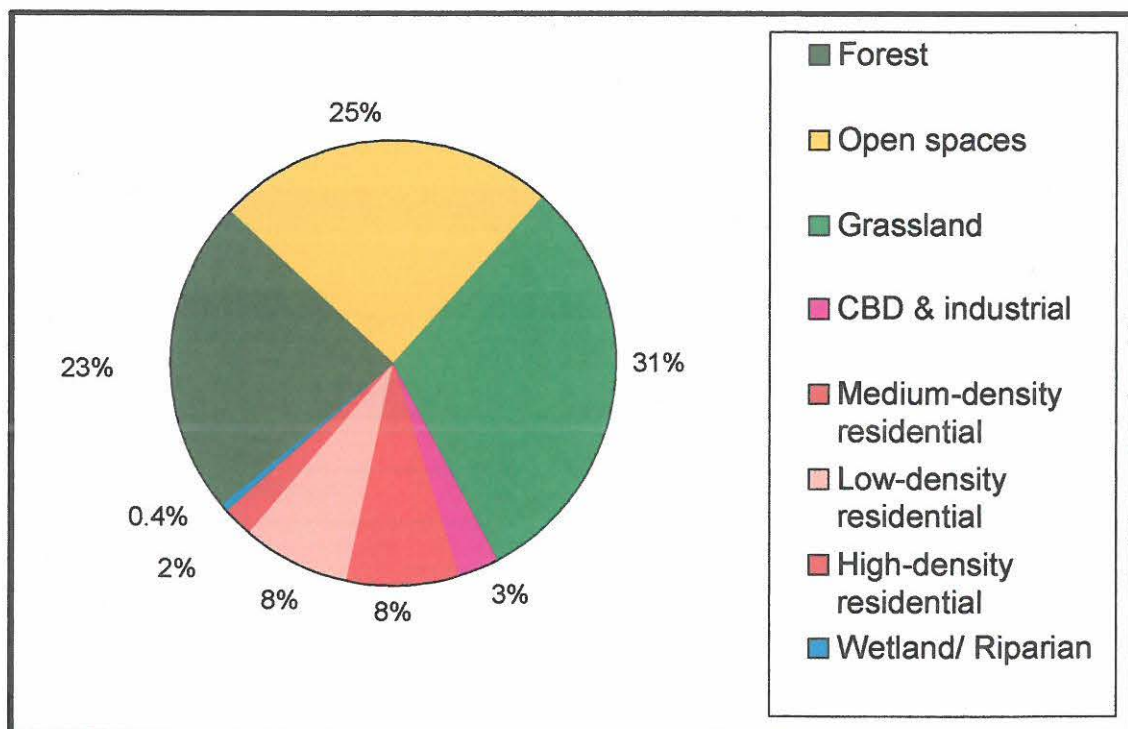
Although many people consider the Msunduzi as an urban river, this is only true in the greater Pietermaritzburg area. Overall, the amount of residential land ( $\pm 189 \text{ km}^2$ ) is smaller than the amount of grassland ( $\pm 276 \text{ km}^2$ ), reflecting the rural nature of the upper catchment.

The Msunduzi River Catchment is divided into eight main land-use groups. It can be categorised as follows:

- **Forest (1):** This includes Eucalyptus-, indigenous-, mixed or undefined-, other- and pine forest, as well as woodlands and wattles.
- **Open spaces (2):** This includes cropland (irrigated pastures, low-density smallholdings, maize, sugar cane, and undefined cropping), participation recreation (Parks: sport and fields), urban open space, savannah veld and Valley of Thousand Hills.
- **Grassland (3):** This includes undefined open spaces and normal grassland.
- **Central Business District (CBD) and industrial (4):** This includes the CBD's of the main cities or towns and all the industrial areas.

- **Medium-density residential (5):** This includes the transfer between rural and urban areas and residential areas with few trees.
- **Low-density residential (6):** This includes residential areas, especially gardens and mining.
- **High-density residential (7):** This includes small residential areas, spectator recreation and multi-family residential areas.
- **Wetland/Riparian (8):** This includes wetland, water-based recreation, water and waste disposal.

The percentage-distributions of these main land-use groups are shown and listed in Figure 5.2 and Table 5.3. The percentage-distribution (where applicable) of each individual land-use is summarised in Tables 5.4 to 5.8.



**Figure. 5.2:** Distribution of main land-use groups



**Table 5.3:** Main land-use groups

Main land-use group	Area (km <sup>2</sup> )	Percentage-distribution (%)
Forest	208	23
Open spaces	223	25
Grassland	276	31
CBD and industrial	26	3
Medium-density residential	73	8
Low-density residential	70	8
High-density residential	21	2
Wetland/ Riparian	4	0.4
<b>Total:</b>	<b>901</b>	<b>100</b>

**Table 5.4:** Land-use: Forest

Land-use	Area (km <sup>2</sup> )	Percentage-distribution (%)
Eucalyptus	21	10
Indigenous forest	41	20
Mixed- or undefined forest	23	11
Other forest	0.3	0
Pines	8	4
Woodland	109	53
Wattles	5	2
<b>Total:</b>	<b>207</b>	<b>100</b>

**Table 5.5:** Land-use: Open spaces

Land-use	Area (km <sup>2</sup> )	Percentage-distribution (%)
Irrigated pastures	1	0.5
Low-density smallholdings	15	7
Maize	33	15
Sugar cane	35	16
Undefined cropping	78	35
Parks: sport and fields	9	4
Savannah veld	51	23
Valley of Thousand Hills	1	0.5
<b>Total:</b>	<b>223</b>	<b>100</b>

**Table 5.6:** Land-use: Grassland

Land-use	Area (km <sup>2</sup> )	Percentage-distribution (%)
Undefined open spaces	39	14
Normal grassland	237	86
<b>Total:</b>	<b>276</b>	<b>100</b>

**Table 5.7:** Land-use: Medium-density residential

Land-use	Area (km <sup>2</sup> )	Percentage-distribution (%)
Medium-density residential (few trees)	11	15
Medium-density residential (trees)	27	37
Rural-urban transition	35	48
<b>Total:</b>	<b>73</b>	<b>100</b>

**Table 5.8:** Land-use: Wetland/ Riparian

Land-use	Area (km <sup>2</sup> )	Percentage-distribution (%)
Wetlands	4	80
Dams	1	20
<b>Total:</b>	<b>5</b>	<b>100</b>

These eight main land-use groups are assigned to pervious and impervious land segments in HSPF. As discussed in Chapter 3, pervious land allows water to infiltrate, while impervious land does not. However, there is an important distinction between the common understanding and HSPF's use of impervious land.

Generally, impervious land refers to any surface, which does not allow water to penetrate. This includes man-made structures such as driveways, sidewalks and rooftops. Impervious land can also include natural features such as rock outcrops of dense clay soil.

Water falling on an impervious surface cannot directly infiltrate and therefore it may travel to a pervious surface where it can infiltrate. Even though a surface is considered impervious, water falling on it still has the opportunity to infiltrate (Munson, 1998: 42).

This is the point at which HSPF's use of impervious land differs. In HSPF the impervious land segment is considered to be directly connected to a river, thus all the precipitation falling on it runs off. There is no infiltration or evaporation (Munson, 1998: 42-43).

In HSPF true impervious land is only likely to occur in urbanised areas in the proximity of a river. An urban driveway that drains to a storm sewer in the street and the sewer, in turn, drains to the nearest river is a typical example. The HSPF Application Guide (Donigian *et al.*, 1984) states that: "if urban runoff does not contribute significant water or pollutants to the study area, it is appropriate to represent the entire watershed with pervious land segments."

#### **5.1.5 Discretisation of Catchment and River Network**

Any discretisation method employed must take all the elements of a catchment, as well as the variation in the catchment parameters like land-use, soil types, vegetation, pollutant sources and topography into consideration (Coleman & Simpson, 1996: 3.3).

##### **□ Grid Method:**

A uniform grid is superimposed over the surfaces of the catchment regardless of any internal boundaries or drainage systems. Two-dimensional modelling is



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##### **□ Grid Method:**

A uniform grid is superimposed over the surfaces of the catchment regardless of any internal boundaries or drainage systems. Two-dimensional modelling is

normally used in this method. These grids play an important role in the solution of the overland flow equations. Even if the grid is very fine, the topography and rivers are normally poorly detailed. This method is normally used to model rural catchments (Coleman & Simpson, 1996: 3.4).

❑ **Element Method:**

This method involves the subdivision of the catchment into sub-catchments with the boundaries chosen along flow lines or water divides. These segments are further divided into elements parallel to the contours and to the stream at the end of a hillslope. Only one-dimensional modelling is possible. This method requires effort and a lot of data (Coleman & Simpson, 1996: 3.5).

❑ **Sub-catchment Method:**

This is one of the more common methods in use. The catchment is also divided into sub-catchments on the basis of topography, vegetation, soil types and land-use. Each catchment is considered to be homogeneous with only one set of parameters. As in the case of kinematic routing where the flow length has to be specified, the catchment is handled as a rectangular plane with a length and a width. This one-dimensional sheet flow from the rectangular plane will produce the same runoff as the actual catchment. This method is effective and can be used as an urban planning model for stormwater (Coleman & Simpson, 1996: 3.6-3.7).

❑ **Modular Method:**

This method is an improvement and extension of the sub-catchment method. The sub-catchment method is used for the overall discretisation of the catchment. The catchment is then further divided into modules, which are linked in such a way to represent the catchment and the stormwater drainage

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system. This method is frequently used in urban planning (Coleman & Simpson, 1996: 3.7).

The discretisation used in this HSPF study of the Msunduzi River Catchment is in accordance with the original ACRU model discretisation of the study area into hydrologically homogeneous sub-catchments (Sub-catchment method). These sub-catchments were further subdivided according to the land-use type, which constituted the HSPF pervious (PERLND) and impervious (IMPLND) land segments.

The sub-catchment layout of the Msunduzi River Catchment was numbered from one to 42. To this numbering scheme was appended a number from one to eight to indicate one of eight possible land-use groups. For instance, sub-catchment 1, land-use 8 was numbered 18, sub-catchment 10, land-use 5 was numbered 105, etc. Since all eight land-use groups do not occur in each sub-catchment, not all possible combinations of sub-catchment number or land-use group are represented in the HSPF input. HSPF performs the simulation in the order that land segment operations are specified in the OPN SEQUENCE (Operation Sequence) block of the HSPF user control input file.

Kienzle *et al.* (1997) stated that the discretisation standards, which were applied in the Msunduzi River Catchment, could be summarised as follows:

- The size of sub-catchments is normally determined by the level of homogeneity and the distribution of precipitation gauging networks; normally not greater than 50 km<sup>2</sup>;



- 
- Sub-catchments must be homogeneous in terms of the land-use, climate and soil and;
  - It's preferable that, if possible, there must be operational gauging weirs and water quality sampling stations with lengthy records at the outlet of the particular sub-catchment.

The river network in a catchment is also segmented into reaches, each of which is considered to be a perfectly mixed tank. Flow constraints and transport considerations normally determine the length of river reaches. Reach boundaries should also coincide with physical structures in the river, such as dams, tributaries and farm dams.

River hydrogeometry is a primary consideration in the process of river segmentation. Associated with each reach is a sub-catchment, which may then be divided into pervious and impervious land segments (Munson, 1998: 29).

The sub-catchments are defined so that precipitation falling anywhere in one of the 42 sub-catchments eventually drains to a single river reach. A total of 42 reaches were delineated. The drainage area for each reach was determined by using topographical maps and GIS data. Sub-catchments range in area from approximately 0 to 65 km<sup>2</sup>.

Unfortunately, there are few stage-discharge measurements in the Msunduzi River and little is known about channel geometry. The stage-discharge relationships of the river network can be determined making use of Manning's open channel flow equations (Munson, 1998: 38).

Multiplying the area of each land segment with the unit area runoff, the runoff to each reach can be determined and in conjunction the instream hydraulics and water quality processes can be simulated for the entire catchment (Donigian *et al.*, 1984: 39, 48).

The discretisation and relevant details of the Msunduzi River Catchment are summarised in Table 5.9.

**Table 5.9:** Summary of catchment discretisation

Reach-/ sub-catchment number	Area (km <sup>2</sup> )	Average sub-catchment slope (%)	River (tributaries where applicable)	Length (km)
1	39	12	Msunduzi	13
2	54	15	Msunduzi	14
3	1.4	9	Msunduzi	2
4	39	15	Msunduzi/ Inkobongwana	5
5	45	19	Msunduzi	13
6	0.4	15	Msunduzi	1
7	30	22	Nqabeni	10
8	7	33	Nqabeni	5
9	4	17	Nqabeni	2
10	30	28	Msunduzi	10
11	36	31	Sinathingi	9
12	19	20	Msunduzi	6
13	15	18	Kwapata	9
14	9	12	Msunduzi	5
15	27	13	Wilgerfontein River	10
16	18	13	Slang Spruit	8
17	4	5	Slang Spruit	2
18	4	9	Slang Spruit / Camp's Drift	2

Table 5.9 (continued):

Summary of catchment discretisation

Reach-/ sub-catchment number	Area (km <sup>2</sup> )	Average sub-catchment slope (%)	River (tributaries where applicable)	Length (km)
19	3	4	Camp's Drift / Msunduzi	1
20	2	4	Msunduzi	1
21	13	5	Msunduzi	3
22	19	22	Dorpspruit	6
23	31	23	Town Bush Stream	5
24	24	11	Town Bush Stream/ Dorpspruit	11
25	6	2	Msunduzi	3
26	15	2	Blackborough Spruit	6
27	4	6	Msunduzi	3
28	1	1	Msunduzi	2
29	28	11	Baynes Spruit	6
30	11	10	Msunduzi	6
31	15	8	Mkhondeni	8
32	56	8	Mpushini	12
33	13	4	Mpushini	4
34	65	9	Msunduzi	18
35	41	16	Msunduzi	8
36	7	16	Msunduzi	4
37	46	27	Msunduzi	12
38	22	12	Mnambiti	12
39	33	12	Mshwati	12
40	36	19	Mshwati	11
41	9	10	Msunduzi	6
42	19	13	Msunduzi	15
<b>Total:</b>	<b>901</b>		<b>Total:</b>	<b>301</b>



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### 5.1.6 Data Collection

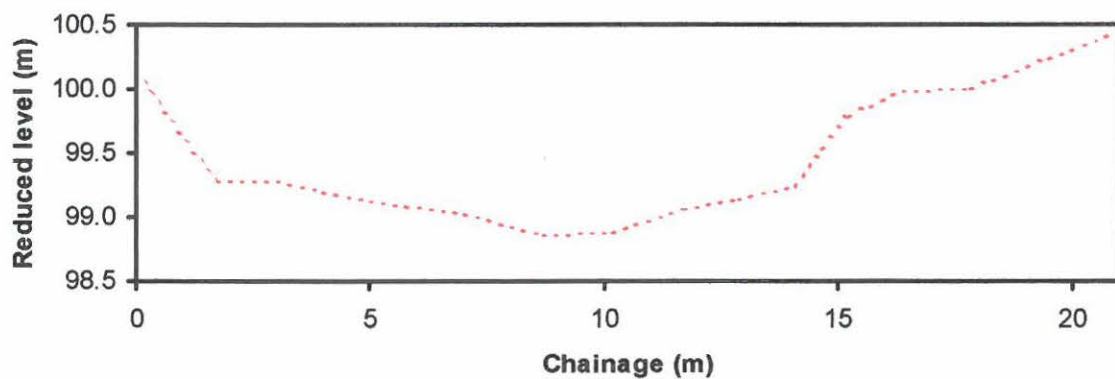
Due to the lack of stage-discharge measurements and channel geometry information, data were collected from topographical maps, orthographical photographs, GIS data and limited field visits. River hydrogeometry data, which were estimated, include characteristics such as the length, geometry (depth at mean flow, top- and bottom width), average slope of the channel, slope of the flood plane and the estimated values of Manning's  $n$  for both the channel and the floodplain.

The river cross-sections used to compute the stage-discharge relationships in HSPF were also based on limited topographical- and section surveys. The surveys were obtained from DWAF (Figures 5.3 to 5.15).

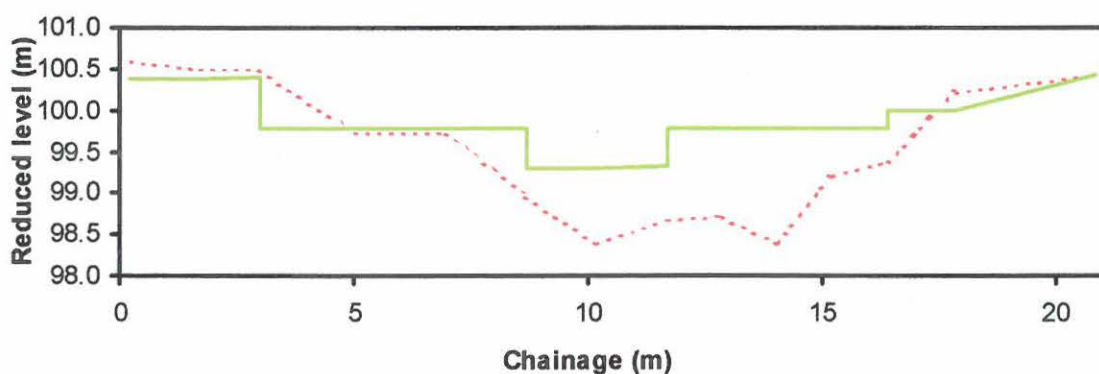
The stage-discharge relationship of each reach is specified in the HSPF user control input file with an FTABLE (Function Table) entry. Each FTABLE was generated with a small stand-alone Fortran program, XSECT, which was obtained from the USGS. The XSECT program assumes a uniform channel with trapezoidal cross-section and a shallow V-notch bottom. It uses the above-mentioned data as input data to compute the FTABLE for each reach. The FTABLE for Henley Dam was determined using data obtained from Umgeni Water. These data related dam water depth to area and capacity, as well as spillway discharge.

The average slope of each river reach was determined with the 10-85-equation as discussed in Chapter 6. Reach numbers were chosen to correspond with their associated sub-catchment number.

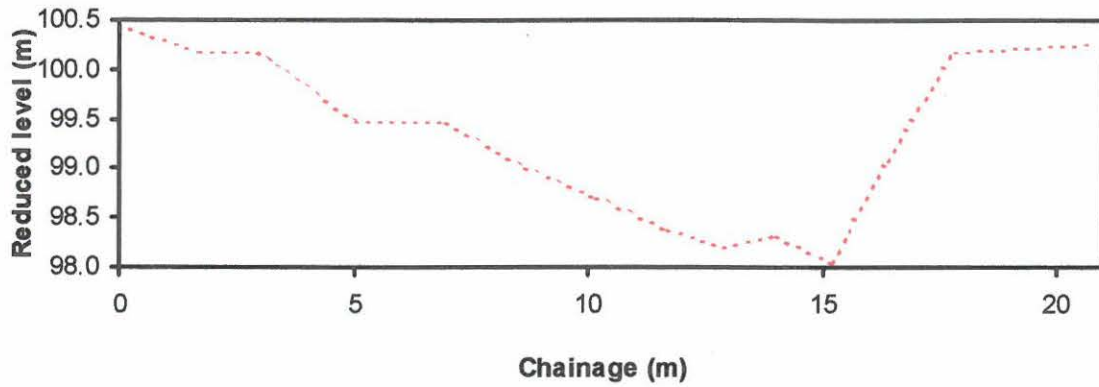
Typical river cross-sections representative of reaches six, 14, 18, 19, 20, 30 and 41 are shown in Figures 5.3 to 5.15. The dashed lines (---) represent the natural ground level of each river cross-section, while the solid lines (—), where applicable, represent the reduced levels of the hydrological gauging weirs' overflows (Sharp-crested, Crump- and V-crump weirs). The reduced levels of each cross-section are not related to each other and are at a local datum, except for sections for Camp's Drift, where mean sea level is used as datum.



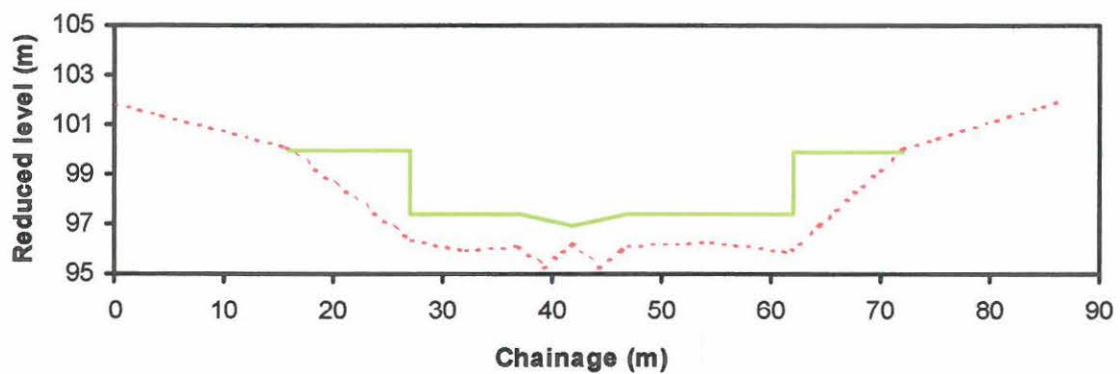
**Figure 5.3:** Sub-catchment 6: U2H011 Msunduzi River at Henley Dam: Inlet section



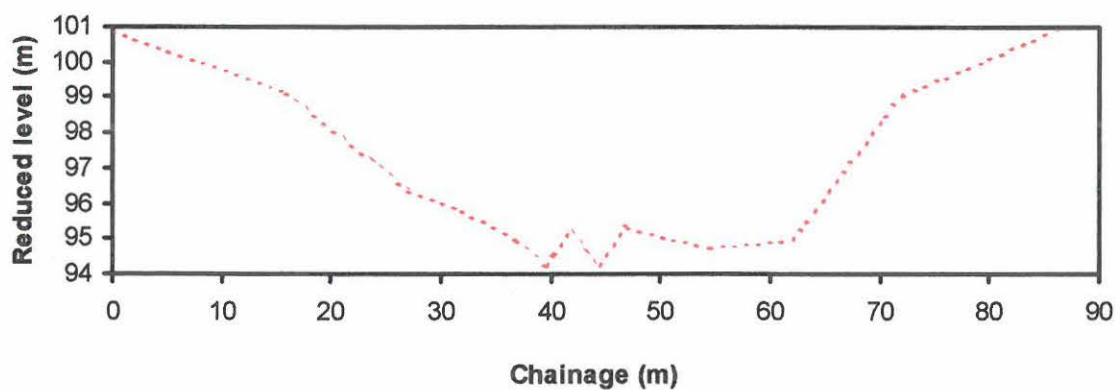
**Figure 5.4:** Sub-catchment 6: U2H011 Msunduzi River at Henley Dam: Upstream section



**Figure 5.5:** Sub-catchment 6: U2H011 Msunduzi River at Henley Dam: Downstream section

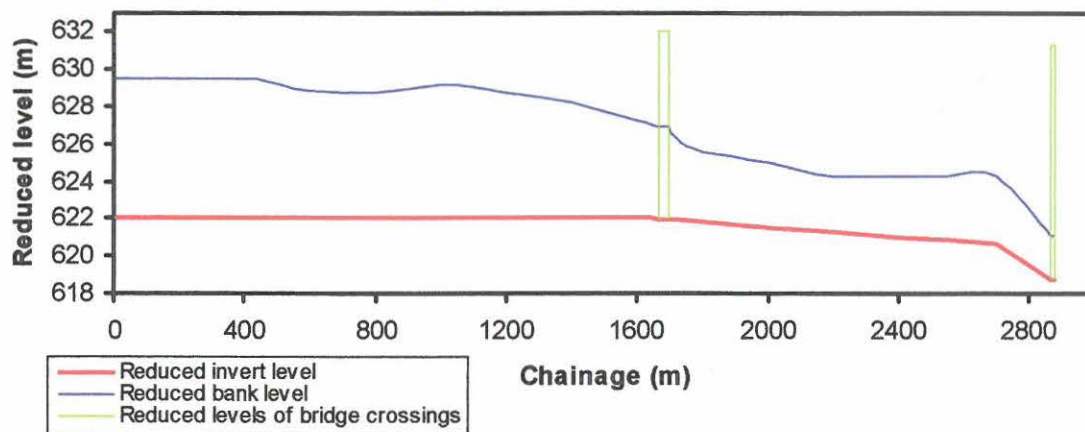


**Figure 5.6:** Sub-catchment 14: U2H058 Msunduzi River at Mason's Mill: Upstream section

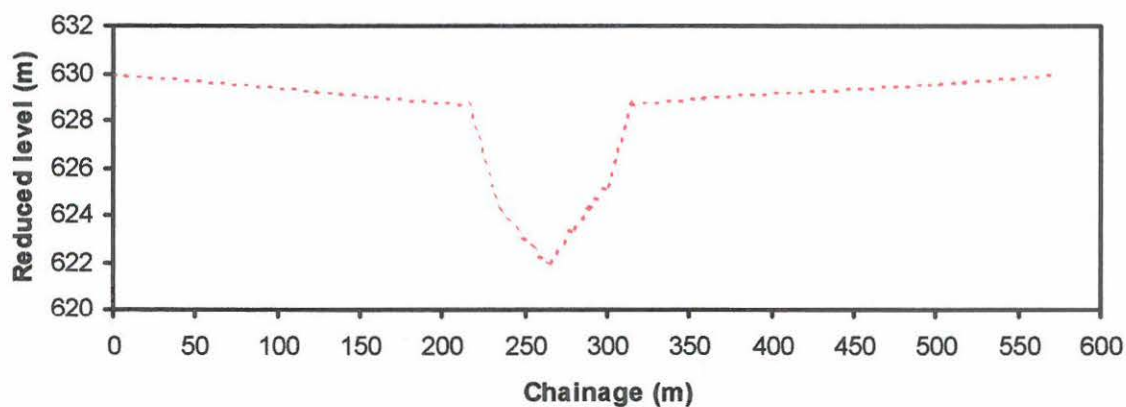


**Figure 5.7:** Sub-catchment 14: U2H058 Msunduzi River at Mason's Mill: Downstream section

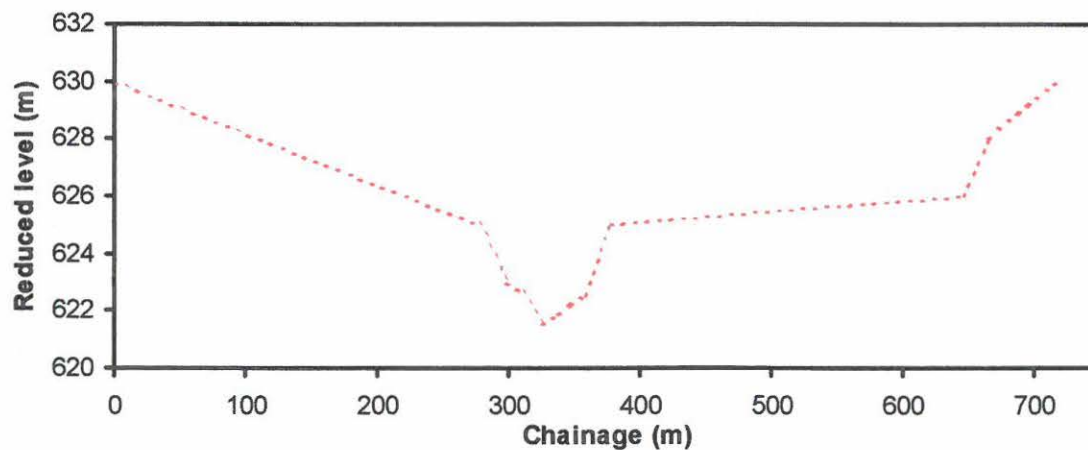




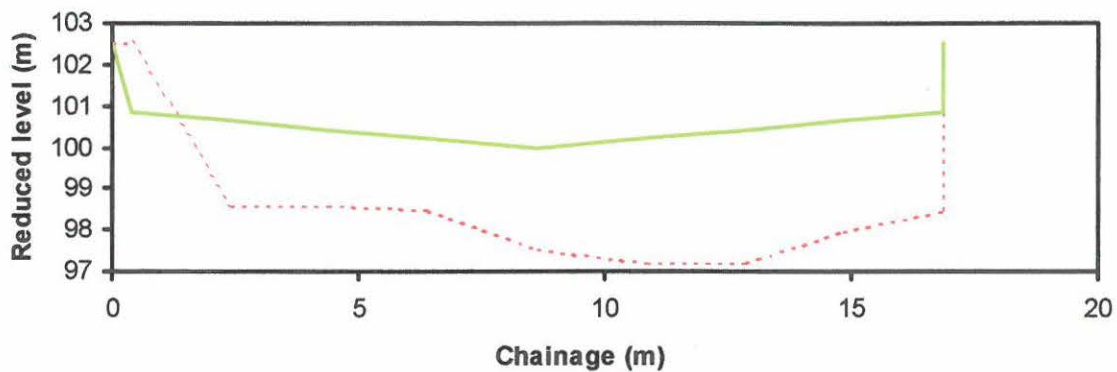
**Figure 5.8:** Sub-catchments 18-20: Camp's Drift: Longitudinal section



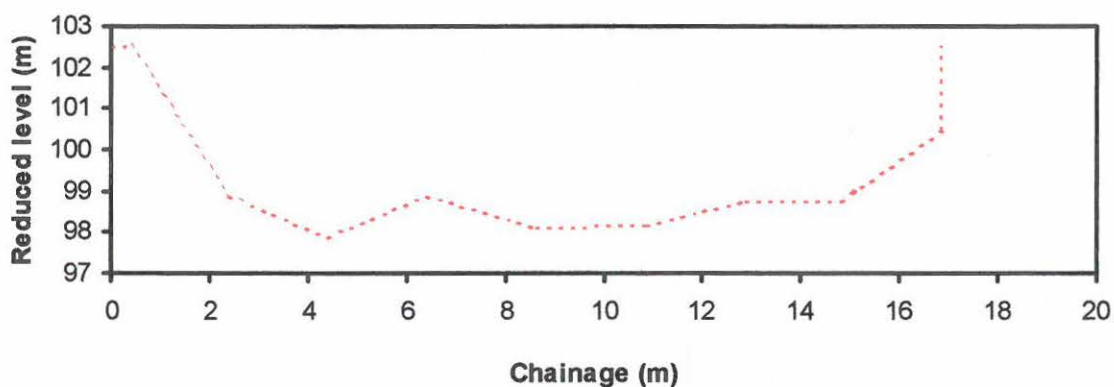
**Figure 5.9:** Sub-catchments 18-20: Camp's Drift: Cross-section at 1.2 km chainage



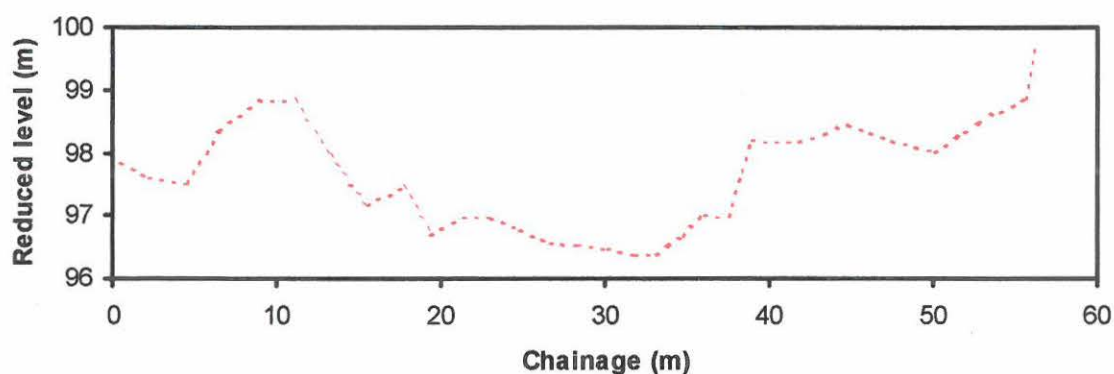
**Figure 5.10:** Sub-catchments 18-20: Camp's Drift: Cross-section at 2 km chainage



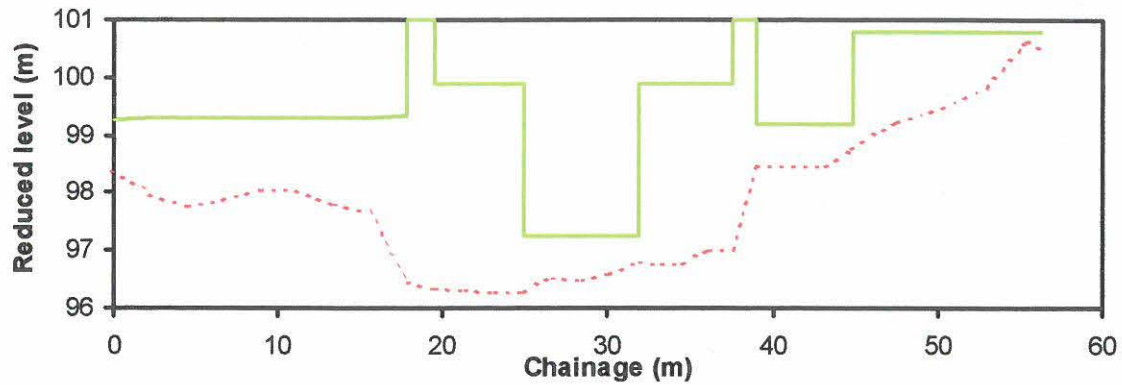
**Figure 5.11:** Sub-catchment 30: U2H041 Msunduzi River at Hampstead Park, Moto-X: Upstream section



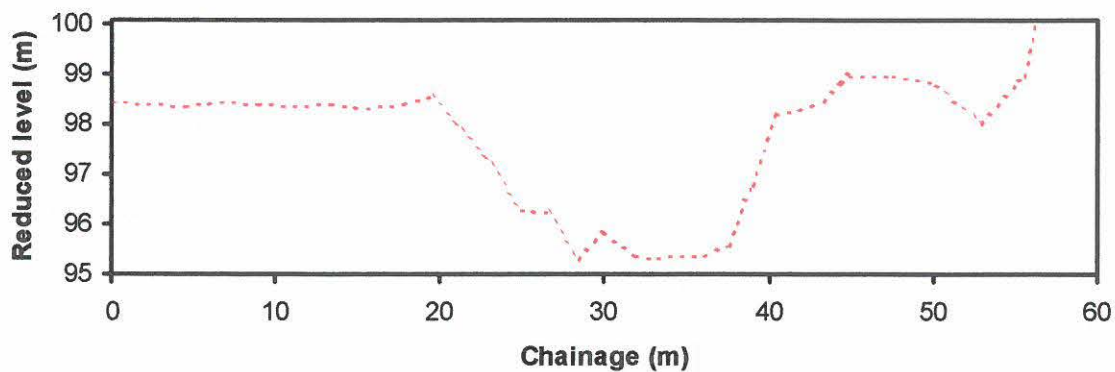
**Figure 5.12:** Sub-catchment 30: U2H041 Msunduzi River at Hampstead Park, Moto-X: Downstream section



**Figure 5.13:** Sub-catchment 41: U2H022 Msunduzi River at Nomfihlelo: Inlet section



**Figure 5.14:** Sub-catchment 41: U2H022 Msunduzi River at Nomfihlelo: Upstream section



**Figure 5.15:** Sub-catchment 41: U2H022 Msunduzi River at Nomfihlelo: Downstream section

## 5.2 HYDROLOGICAL DATA

This section describes the most significant anthropogenic hydrological alterations of the Msunduzi River Catchment.

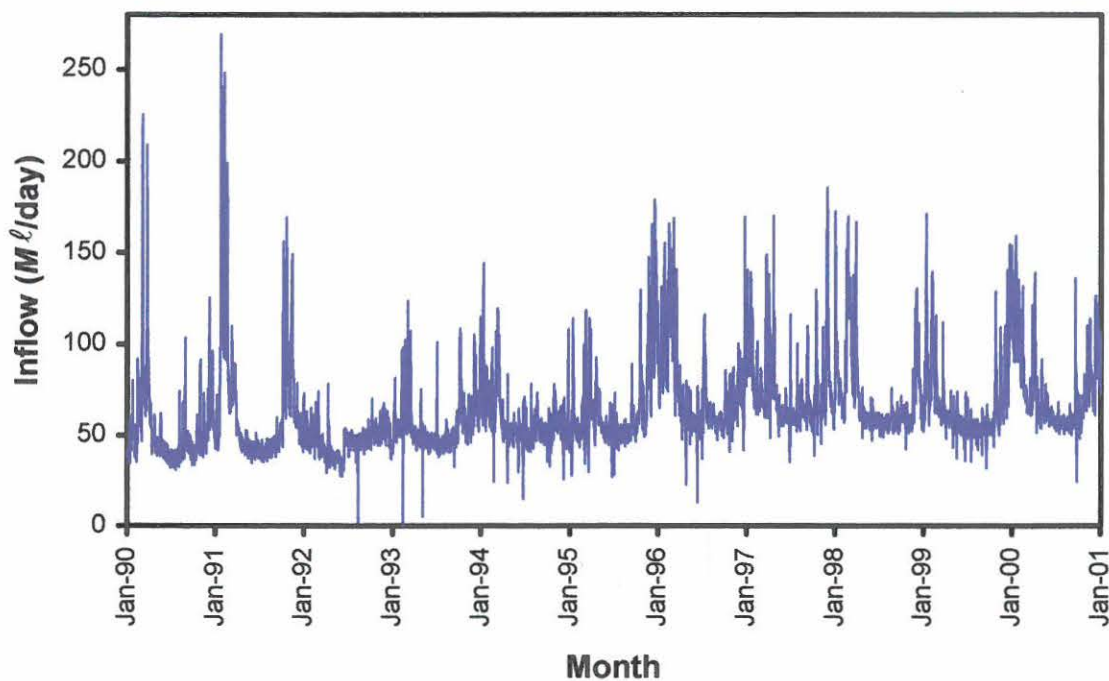
### 5.2.1 Human Water use

The primary water supply to Pietermaritzburg comes from the Midmar Dam, which is situated outside the Msunduzi River Catchment. Henley Dam (U2R005) also contributed until mid-1995, when all supply from Henley Dam ceased with the closure of the H.D. Hill water purification plant (Simpson, 2002).



### 5.2.2 Wastewater Treatment Plants

The Darvill wastewater treatment plant contributes water to the Msunduzi River. Much of the outflow represents an inter-basin transfer from Midmar Dam. The inflow data ( $M\ell/day$ ) at the plant for the period of record (1990 to 2001) are shown graphically in Figure 5.16. Outflow is not separately recorded.



**Figure 5.16:** Inflow to Darvill wastewater treatment plant: January 1990-January 2001 (Umgeni Water, 2002)

The assumption has been made that the total inflow equals the total outflow, thus ignoring any losses. Discharge is the lowest in late autumn and winter and the highest during late summer and early autumn. These discharges include not only municipal sewage, but also infiltrated groundwater in the pipes. The amount of infiltration is a function of the water table height. The water table is generally highest in late summer and lowest in winter, which correlates with the seasonal variation of total wastewater discharge (Simpson, 2002).

### 5.2.3 Hydrological Gauging Stations

The spatial locations of hydrological gauging-, precipitation- and water quality stations are illustrated in Chapter 9, Annexure A, Plates 6 and 7 and are listed in Table 5.10.

**Table 5.10:** Summary of hydrological gauging stations (DWAF, 1990)

Station description (DWAF number & name)	Data set	Period of record	Location (Lat./ Long.)
U2H011 Msunduzi River at Henley Dam	Stage-discharge  Water quality	1993 - 1996  1977 - 2001	Sub-catchment 6 Lat: 29°38'44" Long: 30°15'34"
U2R005 Henley Dam	Stage-discharge  Water quality	1989 - 2001  1985 - 2001	Sub-catchment 9 Lat: 29°37'23" Long: 30°14'50"
U2H058 Msunduzi River at Mason's Mill	Stage-discharge  Water quality	1995 - 2000  1995 - 2000	Sub-catchment 14 Lat: 29°37'51" Long: 30°21'12"
U2H041 Msunduzi River at Hampstead Park, Moto-X	Stage-discharge  Water quality	1996 - 2000  1985 - 2000	Sub-catchment 30 Lat: 29°36'27" Long: 30°27'00"
U2H022 Msunduzi River at Nomfihlelo	Stage-discharge  Water quality	1987 - 2000  1984 - 2001	Sub-catchment 41 Lat: 29°39'39" Long: 30°38'13"

### 5.2.4 Data Quality

All the processed daily streamflow- and precipitation data for the Msunduzi River Catchment for the relevant period of record have been extracted from the DWAF files through the Computing Centre for Water Research (CCWR). Although there are many sites at which water quality grab samples are taken, there is paucity of measured streamflow data and even at sites with streamflow measurements; the data were often unreliable or missing.

All the missing streamflow data are summarised in Table 5.11.

**Table 5.11:** Summary of missing streamflow data

Station description (DWAF number & name)	Data set	Period of missing data
U2H011 Msunduzi River at Henley Dam	Stage-discharge (1993 - 1996)	1995/01/15 - 1995/02/15; 1995/06/30 - 1995/08/01; 1995/08/11 - 1995/11/21; 1995/12/21 - 1996/01/04; 1996/01/07 - 1996/02/01.
U2H058 Msunduzi River at Mason's Mill	Stage-discharge (1995 - 2000)	1996/09/17 - 1996/09/25; 1997/03/18 - 1997/03/25; 1997/11/25 - 1997/12/02; 1998/01/13 - 1998/01/20; 1998/04/28 - 1998/06/11; 2000/04/04 - 2000/04/14.
U2H041 Msunduzi River at Hampstead Park, Moto-X	Stage-discharge (1996 - 2000)	1996/11/26 - 1997/11/03; 1999/08/13 - 1999/09/28; 1999/12/28 - 2000/01/04.
U2H022 Msunduzi River at Nomfihlelo	Stage-discharge (1987 - 2000)	1987/08/24 - 1987/08/31; 1987/09/21 - 1987/10/10; 1988/01/25 - 1989/07/20; 1991/06/27 - 1991/11/10; 1991/11/23 - 1992/01/10; 1994/06/01 - 1994/12/09; 1995/01/25 - 1995/02/01; 1996/04/03 - 1996/04/23; 1996/10/09 - 1996/10/16; 1997/12/09 - 1997/12/31; 1998/07/08 - 1998/07/24.

The HSPF model was therefore verified with the hydrological gauging stations with the best streamflow data, viz. at U2H022, U2H041 and U2H058.



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### **5.2.5 Conclusions regarding Flow Data**

The human-induced hydrological alterations (human water use and treatment plants) can have a substantial impact on the hydrological response of the Msunduzi River Catchment. Therefore, it is important to take all these parameters and factors into consideration during the process of initial data development for the period of simulation, as well as during the actual simulation process. Streamflow data records are vital to verify the ability of the HSPF modelling system to simulate the observed flows accurately, before any impact scenarios can be simulated. This fact also emphasises the importance that the data quality obtained from hydrological gauging stations must be good and a true reflection of the real world. Data collection is a continuous process, as new data are compiled it can be incorporated into the HSPF model with the goal of improving hydrological simulations.

## **5.3 METEOROLOGICAL DATA**

The HSPF simulation is driven by meteorological data measured in or near the Msunduzi River Catchment. The data must be representative of the catchment. A good hydrological simulation is unlikely to be achieved without accurate and representative meteorological data. Precipitation and evaporation are the most important meteorological variables for hydrological simulation.

### **5.3.1 Precipitation**

Precipitation data are one of the most important variables in hydrological modelling; therefore it is important to obtain the best daily precipitation data for each catchment. In practice it is normally applicable to make use of one “driver”

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precipitation station to forecast the hydrological response of a sub-catchment (Kienzle *et al.*, 1997: 14).

Target precipitation stations were selected based on their position in the Msunduzi River Catchment and number of years of record. A set of nearby stations with similar Mean Annual Precipitation (MAP) to the target station was selected to fill in missing data at the target stations.

Only six of the 13 precipitation stations within the catchment were identified as suitable target stations, mainly based on the quality and period of record. The set of nearby stations consists of a further six precipitation stations.

The precipitation stations were numbered by the CCWR according to a numbering scheme devised by the South African Weather Bureau (SAWB). In addition to the SAWB number for each station, the source organisation of the data was indicated by a suffix: 'A' for Department of Agriculture, 'P' for private individual, 'S' for South African Sugar Experiment Station and 'W' for SAWB. The details and relevant data of these precipitation stations are listed in Tables 5.12 and 5.13. The location of each station in relation to the catchment boundary is shown in Chapter 9, Annexure A, Plate 6.

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precipitation station to forecast the hydrological response of a sub-catchment (Kienzle *et al.*, 1997: 14).

Target precipitation stations were selected based on their position in the Msunduzi River Catchment and number of years of record. A set of nearby stations with similar Mean Annual Precipitation (MAP) to the target station was selected to fill in missing data at the target stations.

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**Table 5.12:** Summary of target precipitation data within the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Altitude above MSL (m)	Period of record	MAP (mm)
Allerton, 0239604W	Sub-catchment 23 Lat: 29°35' Long: 30°21'	703	1912 - 2000	957
Camperdown, 0240073W	Sub-catchment 39 Lat: 29°43'00" Long: 30°33'00"	762	1914 - 2000	685
Edendale, 0239518W	Sub-catchment 12 Lat: 29°38' Long: 30°18'	765	1946 - 1997	946
Pietermaritzburg purification works, 0239756W	Sub-catchment 24 Lat: 29°36' Long: 30°26'	609	1969 - 2000	826
Pietermaritzburg purification works, 0239577W	Sub-catchment 24 Lat: 29°37' Long: 30°20'	765	1949 - 1997	927
Ukulinga-AGR, 0239700W	Sub-catchment 21 Lat: 29°40' Long: 30°24'	775	1959 - 1991	706

**Table 5.13:** Summary of target precipitation data near the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Altitude above MSL (m)	Period of record	MAP (mm)
Baynesfield Estates, 0239585W	Lat: 29°46' Long: 30°20'	808	1927 - 2000	771
Cedara College, 0239482W	Lat: 29°32' Long: 30°17'	1066	1959 - 2000	842
Elandshoek, Boston, 0239097A	Lat: 29°37' Long: 30°04'	1540	1905 - 2000	1007
Inchanga, 0240284W	Lat: 29°44' Long: 30°40'	620	1928 - 1998	790

**Table 5.13 (continued):** Summary of target precipitation data near the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Altitude above MSL (m)	Period of record	MAP (mm)
Nagle, 0240185W	Lat: 29°35' Long: 30°37'	442	1941 - 2000	749
Vaucluse, 0239133W	Lat: 29°43' Long: 30°05'	1430	1933 - 1997	1005

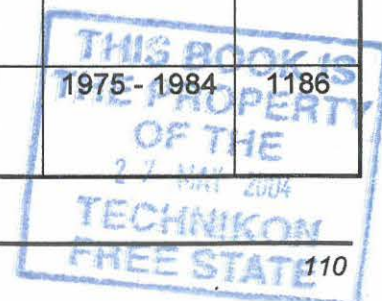
The MAP varies from 684 mm to 1186 mm throughout the entire catchment. The detail and relevant data of the additional precipitation stations used to fill in any missing data are listed in Table 5.14.

**Table 5.14:** Summary of additional precipitation data in and near the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Period of record	MAP (mm)
Allerton, 0239184A	Lat: 29°34' Long: 30°21'	1959 - 1960	1069
Ashley Grange, Merrivale, 0239184A	Lat: 29°34' Long: 30°07'	1903 - 1989	732
Baynesfield Estates, 0239585A	Lat: 29°45' Long: 30°20'	1927 - 1991	767
Bloemendal, Bishopstown, 0239812A	Lat: 29°32' Long: 30°28'	1952 - 1989	884
Botha's Hill, 0240404W	Lat: 29°44' Long: 30°44'	1951 - 2000	823
Bruyns Hill, 0270329S	Lat: 29°29' Long: 30°41'	1976 - 1998	981
Cedara Agricultural Station, 0239482A	Lat: 29°32' Long: 30°17'	1914 - 1991	875
Cosmoore, Cato Ridge, 0239855A	Lat: 29°45' Long: 30°29'	1956 - 1988	763

**Table 5.14 (continued):** Summary of additional precipitation data in and near the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Period of record	MAP (mm)
Dargle, 0239002W	Lat: 29°32' Long: 30°01'	1953 - 2000	976
Edmonds, J.P., 0240033A	Lat: 29°33' Long: 30°32'	1949 - 1989	734
Hilly Prospect, 0239196A	Lat: 29°46' Long: 30°07'	1947 - 1983	1035
Intake, 0240564W	Lat: 29°47' Long: 30°46'	1923 - 1991	867
Kensington, 0239784W	Lat: 29°34' Long: 30°27'	1965 - 1987	932
Kloof purification works, 0240586W	Lat: 29°46' Long: 30°50'	1932 - 2000	1064
Merrivale, 0239421W	Lat: 29°31' Long: 30°14'	1914 - 1978	805
Natal Est., Mount Edgecombe, 0240883A	Lat: 29°43' Long: 31°00'	1929 - 1989	1043
Natal Est., Thornville, 0239705A	Lat: 29°45' Long: 30°24'	1981 - 1989	737
Phipson, Merrivale, 0239483A	Lat: 29°33' Long: 30°17'	1983 - 1989	925
Pietermaritzburg Botanic Gardens, 0239605P	Lat: 29°35' Long: 30°21'	1907 - 1989	1026
Pietermaritzburg Country Club, 0239574W	Lat: 29°35' Long: 30°20'	1932 - 1964	1161
Pietermaritzburg Municipality, 0239666W	Lat: 29°36' Long: 30°23'	1959 - 1968	822
Sevontein, Elandskop, 0239225A	Lat: 29°45' Long: 30°08'	1983 - 1989	861
St. John's Cottage, 0239633A	Lat: 29°33' Long: 30°22'	1975 - 1984	1186

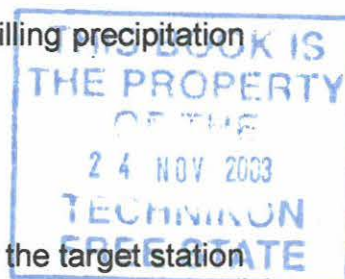




**Table 5.14 (continued):** Summary of additional precipitation data in and near the Msunduzi River Catchment

Station description (Name & number)	Location (Lat./ Long.)	Period of record	MAP (mm)
Symonds Lane, Howick, 0269388A	Lat: 29°28' Long: 30°13'	1970 - 1989	961
Thornville, 0239705S	Lat: 29°45' Long: 30°24'	1983 - 2000	758
Ukulinga- AGR. Res., 0239730W	Lat: 29°40' Long: 30°25'	1959 - 1966	793
Umlaas Road, 0240014W	Lat: 29°44' Long: 30°31'	1931 - 1976	749
Wahroonga, 0239216W	Lat: 29°36' Long: 30°08'	1925 - 1944	875
Windy Hill, 0270119S	Lat: 29°29' Long: 30°34'	1966 - 1996	984
Windy Hill No. 2, 0270119W	Lat: 29°30' Long: 30°34'	1932 - 1999	994

The daily precipitation data were obtained from the CCWR precipitation database and in-filled with the Time Series Extraction and Manipulation (TEAMS) program available on the CCWR. The technique used by TEAMS for in-filling precipitation data is as follows:



The set of selected nearby stations used to fill in missing data at the target station was ordered according to preference. The order of preference was based on the similarity of MAP and geographic proximity to the target station. Each of the infilling stations, starting with the most preferred station and ending with the least preferred station, was selected in turn, and their data used for filling in the target until all missing values were filled, or the set of infilling stations was exhausted.

The data from an infilling station were adjusted before filling in the target by multiplying by the ratio of the MAPs of the target- and infilling stations. The details of the target- and infilling stations are listed in Table 5.15.

**Table 5.15:** Summary of the infilling procedure of precipitation data

Target precipitation station (Name & number)	List of infilling precipitation stations used (in order of preference)
Allerton, 0239604W	0239604A, 0239574W, 0239605P, 0239216W, 0239576W, 0239666W, 0239756W, 0239756AW, 0239633A and 0239482W
Baynesfield Estates, 0239585A	239585A, 239705A, 0239705S, 0239700A, 0239730W, 0239855A, 0240014W, 0239518W and 0239577W
Camperdown, 0240073W	240014W, 0239855A, 0240284W, 0240404W, 0239705A, 0239730W, 0239700A, 0239818A and 0240586W
Cedara College, 0239482W	0239482A, 0239483A, 0239421W, 0269388A, 0239184A, 0239574W, 0239518W, 0239216W, 0239604W and 0239604A
Edendale, 0239518W	0239577W, 0239574W, 0239604W, 0239605P, 0239576W, 0239666W, 0239604A, 0239585A and 0239585W
Elandshoek, Boston, 0239097A	0239216W, 0239002W, 0239184A, 0239133W, 0239225A, 0239196A, 0239421W, 0239483A and 0239482W
Inchanga, 0240284W	0240564W, 0240404W, 0240073W, 0240185W, 0240014W, 0239855A and 0240586W
Nagle, 0240185W	0270329S, 0270119S, 0270119W, 0240033A, 0239818A, 0239756W, 0239784W, 0240073W, 0240284W and 0240586W

**Table 5.15 (continued):** Summary of the infilling procedure of precipitation data

Target precipitation station (Name & number)	List of infilling precipitation stations used (in order of preference)
Pietermaritzburg, purification works 0239577W	0239518W, 0239574W, 0239756AW, 0239756W, 0239604W, 0239604A, 0239576W, 0240185W and 0239700A
Pietermaritzburg, purification works 0239756W	0239577W, 0239518W, 0239574W, 0239756AW, 0239604W, 0239604A, 0239576W, 0240185W and 0239700A
Ukulinga- AGR, 0239700W	239730W, 0239518W, 0239756AW, 0239756W, 0239666W, 0240883A, 0239585A, 0239585W and 0239855A
Vaocluse, 0239133W	0239196A, 0239225A, 0239097A, 0239216W, 0239184A, 0239518W, 0239002W, 0239421W, 0239482W and 0239482A

There are two main sources of precipitation error relevant to the Msunduzi River Catchment HSPF model: Poor areal representation and possible instrumentation error. Each is discussed below:

- It is difficult to accurately represent precipitation falling on a catchment as large as the Msunduzi. Precipitation is highly variable in time and space. Precipitation gauges only measure precipitation at a single point. Modellers must therefore be satisfied to apply data collected at one point over a large area of the catchment, unless a catchment is peppered with precipitation stations. The total amount of precipitation, areal- and time distributions of a storm throughout the duration of the storm are three major factors that affect the peak rate of runoff (Brach, 2000: 8.20-2).



The HSPF Application Guide (Donigian *et al.*, 1984) states that: “the assumption of uniform areal precipitation is a major source of error with direct effects on the simulation, since precipitation is the driving force of HSPF. Precipitation is rarely uniform and is highly non-uniform in thunderstorm prone regions of the country. Catchments greater than approximately 100 km<sup>2</sup> require at least three different precipitation records, perhaps more if precipitation patterns are highly variable.”

- Instrumentation error might be a second source of inaccuracy in precipitation measurements. First, gauges must be designed so they do not deflect precipitation away from the collector.

The area influenced by each target precipitation station was determined using Thiessen polygons (Wilson, 1990: 21-25). These polygons are also representative of a specific sub-catchment (-s). The borders of the sub-catchments and polygons did not coincide. Thus, the area of each sub-catchment was divided into sub-areas according to its coincidence with a Thiessen polygon using GIS. These sub-areas were expressed as a percentage-distribution of the relevant polygon of each target precipitation station. These percentage-distributions were then used in the External Sources Table (EXT SOURCES) in the HSPF user control input file and linked to the specific target precipitation stations stored in WDM data sets using an appropriate multiplier.

Assumptions made to distribute the measured precipitation over the catchment and throughout the time distribution of the precipitation event may have a great impact on the simulation results, especially in the processes of calibration and

verification. The farther the precipitation station is from a specific part of the catchment, the less likely the data from the station will adequately represent the depth or time distribution that actually occurred there (Brach, 2000: 8.20–2).

### 5.3.2 Evaporation

Mean monthly A- or S-pan potential evaporation values can be superimposed on the boundaries of a sub-catchment in order to produce mean monthly A-pan equivalent values for each sub-catchment (Kienzle *et al.*, 1997: 15). Gridded images of mean monthly A-pan equivalent evaporation have been developed and are available at a resolution of one minute by one minute of a degree latitude and longitude covering Southern Africa (Schulze, 1995: AT 4.10). These gridded images of the monthly A-pan equivalent evaporation of the Msunduzi River Catchment are illustrated in Chapter 9 (Annexure A, Plate 9).

The only weather stations near the catchment for which any evaporation data were available were Cedara College (0239482W) and the South African Sugar Experiment Station at Mount Edgecombe. Neither of these is representative of the catchment and according to Donigian *et al.* (1984) the model is not sensitive to evaporation inputs. Therefore the mean monthly A-pan equivalent evaporation values, as developed by Schulze and Maharaj (1991), were obtained from the CCWR.

The catchment was divided into three evaporation regions in two steps. First, the mean monthly evaporation within each sub-catchment was determined. Secondly, similar monthly evaporation values were assigned to an evaporation region. This resulted in a total of three evaporation regions. Representative grid

points were chosen for the upper, mid and lower regions and the 12 monthly grid values were used to generate a monthly time series, which was disaggregated to three daily total evaporation time series in HSPF.

The monthly A-pan evaporation is converted to Potential Evapotranspiration (PET) in HSPF by multiplying each value by a factor ranging between 0.8 and 0.85 (Munson, 1998: 75). The percentage-distribution of the mean annual evaporation of the Msunduzi River Catchment is listed in Table 5.16.

**Table 5.16:** Mean annual evaporation

Mean annual evaporation range (mm)	Area (km <sup>2</sup> )	Percentage-distribution (%)
1572-1630	79	9
1631-1663	197	22
1664-1693	256	28
1694-1720	256	28
1721-1779	113	13
<b>Total:</b>	<b>901</b>	<b>100</b>



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## **RESULTS & DISCUSSIONS**

### **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

## 6. RESULTS AND DISCUSSIONS

### 6.1 DATA COLLECTION RESULTS

This section discusses the results obtained from the collection of river hydrogeometry data, stage-discharge measurements and development of Thiessen polygons for precipitation distribution in the Msunduzi River Catchment.

One of the important river hydrogeometry characteristics, the average slope of a river reach was determined by making use of the “10-85” method as prescribed by the USGS (Rooseboom, Basson, Loots & Wiggett, 1993: 2.29). The 10-85-equation is as follows:

$$S_{AVG.} = \frac{(H_{0.85L} - H_{0.10L})}{(1000 * 0.75L)} \quad (13)$$

where:

$L$  = Length of river reach (km)

$S_{AVG.}$  = Average slope (m/m)

$H_{0.85L}$  = Height of river reach at length 0.85  $L$

$H_{0.10L}$  = Height of river reach at length 0.10  $L$

However, during the determination of the stage-discharge relationships 100% of the river reach length is used and not 75% as in the 10-85-equation. The results of the stage-discharge relationship computations are listed in the FTABLES (Annexure C: 236 & 270).

The results are listed in Table 6.1.

**Table 6.1:** Summary of the average slope of the river reaches

Reach- or sub-catchment number	Length (km)	$H_{0.85L}$ (m)	$H_{0.10L}$ (m)	Average slope ( $S_{AVG.}$ ) (m/m)
1	12.9	1462	1169	0.030
2	13.5	1351	1156	0.019
3	1.7	1128	1117	0.008
4	5.4	1397	1108	0.072
5	12.5	1050	949	0.011
6	0.6	936	935	0.001
7	9.5	1305	979	0.046
8	5.2	1275	975	0.076
9	2.4	934	930	0.002
10	9.5	896	723	0.024
11	9.2	1159	754	0.059
12	5.7	694	665	0.007
13	9.2	1140	718	0.061
14	4.8	655	626	0.008
15	10.2	936	693	0.032
16	7.9	902	689	0.036
17	1.8	658	647	0.008
18	1.8	622	621.9	0.0001
19	1.1	622	621.8	0.0001
20	0.9	622	621	0.002
21	2.9	620	619	0.001
22	5.7	993	758	0.055
23	4.6	933	698	0.068
24	10.7	710	629	0.010
25	3.4	618	611	0.003
26	6.4	746	623	0.025
27	2.6	609	603	0.003
28	1.8	602	600	0.001
29	6.1	798	648	0.033
30	5.8	621	591	0.007
31	8.2	794	666	0.021
32	11.9	856	660	0.022



**Table 6.1 (continued):** Summary of the average slope of the river reaches

Reach- or sub-catchment number	Length (km)	$H_{0.85L}$ (m)	$H_{0.10L}$ (m)	Average slope ( $S_{AVG.}$ ) (m/m)
33	4.2	628	598	0.010
34	18.1	574	509	0.005
35	7.5	492	449	0.008
36	4.2	441	432	0.003
37	12.4	423	383	0.004
38	11.6	679	495	0.021
39	11.5	709	498	0.025
40	11	456	385	0.009
41	6.1	375	362	0.003
42	15.1	349	292	0.005

As discussed in Chapter 5, the sub-areas of each sub-catchment influenced by the different target precipitation stations were determined by making use of Thiessen polygons. The average precipitation calculated with the arithmetic mean had a value of 850 mm, which correlates well with the result of the Thiessen polygons (844 mm). The summary of Thiessen polygons are shown in Table 6.2.

**Table 6.2:** Summary of Thiessen polygons

Sub-catchment	Station description (Name & number)	MAP (mm)	Sub-area (km <sup>2</sup> )	Precipitation area (mm.km <sup>2</sup> )
1	Elandshoek, Boston (0239097A)	1007	12.2	12304
	Vaucluse, Elandskop (0239133W)	993	26.8	26561
2	Vaucluse, Elandskop (0239133W)	993	53.7	53280
3	Vaucluse, Elandskop (0239133W)	993	1.4	1387
4	Elandshoek, Boston (0239097A)	1007	11.8	11829
	Vaucluse, Elandskop (0239133W)	993	24.3	24166
	Edendale (0239518W)	946	3	2844

**Table 6.2 (continued):** Summary of Thiessen polygons

Sub-catchment	Station description (Name & number)	MAP (mm)	Sub-area (km <sup>2</sup> )	Precipitation.area (mm.km <sup>2</sup> )
5	Elandshoek, Boston (0239097A)	1007	0.9	903
	Vaucluse, Elandskop (0239133W)	993	2.6	2598
	Edendale (0239518W)	946	40.4	38206
	Baynesfield Estates (0239585A)	767	0.7	504
6	Edendale (0239518W)	946	0.4	348
7	Elandshoek, Boston (0239097A)	1007	9.9	10002
	Edendale (0239518W)	946	20.4	19329
8	Cedara College (0239482W)	842	0.2	173
	Edendale (0239518W)	946	7.2	6845
9	Edendale (0239518W)	946	4.1	3914
10	Cedara College (0239482W)	842	1.7	1397
	Edendale (0239518W)	946	28.2	26695
11	Edendale (0239518W)	946	31.8	30095
	Baynesfield Estates (0239585A)	767	3.9	3017
12	Edendale (0239518W)	946	18.4	17355
	Pietermaritzburg purification works (0239577W)	927	1	933
13	Edendale (0239518W)	946	10.8	10195
	Pietermaritzburg purification works (0239577W)	927	0.8	748
	Baynesfield Estates (0239585A)	767	3.4	2636
14	Edendale (0239518W)	946	0.1	80
	Pietermaritzburg purification works (0239577W)	927	8.7	8096
15	Edendale (0239518W)	946	3.6	3362
	Pietermaritzburg purification works (0239577W)	927	0.8	708

**Table 6.2 (continued):** Summary of Thiessen polygons

Sub-catchment	Station description (Name & number)	MAP (mm)	Sub-area (km <sup>2</sup> )	Precipitation area (mm.km <sup>2</sup> )
15	Baynesfield Estates (0239585A)	767	14.5	11109
	Ukulinga-AGR (0239700A)	706	7.8	5495
16	Baynesfield Estates (0239585A)	767	1.4	1053
	Ukulinga-AGR (0239700A)	706	16.5	11672
17	Pietermaritzburg purification works (0239577W)	927	0.6	569
	Ukulinga-AGR (0239700A)	706	3.6	2537
18	Pietermaritzburg purification works (0239577W)	927	3.0	2773
	Ukulinga-AGR (0239700A)	706	0.6	438
19	Pietermaritzburg purification works (0239577W)	927	1.7	1524
	Ukulinga-AGR (0239700A)	706	1.0	709
20	Pietermaritzburg purification works (0239577W)	927	1.8	1648
	Ukulinga-AGR (0239700A)	706	0.2	168
	Pietermaritzburg purification works (0239756AW)	826	0.1	15
21	Pietermaritzburg purification works (0239577W)	927	0.2	197
	Allerton (0239604W)	957	1.9	1792
	Ukulinga-AGR (0239700A)	706	9.1	6426
	Pietermaritzburg purification works (0239756AW)	826	2.0	1662
22	Cedara College (0239482W)	842	3.2	2704
	Edendale (0239518W)	946	1.6	1477
	Pietermaritzburg purification works (0239577W)	927	8.3	7677
	Allerton (0239604W)	957	6.1	5829



Table 6.2 (continued): Summary of Thiessen polygons

Sub-catchment	Station description (Name & number)	MAP (mm)	Sub-area (km <sup>2</sup> )	Precipitation.area (mm.km <sup>2</sup> )
23	Cedara College (0239482W)	842	5.5	4610
	Allerton (0239604W)	957	25.1	23996
24	Edendale (0239518W)	946	0.1	65
	Pietermaritzburg purification works (0239577W)	927	8.9	8203
	Allerton (0239604W)	957	14.6	13933
	Pietermaritzburg purification works (0239756AW)	826	0.8	660
	Allerton (0239604W)	957	0.5	425
25	Pietermaritzburg purification works (0239756AW)	826	5.5	4514
	Ukulinga-AGR (0239700A)	706	10.3	7266
26	Pietermaritzburg purification works (0239756AW)	826	4.4	3619
	Pietermaritzburg purification works (0239756AW)	826	4.4	3595
27	Pietermaritzburg purification works (0239756AW)	826	1.4	1124
28	Pietermaritzburg purification works (0239756AW)	826	1.4	1124
29	Allerton (0239604W)	957	9.1	8727
	Pietermaritzburg purification works (0239756AW)	826	19.1	15751
30	Pietermaritzburg purification works (0239756AW)	826	11.1	9158
31	Ukulinga-AGR (0239700A)	706	14.9	10527
32	Baynesfield Estates (0239585A)	767	4.9	3748
	Ukulinga-AGR (0239700A)	706	42.3	29861
	Camperdown (0240073W)	685	8.6	5861
33	Ukulinga-AGR (0239700A)	706	9.6	6785
	Pietermaritzburg purification works (0239756AW)	826	0.7	570
	Camperdown (0240073W)	685	3.0	2035

**Table 6.2 (continued):** Summary of Thiessen polygons

Sub-catchment	Station description (Name & number)	MAP (mm)	Sub-area (km <sup>2</sup> )	Precipitation.area (mm.km <sup>2</sup> )
34	Ukulunga-AGR (0239700A)	706	9.5	6672
	Pietermaritzburg purification works (0239756AW)	826	47.7	39376
	Camperdown (0240073W)	685	7.9	5398
35	Pietermaritzburg purification works (0239756AW)	826	22.8	18861
	Camperdown (0240073W)	685	9.7	6607
	Nagle (0240185W)	749	8.0	6010
36	Nagle (0240185W)	749	6.7	4995
37	Camperdown (0240073W)	685	8.1	5571
	Nagle (0240185W)	749	38.3	28689
38	Camperdown (0240073W)	685	22.3	15250
39	Camperdown (0240073W)	685	33.1	22687
40	Camperdown (0240073W)	685	28.3	19358
	Nagle (0240185W)	749	0.6	479
	Inchanga (0240284W)	790	7.4	5869
41	Nagle (0240185W)	749	7.3	5454
	Inchanga (0240284W)	790	1.3	996
42	Nagle (0240185W)	749	10.7	7978
	Inchanga (0240284W)	790	8.0	6330

## 6.2 STREAMFLOW SIMULATIONS

Following the data preparation discussed in Chapter 5, model parameters are entered into the HSPF User Control Input (UCI) file and meteorological time-series data are entered into the Watershed Data Management (WDM) file. All the model parameters and related information of these simulation- and utility modules are listed in Annexure C. The various parameters in the UCI were determined according to the possible value ranges and guidelines as stipulated in Chapter 4 and Basins Technical Note 6 (USEPA, 2000).



The modules and their order of execution was set out in the Operational-Sequence (OPN SEQUENCE) Table (Annexure C: 216 & 251), according to the numbering scheme mentioned in Chapter 5. The ACTIVITY Block of each module section is a row of switches, which determine which simulation routines are to be executed. Since only hydrology, and no water quality was simulated in this study, only PWATER section of module PERLND, the IWATER section of module IMPLND and the HYDR section of module RCHRES was activated for each operation.

General descriptions of the pervious- and impervious land segments and reaches or reservoirs were entered in the General Information (GEN-INFO) Tables of PERLND-, IMPLND- and RCHRES modules (Annexure C: 220, 232, 234 & 255, 266, 269).

The PWAT-PARM 1 Table (Annexure C: 222 & 258) consists of a set of flags or switches, and is used to specify whether certain parameters vary monthly or are assumed constant throughout a simulation run. In the land-use categories Crops (2) and Grassland (3), the parameters LZETP and CEPSC were assumed to vary monthly. The monthly values for these land-use categories were entered in tables MON-LZETPARM and MON-INTERCEP (Annexure C: 230, 231 & 264, 265).

In the PWAT-PARM 2 Table (Annexure C: 223 & 258) the different parameters (LZSN, KVARV and AGWRC) were determined according to the possible value ranges and guidelines as stipulated in Chapter 4.



The parameter FOREST applies only to the northern hemisphere coniferous forest, and was set to zero. The average slope of the overland flow plane (SLSUR), which is a function of topography and estimated from GIS-data, indirectly influences the length of LSUR. Therefore lower LSUR values are associated with higher SLSUR values within the range of 0.01 to 0.30 m/m. The LSUR values were initially associated with these SLSUR values of each sub-catchment and then interpolated within the range of 30 to 215 m with the corresponding SLSUR range of 0.01 to 0.30 m/m (USEPA, 2000: 30).

LZSN and INFILT are some of the parameters that determine the annual water balance and were determined by calibration, as discussed in Chapter 4. LSUR and SLSUR are non-calibration model parameters. KVARV is used to represent season variability in groundwater recession rates, and was defaulted to zero.

In the IWAT-PARM 2 Table (Annexure C: 233 & 267) the different parameters (NSUR and RETSC) were determined according to the possible value ranges and guidelines as stipulated in Chapter 4. SLSUR also indirectly influences LSUR as in the case of PWAT-PARM 2. NSUR is a non-calibration model parameter. The criteria for determining LSUR- and SLSUR values are the same as for pervious land segments, except that the LSUR- and SLSUR values are within the range of 15 to 76 m and 0.01 to 0.15 m/m respectively (USEPA, 2000: 31).

In the PWAT-PARM 3 Table (Annexure C: 225 & 261) the different parameters (INFEXP and INFILD) were set according to the possible value ranges and guidelines as stipulated in Chapter 4. DEEPFR was set to zero, since there were no significant dolomitic regions in the study area, and seepage below streamflow

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gauges was considered negligible. BASETP and AGWETP, which represent ET losses from riparian vegetation and groundwater, were assumed non-significant and set to zero.

In IWAT-PARM 3 (Annexure C: 234 & 268) default values were used.

In the PWAT-PARM 4 Table (Annexure C: 228 & 262) the parameters (CEPSC, UZSN, NSUR, INTFW, IRC and LZETP) were initially set to default values according to the possible value ranges and guidelines as stipulated in Chapter 4. UZSN has a minor, and LZETP a major influence on the annual water balance. UZSN, INTFW and IRC were subsequently adjusted by calibration.

PWAT-PARM 5 Table (Annexure C: 230 & 264) was omitted, since defaults were used for all its parameters.

In the MON-INTERCEP Table (Annexure C: 230 & 264) the monthly values of CEPSC are developed according to the results as obtained in PWAT-PARM 4. In the MON-LZETPARM Table (Annexure C: 231 & 265) the monthly values of LZETP are developed using an expected maximum value from the PWAT-PARM 4 Table. These values reflect seasonal changes in ET due to the changes in the density of the vegetation, depth of root zone and the stage of growth.

The PWAT-STATE 1 Table (Annexure C: 232 & 266) is used to set starting conditions at the beginning of a simulation run. The surface related storages (CEPS, SURS and IFWS) were set to zero and allowed to equilibrate during simulation. UZS and LZS were set equal to UZSN and LZSN. Seasonal recession

was not represented, therefore, GWVS is not calculated and was set to zero. Setting AGWS too high or too low will lead to baseflow that is either too high or skewed low. AGWS was therefore initially set to 25 mm (USEPA, 2000: 18-19).

Similarly, the IWAT-STATE 1 Table (Annexure C: 234 & 268) is used to set starting conditions for impervious land segments. The storage of water in retention (RETS) and surface detention storage (SURS) were initially assumed to be zero and allowed to equilibrate during simulation.

In the HYDR-PARM 1 Table (Annexure C: 235 & 269) flags are selected in order to indicate which auxiliary variables must be computed and to specify volume and time-dependent outflow demands for each reach or reservoir. Since most of the auxiliary variables are only used in water quality simulations, the flags selecting their computation were turned off, with the exception of the first flag (AUX1FG), which allowed for the computation of depth, stage, surface area, average depth and top width.

With the exception of Henley Dam, all reaches were assumed to have one volume-dependent outflow. For Henley Dam, three outflows were specified: two volume-dependent outflows, to represent the spillway and downstream releases, and one time-dependent outflow, representing abstractions to the H.D. Hill water purification plant. These abstractions ceased mid-1995. The downstream dam releases were simulated as a volume-dependent outflow, since separate time-series of these releases and the spillway overflows were unobtainable.



The HYDR-PARM 2 Table (Annexure C: 235 & 269) is used to set the parameters LEN (reach length), DELTH (drop in reach elevation) and KS (weighting factor for hydraulic routing). The values determined with GIS were entered for LEN and DELTH. The EPA recommended value of 0.5 was used for KS.

In the HYDR-INIT Table (Annexure C: 235 & 270) the initial conditions for the HYDR section are described. VOL is the initial volume of water in the RCHRES. In this modelling attempt all the reaches, except for Henley Dam, were assumed initially empty and allowed to equilibrate during simulation. An initial storage of 1.4 million m<sup>3</sup> was used for Henley Dam. This corresponds with the dam capacity information as provided by DWAF.

The values calculated from river geometry data with the XSECT program were entered in the FTABLES (Annexure C: 236 & 270).

The External Sources (EXT SOURCES) Table (Annexure C: 244 & 279) is used to specify connections of time series stored in the WDM file and internal simulation modules. The meteorological time series (precipitation and evaporation) used by the PERLND and IMPLND modules, as well as time series representing abstractions or point sources for the RCHRES module are specified in this table.

The WDM data sets are summarised in Table 6.3.

**Table 6.3:** Summary of time series in the Watershed Data Management file

Data set number	Description
1	Elandshoek, Boston (0239097A)
2	Vaucluse (0239133W)
3	Cedara College (0239482W)
4	Edendale (0239518W)
5	Pietermaritzburg purification works (0239577W)
6	Baynesfield Estates (0239585A)
7	Allerton (0239604W)
8	Ukulinga- AGR (0239700W)
9	Pietermaritzburg purification works (0239756W)
10	Camperdown (0240073W)
11	Nagle (0240185W)
12	Inchanga (0240284W)
13	Evaporation region No. 1 (Annual mean of 1664 mm)
14	Evaporation region No. 2 (Annual mean of 1708 mm)
15	Evaporation region No. 3 (Annual mean of 1679 mm)
600	Darvill wastewater treatment plant (inflow)
611	U2H011 Msunduzi River at Henley Dam
622	U2H022 Msunduzi River at Nomfihlelo
641	U2H041 Msunduzi River at Hampstead Park, Moto-X
658	U2H058 Msunduzi River at Mason's Mill
692	Henley Dam abstractions
800-841	Hydrological simulation output, Sub-catchments 1-42

The multiplication-factor column in the EXT SOURCES Table (Annexure C: 244 & 279) is the factor by which the precipitation- and evaporation data will be multiplied before being sent to the target operation. In the case of precipitation these factors correspond with the percentage distribution of the area influenced by each precipitation station relative to the area of the sub-catchments of concern (see Table 6.2). The multiplication factors of the A-pan evaporation data were set to correct to open water equivalent evaporation.

Effective impervious area (EIA) as a function of land-use is also an important aspect worth mentioning, because the effect of development and impervious area on increased runoff and loss of recharge are well known. Therefore the main land-use groups were divided into effective impervious areas as follows (Beyerlein, 2000):

- |                               |     |
|-------------------------------|-----|
| □ CBD and industrial:         | 86% |
| □ Medium-density residential: | 10% |
| □ High-density residential:   | 26% |

These residential EIA values assume that not all roof runoff is tight-lipped to a stormwater drainage system (Beyerlein, 2000).

The SCHEMATIC Block (Annexure C: 246 & 281) is used to specify connections of flow between operating modules. For example, connections between PERLND-, IMPLND- and RCHRES modules to represent runoff into streams, and RCHRES-to-RCHRES connections to specify the stream network, are specified in this table, with a multiplication factor where necessary.

The MASS-LINK Table (Annexure C: 249 & 285) is used in combination with the SCHEMATIC Block to specify flows of individual constituents between operating modules. Since only hydrological simulations were performed, the entries in this table reflect only the connections of water flows from land segments into reaches, and from upstream to downstream reaches.

The NETWORK Block (Annexure C: 250) can be used to specify individual time series connections, which cannot be easily done with the MASS-LINK- and SCHEMATIC Blocks. Output of simulated flows into the WDM file is specified in the External Targets (EXT TARGETS) Block.



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## 6.3 RESULTS OF STREAMFLOW CALIBRATION

The model was calibrated at a daily level against streamflow measured at the four hydrological gauging stations described in Chapter 5. Hydrological calibration proceeded along the lines described in Chapter 4.

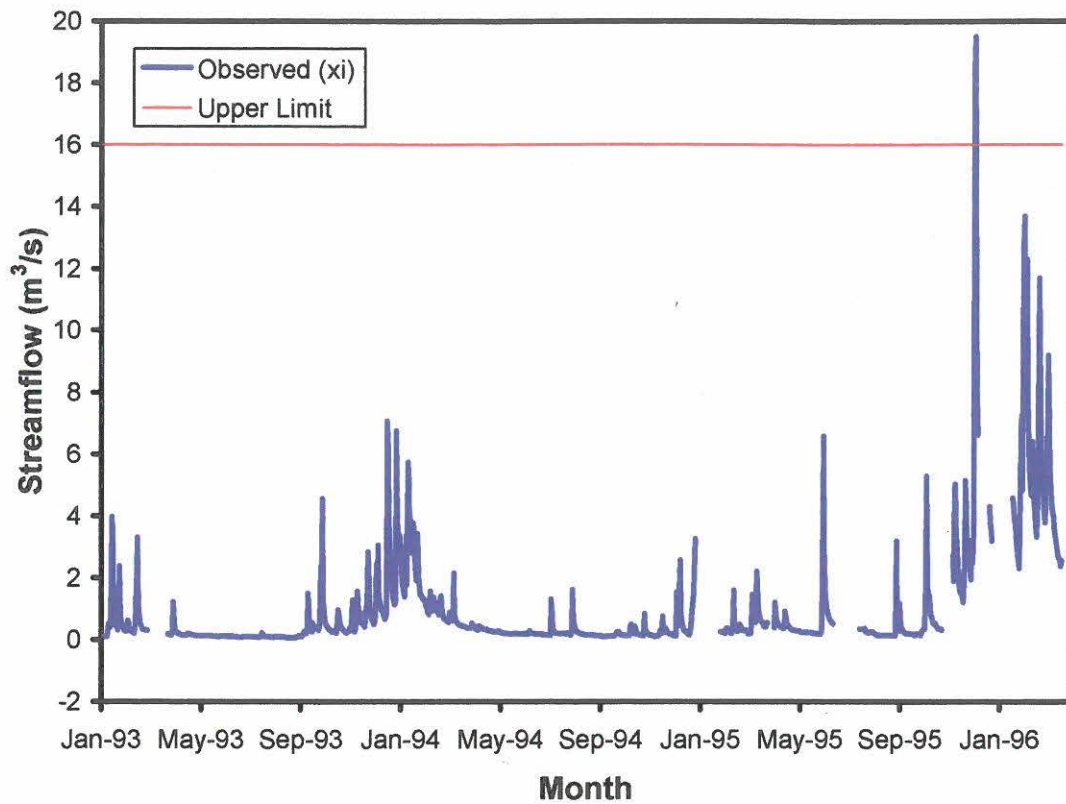
### 6.3.1 Observed Streamflow Data

According to the various rating curves of each hydrological gauging station provided by DWAF (Hydrology), streamflow measurements are only accurate below the upper limit of accurately measured data of each station. Thus, any part of the hydrographs above this limit should be discounted.

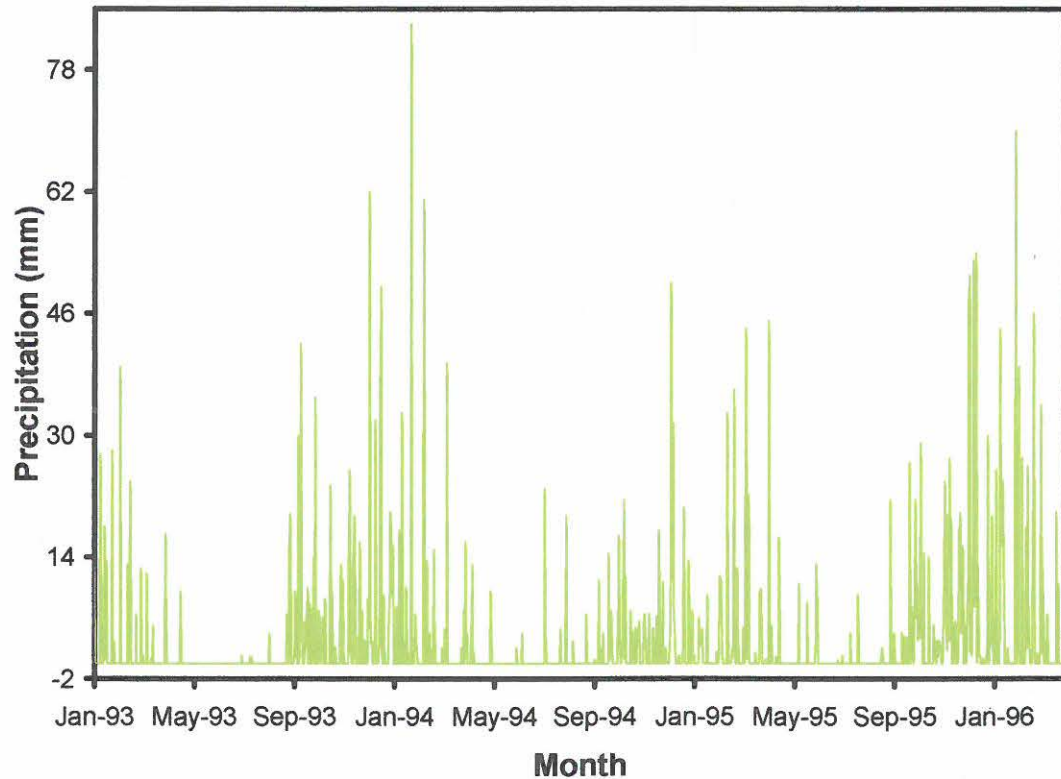
#### *U2H011, Msunduzi River at Henley Dam:*

This hydrological gauging station (sharp-crested gauging weir) represents the upper sub-catchments one to five with a three-year period of available data, 1993 to 1996. Henley Dam is upstream of U2H011. Streamflow measurements mainly represent dam releases and occasional spillway overflow and are accurate to flows of approximately  $16 \text{ m}^3/\text{second}$ .

These sub-catchments encompass 19.7% of the entire catchment and only 0.3% is impervious land. Grassland, undefined open spaces and low-density residential areas dominate. The average slopes ( $0.138 \text{ m/m}$ ) contribute to moderate runoff, but the land-use, pervious land, routing of more water through the subsurface as interflow and high interception storage capacities retain much of the runoff. Henley Dam attenuates any natural factors, which could affect the hydrograph shape. The observed daily hydrograph of U2H011 and the corresponding precipitation data of Vaucluse (0239133W) are shown respectively in Figure 6.1 and 6.2.



**Figure 6.1:** Observed daily hydrograph: U2H011: Msunduzi River at Henley Dam. Hydrological years: 1993-1996



**Figure 6.2:** Observed precipitation data: Vacluse (0239133W). Period of record: 1993-1996

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*U2H058, Msunduzi River at Mason's Mill:*

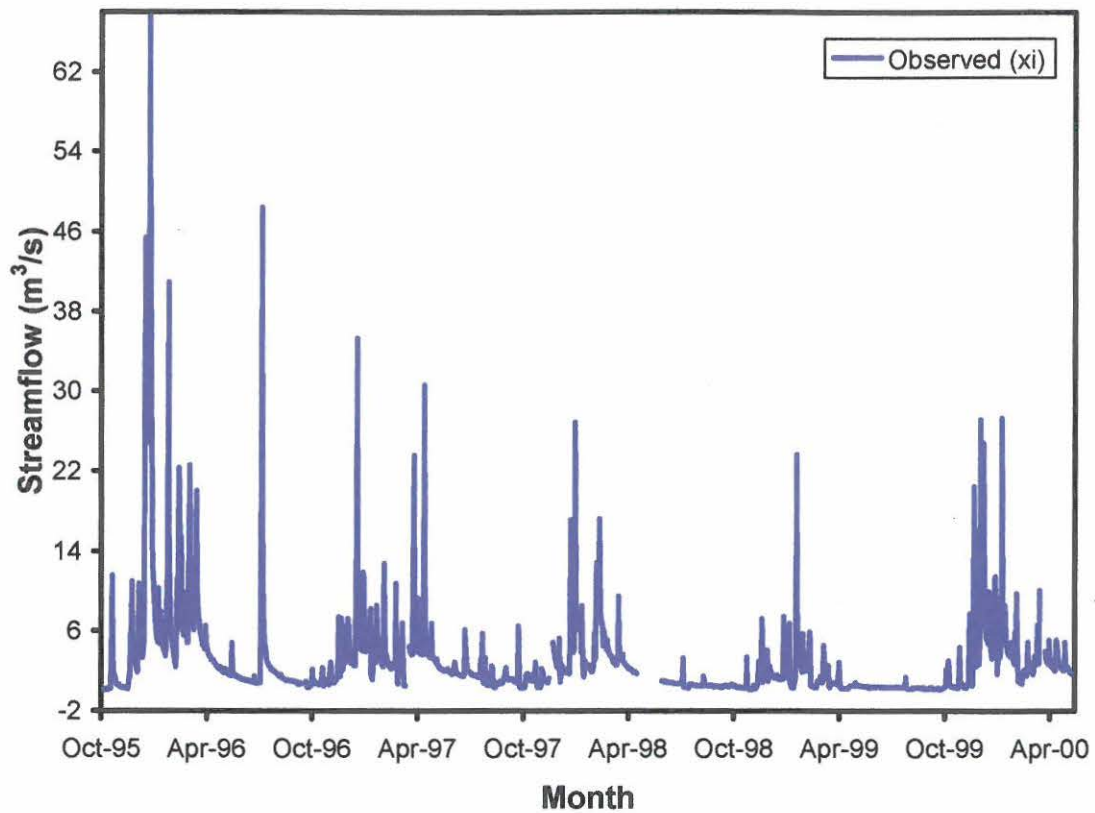
Sub-catchments six to 14 contribute the streamflow to this hydrological gauging station. Data are available for a five-year period, 1995 to 2000. Streamflow measurements at this V-crump/ horizontal-crump gauging weir are accurate to flows of 336.6 m<sup>3</sup>/second.

These sub-catchments cover 16.8% of the entire catchment. The percentage impervious land, namely 5.9% is higher than at U2H011. The average slopes are also steeper (0.217 m/m) and grassland, undefined open spaces, rural-urban transitions, medium-density residential areas and forest dominate.

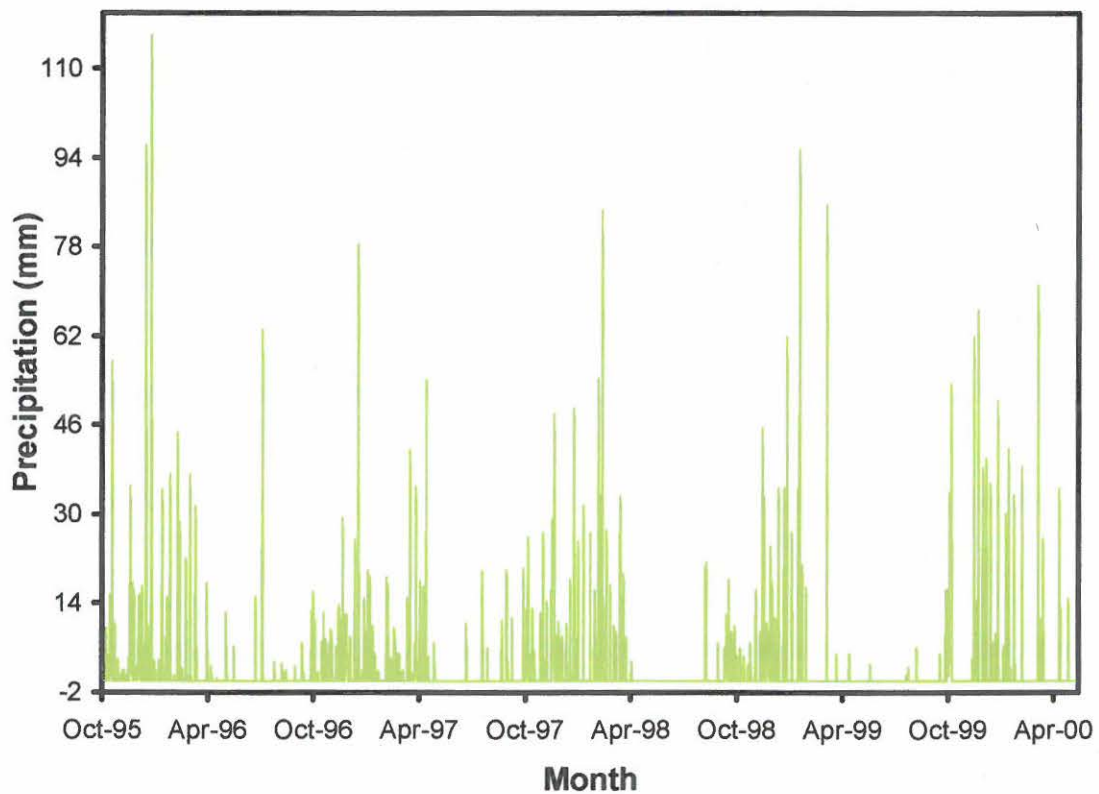
All these factors help to explain the nature of U2H058's hydrograph. The steeper average slopes and higher percentage of impervious land contribute to quick runoff, resulting in the sharper rising- and descending limbs of the daily storm hydrographs.

The observed daily hydrograph of U2H058 is shown in Figure 6.3. The precipitation data of Edendale (0239518W) has the largest influence on the precipitation-distribution and are shown in Figure 6.4 for the corresponding period. The contributions of the other four precipitation stations (0239097A, 0239482W, 0239577W and 0239585A) are insignificant.





**Figure 6.3:** Observed daily hydrograph: U2H058: Msunduzi River at Mason's Mill. Hydrological years: 1995-2000



**Figure 6.4:** Observed precipitation data: Edendale (0239518W). Period of record: 1995-2000

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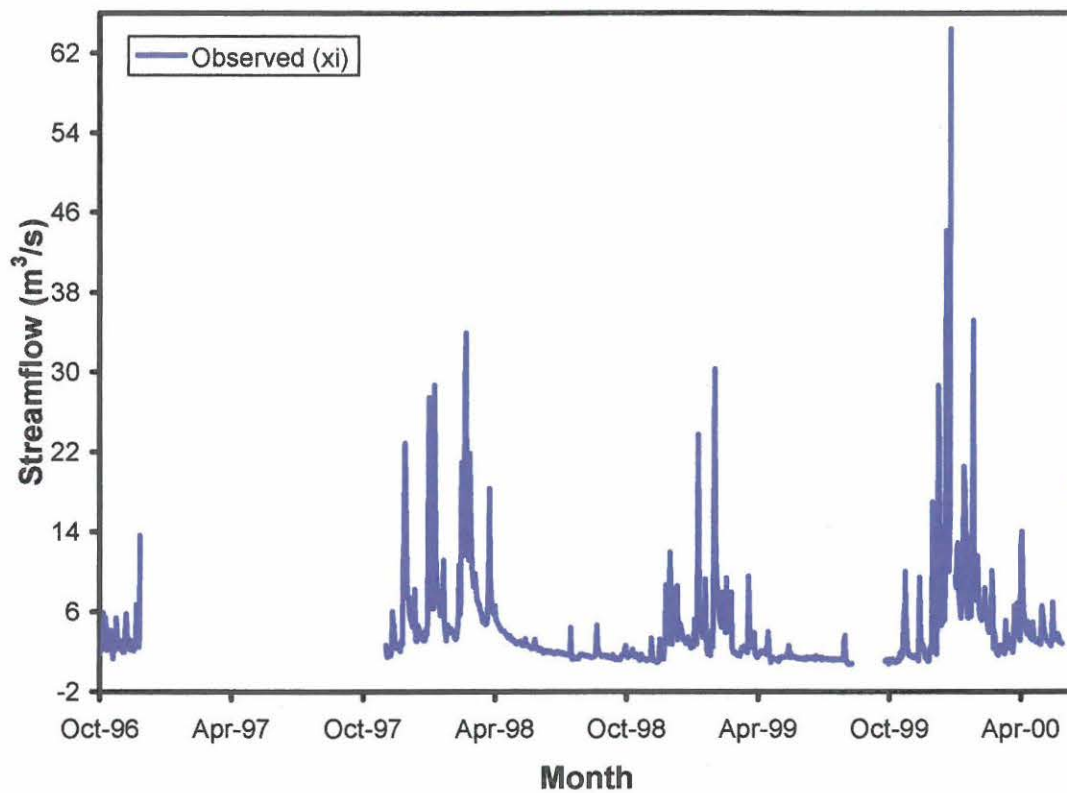
*U2H041, Msunduzi River at Hampstead Park, Moto-X:*

The single notch V-crump gauging weir at this hydrological gauging station can measure the streamflow, contributed by sub-catchments 15 to 30 accurately up to 106.9 m<sup>3</sup>/second. Data are only available for a three-year period, 1997 to 2000.

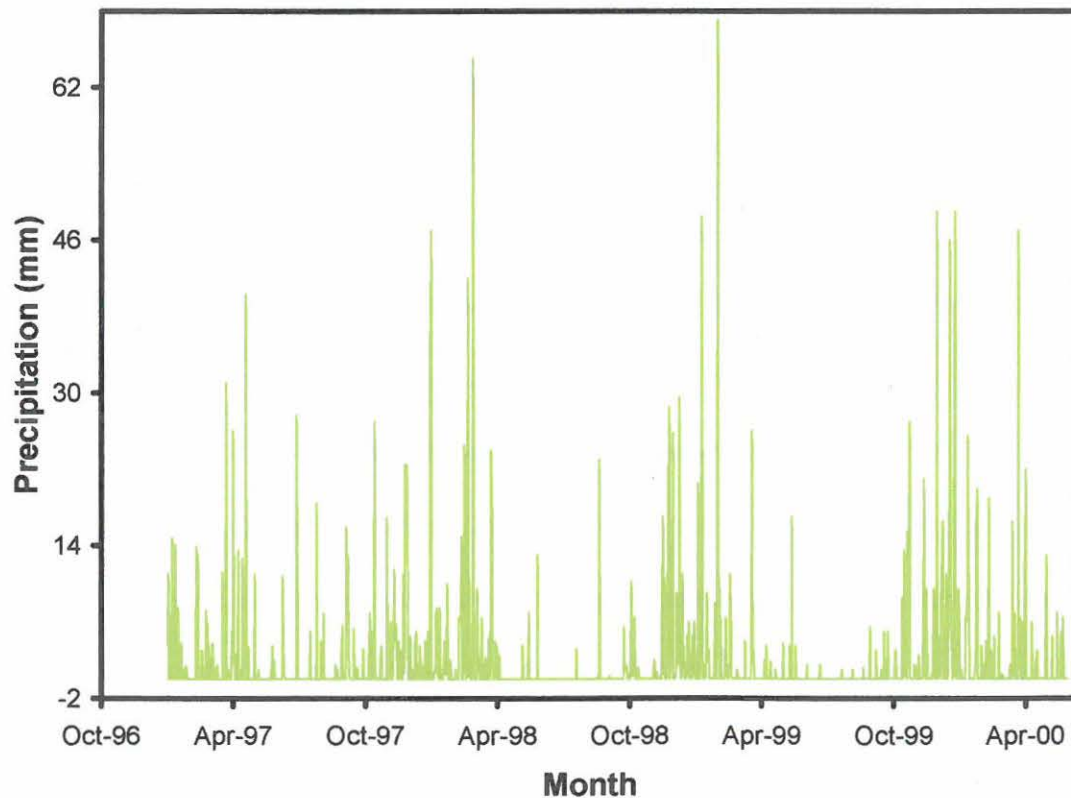
These sub-catchments encompass 23.3% of the entire catchment and the percentage of impervious land is also the highest, namely 11.2%. The sub-catchments above U2H041 have shallow slopes with an average of 0.088 m/m. Grassland, rural-urban transitions, medium-density residential areas, CBD and industrial areas and forest dominate.

The shallow slopes, larger catchment area and longer travel time to the river ensure that the hydrograph varies smoothly over time. On the other hand, the higher percentage of impervious land contributes to quick runoff response with sharper rising- and descending limbs of the daily storm hydrographs during single-event storms. Furthermore the seasonal variation of total wastewater discharge from the Darvill wastewater treatment plant in sub-catchment 30 can affect the natural conditions in this sub-catchment and thus the hydrographs. Although insignificant, it was taken into consideration.

The observed daily hydrograph of U2H041 is shown in Figure 6.5. The precipitation stations, Ukulinga-AGR (0239700W) and Pietermaritzburg purification works (0239756W) have the biggest influence on the precipitation-distribution in sub-catchments 15 to 30. The precipitation data for the corresponding period are shown in Figures 6.6 and 6.7.

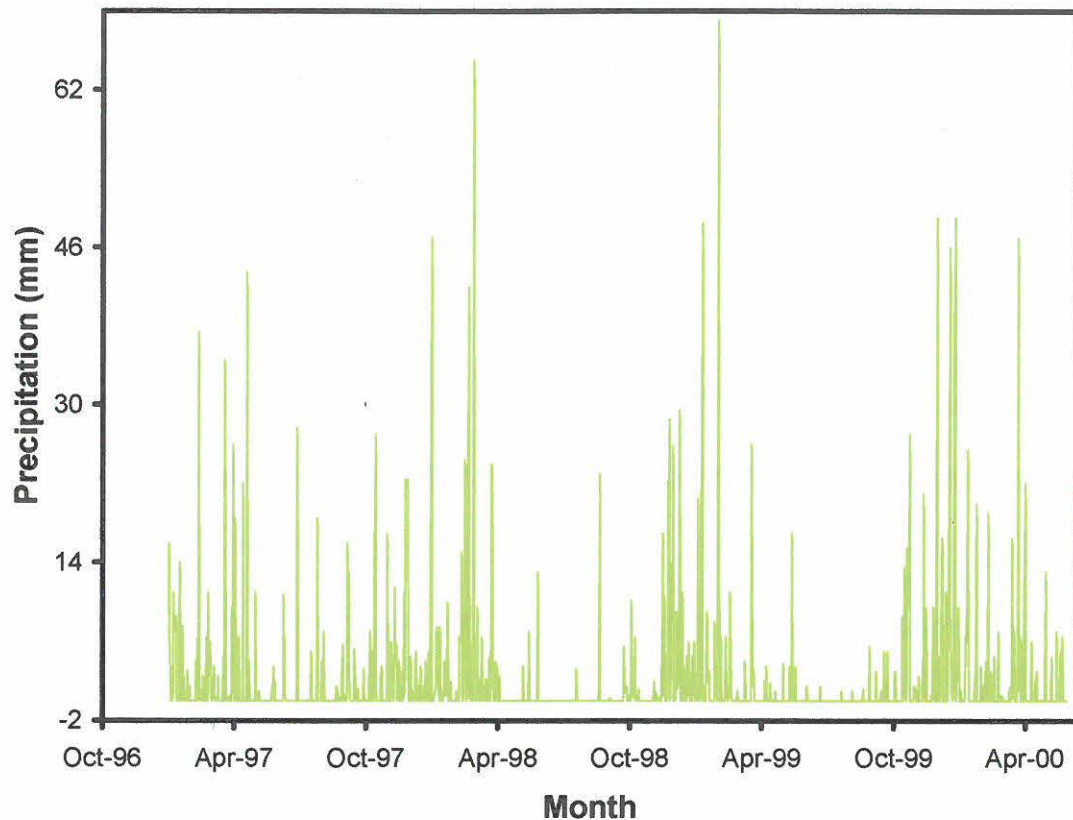


**Figure 6.5:** Observed daily hydrograph: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological years: 1997-2000



**Figure 6.6:** Observed precipitation data: Ukulinga-AGR (0239700W). Period of record: 1997-2000





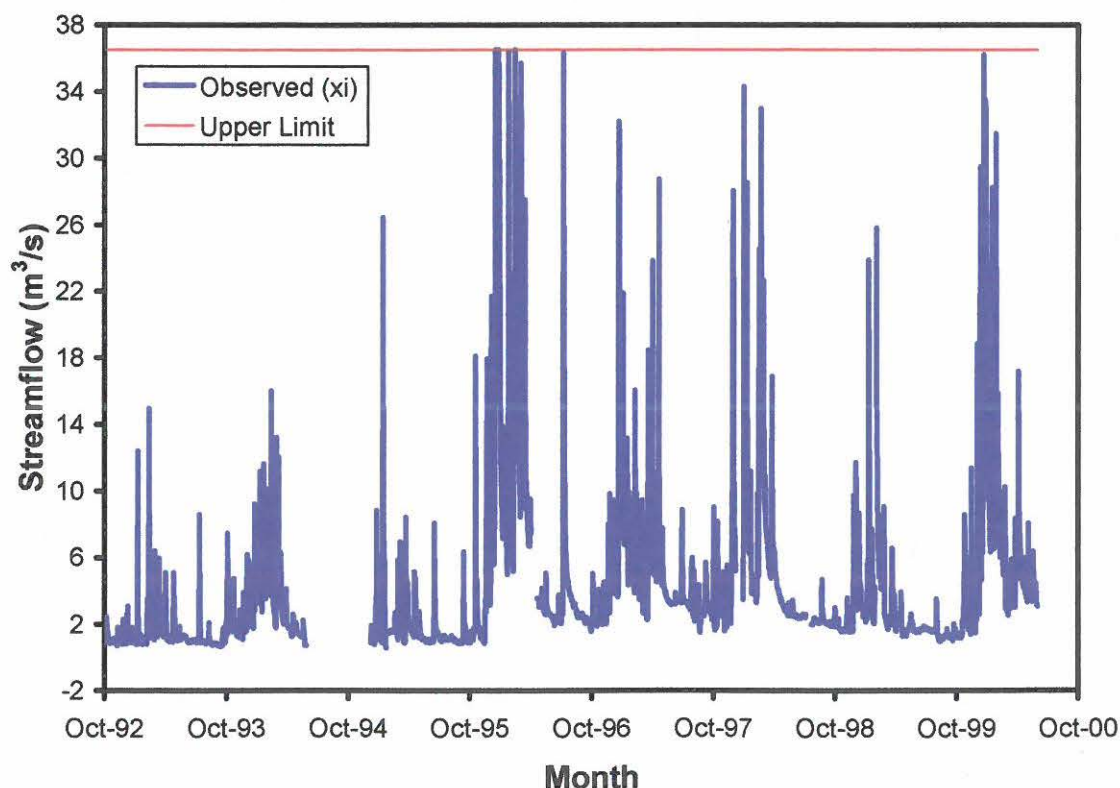
**Figure 6.7:** Observed precipitation data: Pietermaritzburg purification works (0239756W). Period of record: 1997-2000

*U2H022, Msunduzi River at Nomfihlelo:*

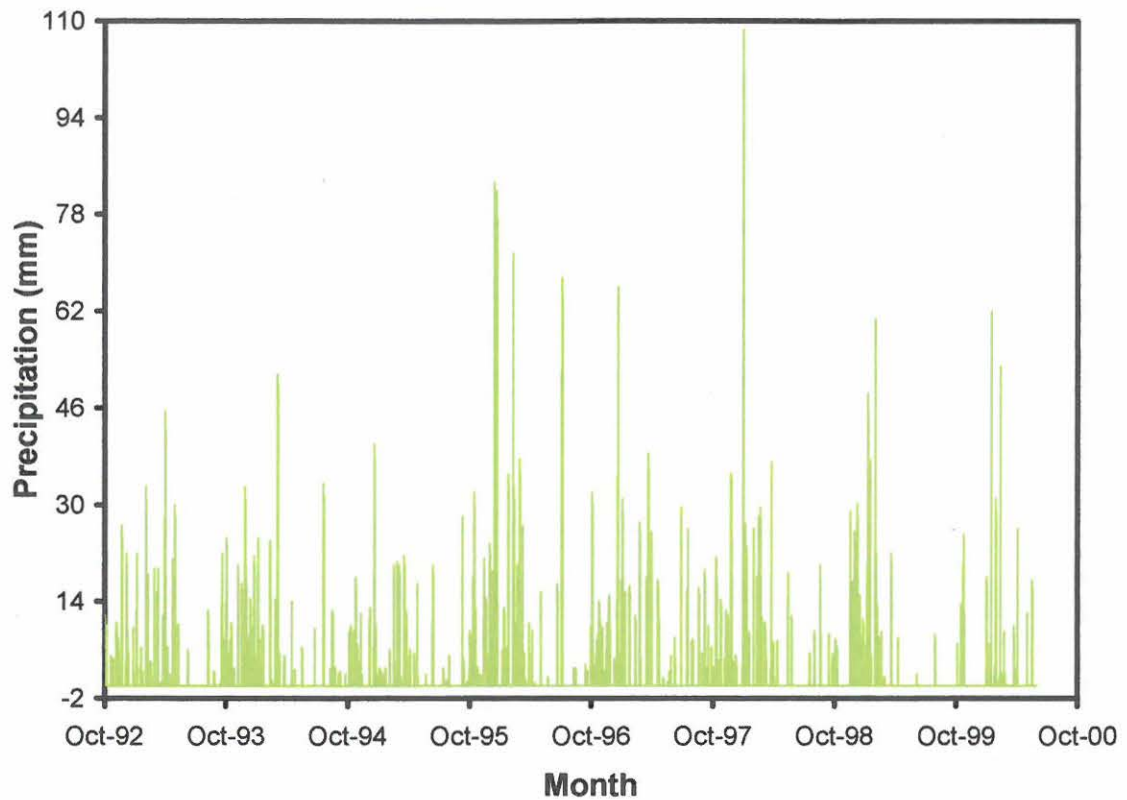
Sub-catchments 31 to 42 contribute the runoff and therefore streamflow to this hydrological gauging station. Compared to the other hydrological gauging stations, the best observed data set (1992 to 2000) was available at this station. This multiple gauging weir consists of a hydro flume and three broad crest notches. Streamflow measurements are characterised by the absence of submergence data and therefore the structure can only be calibrated to flows of approximately  $36.5 \text{ m}^3/\text{second}$ . Even at this streamflow, inaccuracies are obvious as the effects of submergence would no doubt already be evident. Any part of the hydrograph above the upper limit of measured data should be discounted.

These sub-catchments cover 40.2% of the lower Msunduzi River Catchment. The amount of impervious land is 0.5%. Shallow to steep slopes (0.129 m/m), forest, open spaces, cropping and grassland are typical sub-catchment characteristics. All above-mentioned factors, as in the case of U2H041, ensure that the hydrograph varies smoothly over time.

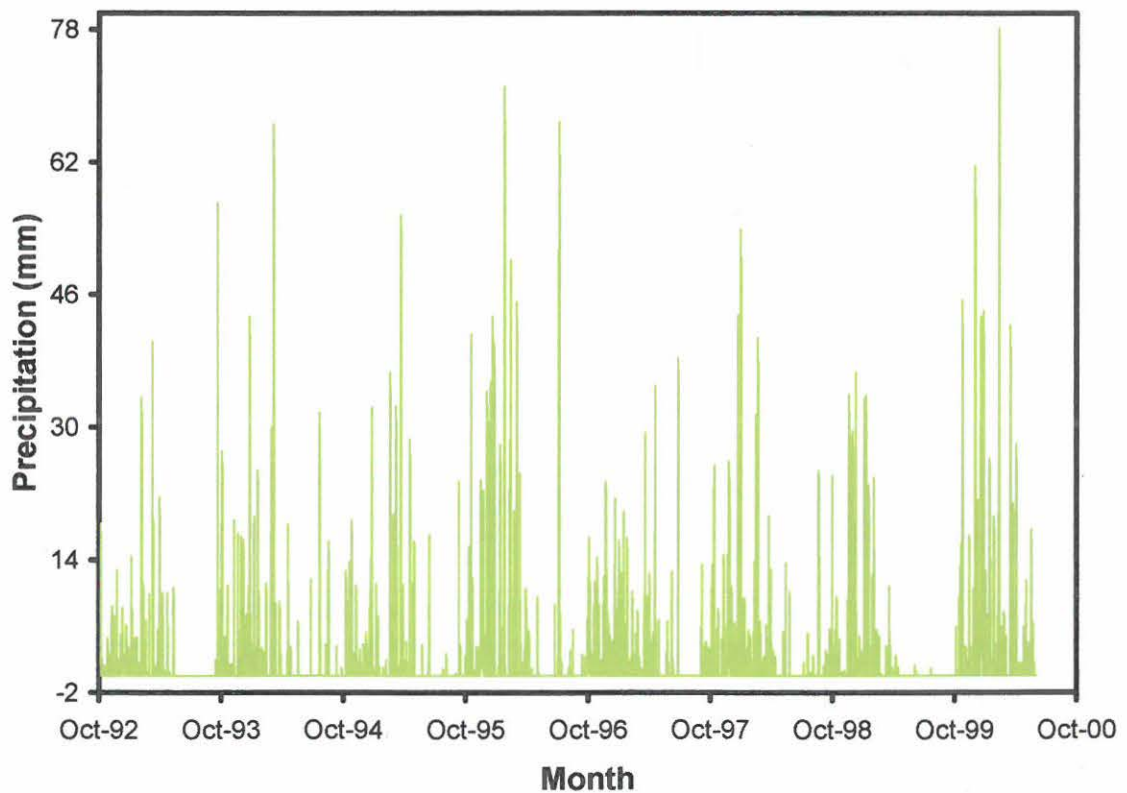
Among all the stations, it can be argued that U2H022 is most representative of the natural conditions in the entire Msunduzi River Catchment, because there are no significant human-induced hydrological alterations present in these sub-catchments. The hydrograph at U2H022 closely mimics the hydrograph at U2H041, with sharp rising- and descending limbs of the daily storm hydrographs. The observed daily hydrograph of U2H022 is shown in Figure 6.8. The precipitation data of Camperdown (0240073W) and Nagle (0240185W) are shown in Figures 6.9 and 6.10.



**Figure 6.8:** Observed daily hydrograph: U2H022: Msunduzi River at Nomfihlelo. Hydrological years: 1992-2000



**Figure 6.9:** Observed precipitation data: Camperdown (0240073W). Period of record: 1992-2000



**Figure 6.10:** Observed precipitation data: Nagle (0240185W). Period of record: 1992-2000



### 6.3.2 Comparison of Observed Data and Simulated Streamflow Values

The period of calibration corresponded with the period of availability of catchment- (topography, geology and land-use), hydrological- (streamflow, human water use and wastewater treatment plants) and meteorological data (precipitation and evaporation). The periods used for the calibration of the four hydrological gauging stations are summarised in Table 6.4.

**Table 6.4:** Summary of calibration data

Station description (DWAf number & name)	Period of calibration
U2H011 Msunduzi River at Henley Dam	1993/02/01-1996/03/31
U2H058 Msunduzi River at Mason's Mill	1995/10/01-1998/09/31
U2H041 Msunduzi River at Hampstead Park, Moto-X	1996/10/01-1999/09/31
U2H022 Msunduzi River at Nomfihlelo	1992/10/01-1998/09/31

In the Msunduzi River Catchment, only three of the main land-use groups are thought to contribute runoff directly to the river. The CBD and industrial, medium-density residential and high-density residential land-use groups are therefore simulated as EIA as discussed earlier. The percentage-distribution of these EIA as a percentage of the total area of impervious land is summarised in Table 6.5.

**Table 6.5:** Summary of Effective Impervious Areas (EIA)

Main land-use group	EIA (km <sup>2</sup> )	Percentage-distribution (%)
CBD and industrial	22	63
Medium-density residential	7	20
High-density residential	6	17
<b>Total:</b>	<b>35</b>	<b>100</b>

The AGWRC parameters varied between 0.93 and 0.99 throughout the catchment. Values for the AGWRC parameters were determined by comparing the slopes of semi-logarithmic plots of the observed streamflow data and simulated streamflow values during the baseflow period (winter months) at the different hydrological gauging stations. According to Johanson (1997) HSPF assumes the groundwater reservoir is “linear” and since AGWRC represents the ratio of today’s flow divided by yesterday’s flow, semi-logarithmic plots will result in a straight line, because the flow is decreasing exponentially.

The INFILT parameter varied between 0.6 and 1.0 throughout sub-catchments one to 42. The higher values used on sub-catchments six to 42 could account for the over-simulation of the baseflow period. Since INFILT also has an influence on the seasonal runoff distribution, it is possible that, together with IRC and AGWRC it might be responsible for the high autumn baseflow in these sub-catchments.

The LZSN parameter varied throughout the catchment. Lower values were used in sub-catchments six to 14 in order to increase the annual runoff. In the other sub-catchments, higher values were used to increase the actual ET, thus decreasing the annual water balance.

Lower values of UZSN at the dominant land-use groups (forest and grassland) were used in sub-catchments one to five in order to increase the peak flows of the rising limb of the hydrograph at U2H011. In the rest of the catchment, higher UZSN values were used at the dominant land-use groups in order to retain more water in the upper zone and increase the amount available for ET.

The AGWRC parameters varied between 0.93 and 0.99 throughout the catchment. Values for the AGWRC parameters were determined by comparing the slopes of semi-logarithmic plots of the observed streamflow data and simulated streamflow values during the baseflow period (winter months) at the different hydrological gauging stations. According to Johanson (1997) HSPF assumes the groundwater reservoir is “linear” and since AGWRC represents the ratio of today’s flow divided by yesterday’s flow, semi-logarithmic plots will result in a straight line, because the flow is decreasing exponentially.

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LZETP values were selected in accordance with the land-use groups, as discussed in Chapters 4 and 5.

During the final stage of streamflow calibration at a daily level, the individual hydrographs are calibrated to best match response time, maximum peakflows and recession rate. The most important adjustments were made to the parameters INTFW and IRC.

The final calibrated values of INTFW in the catchment range from 1.0 to 3.0 for the various main land-use groups. These relatively high INTFW values help to better match the hydrograph peaks, while maintaining a reasonable amount of runoff volume. INTFW is also responsible for the division of water between interflow and surface processes, thus influencing the timing of the simulated hydrographs.

Various IRC values, recommended in the literature, were used with little or no effect, especially with respect to the autumn baseflow. The simulation results varied; simulated interflow recession was either too slow or accurate. A constant IRC value of 0.7 gave the best results overall.

The over-simulation of single storm events (higher streamflow peaks) was the most consistent error that occurred during the period of calibration. This is likely due to the poor representation and areal distribution of precipitation data, which did not account accurately for the spatial variation in precipitation and storm distributions.

*U2H011, Msunduzi River at Henley Dam:*

The daily calibration at U2H011 was better than the calibration at U2H058. The variations in the simulation results were indicative that there is no model bias.

The wet period was under-simulated by 7%, thus reflecting an overall under-simulation of streamflow peaks, especially during the hydrological year 1994 to 1995. High streamflow peaks were simulated for the period of 1995/12/21 to 1996/02/01, but there were no observed data available for this period. The dry period was over-simulated by 35%.

The observed data at U2H011 are also questionable, especially considering all the missing data during the period of calibration. Another factor complicating the accurate simulation of U2H011 was the simplifying assumptions made about downstream dam releases from Henley Dam. The final calibrated values of the five parameters, which determine the seasonal and annual water balance in sub-catchments one to five are listed in Table 6.6.

**Table 6.6:** Summary of seasonal and annual water balance parameters in sub-catchments one to five

Main land-use group	LZETP	UZSN (mm)	LZSN (mm)	AGWRC	INFILT (mm/h)
Forest	0.80	19	380	0.99	0.60
Open spaces	0.60	17			
Grassland	0.50	6			
CBD and industrial	0.10	3			
Medium-density residential	0.20	7			
Low-density residential	0.35	10			
High-density residential	0.15	4			
Wetland	0.75	10			

In the hydrological year 1993 to 1994 the timing of the hydrograph was incorrectly shifted to a later date, but the overall period of calibration at U2H011 must be taken into consideration. Overall, interflow recession was correctly simulated throughout the period of calibration.

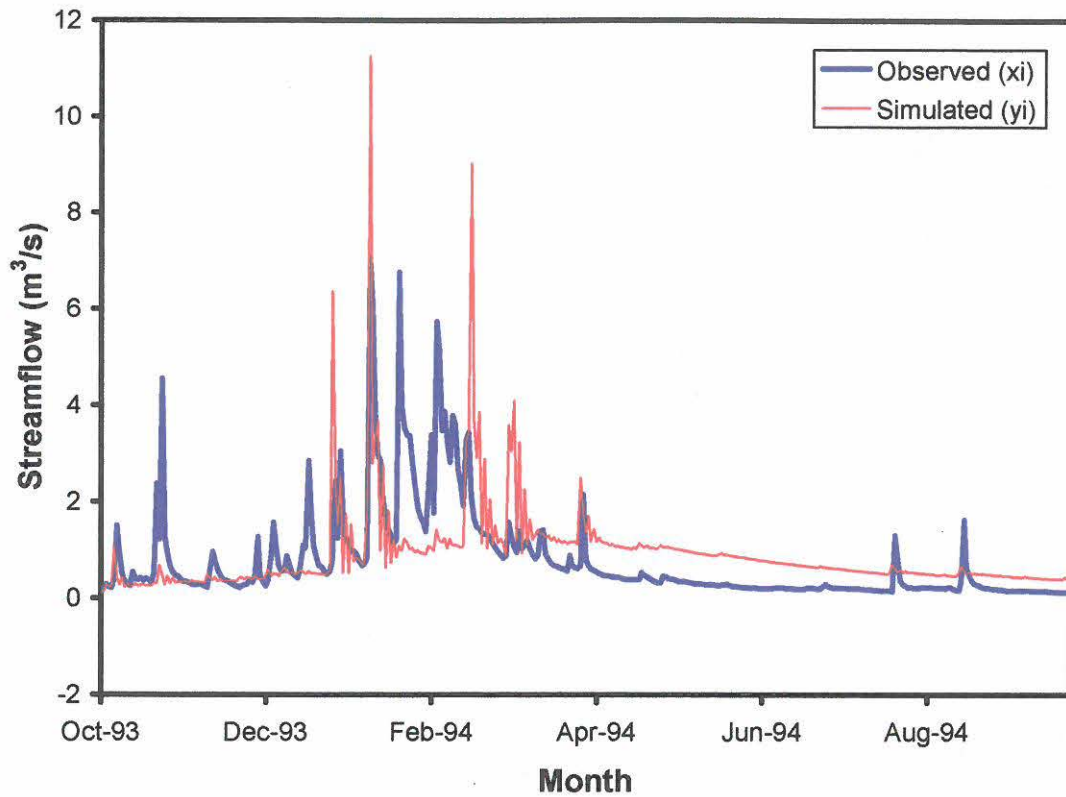
The final calibrated values for INTFW and IRC are listed in Table 6.7.

**Table 6.7:** Summary of hydrograph shape parameters in sub-catchments one to five

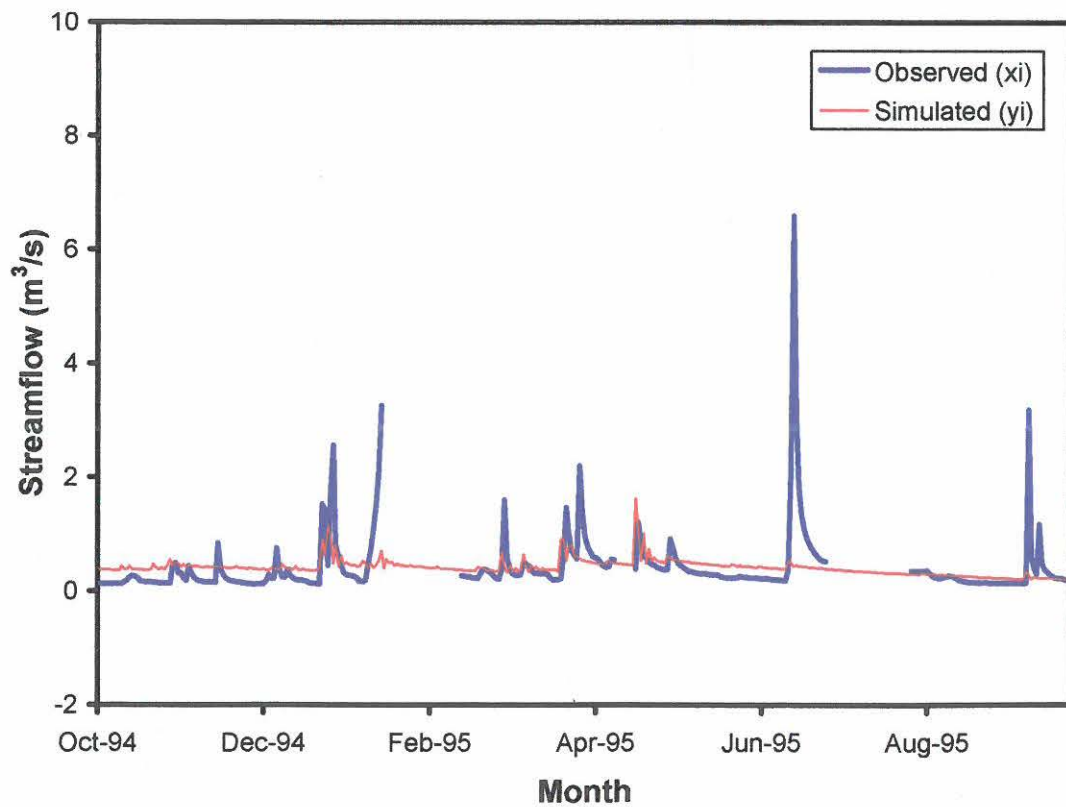
Main land-use group	INTFW	IRC
Forest	2.5	0.7
Open spaces	3.0	
Grassland	3.0	
CBD and industrial	1.5	
Medium-density residential	1.5	
Low-density residential	1.8	
High-density residential	1.0	
Wetland	2.0	

The comparison of observed- and simulated daily streamflow at U2H011 for each hydrological year is shown in Figures 6.11 to 6.13.

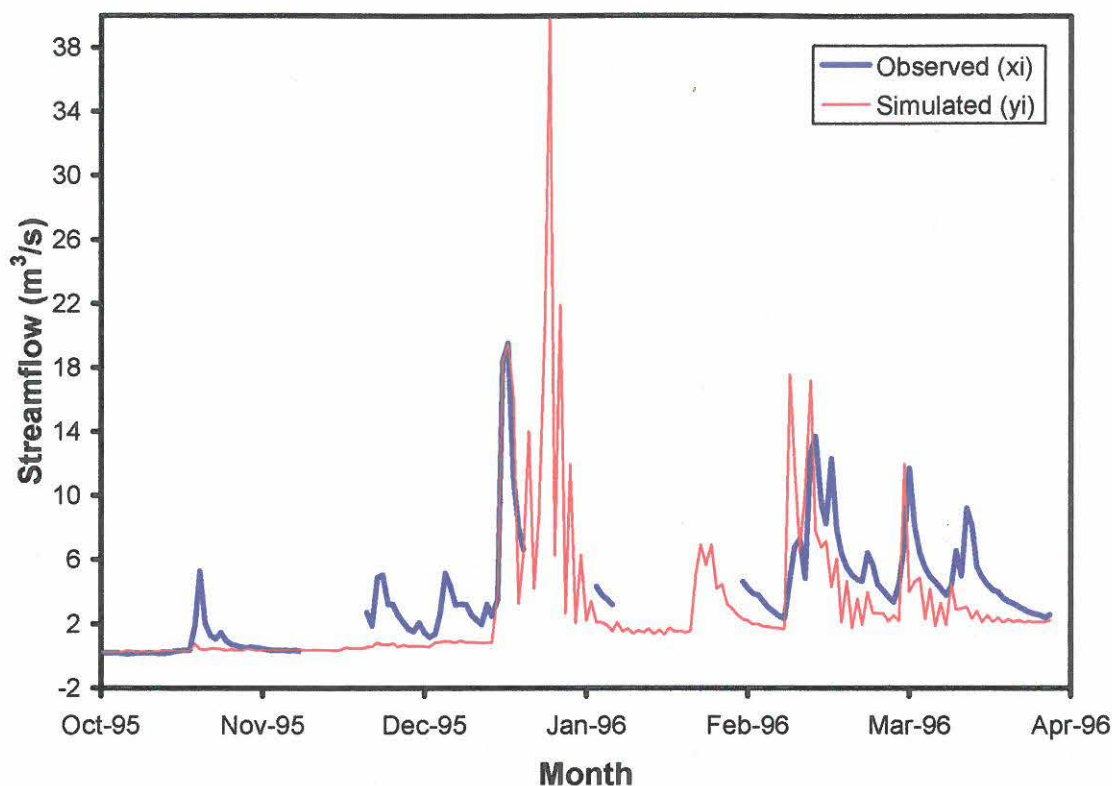




**Figure 6.11:** Comparison of observed and simulated daily streamflow: U2H011: Msunduzi River at Henley Dam. Hydrological year: 1993-1994



**Figure 6.12:** Comparison of observed and simulated daily streamflow: U2H011: Msunduzi River at Henley Dam. Hydrological year: 1994-1995



**Figure 6.13:** Comparison of observed and simulated daily streamflow: U2H011: Msunduzi River at Henley Dam. Hydrological year: 1995-1996

*U2H058, Msunduzi River at Mason's Mill:*

The daily calibration at U2H058 was the poorest among the four hydrological gauging stations. The wet period was under-simulated by 13%. Only on single occasions, the streamflow peaks were over-simulated during the hydrological years 1995 to 1996 (1995/12/26; 1996/07/08) and 1997 to 1998 (1998/02/22 - 1998/02/24).

The dry period was over-simulated by 9%. The baseflow was generally correctly simulated, but at some single events it was either over- or under-simulated. Overall, the annual water balance at U2H058 was slightly under-simulated.

The representation and areal distribution of the precipitation stations used in the hydrological simulation of sub-catchments six to 14 is better than at U2H011, however the assumption of uniform precipitation has an influence on the simulation results.

The final calibrated values of the five parameters, which determine the seasonal and annual water balance in sub-catchments six to 14 are listed in Table 6.8.

**Table 6.8:** Summary of seasonal and annual water balance parameters in sub-catchments six to 14

Main land-use group	LZETP	UZSN (mm)	LZSN (mm)	AGWRC	INFILT (mm/h)
Forest	0.80	29	280	0.95	1.00
Open spaces	0.60	17	280		
Grassland	0.50	17	280		
CBD and industrial	0.10	3	280		
Medium-density residential	0.20	7	280		
Low-density residential	0.40	10	280		
High-density residential	0.15	4	280		
Wetland	0.75	10	280		

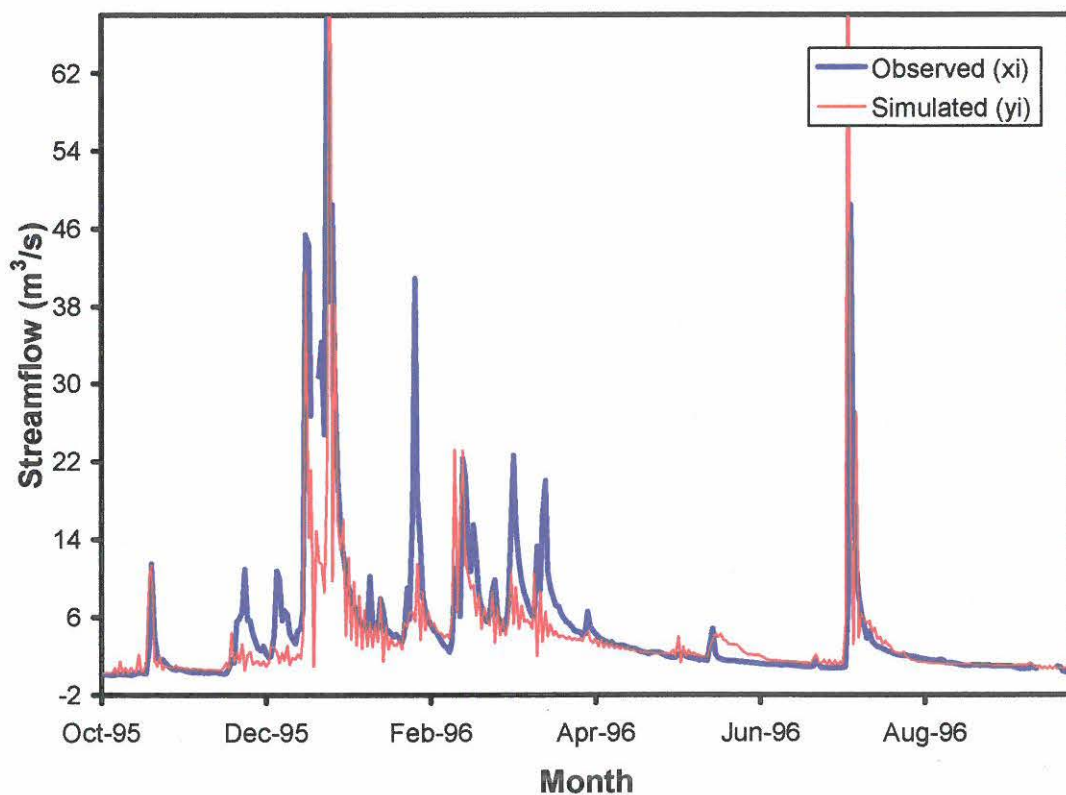
**Table 6.9:** Summary of hydrograph shape parameters in sub-catchments six to 14

Main land-use group	INTFW	IRC
Forest	2.5	0.7
Open spaces	3.0	
Grassland	3.0	
CBD and industrial	1.5	
Medium-density residential	1.5	
Low-density residential	1.8	
High-density residential	1.0	
Wetland	2.0	

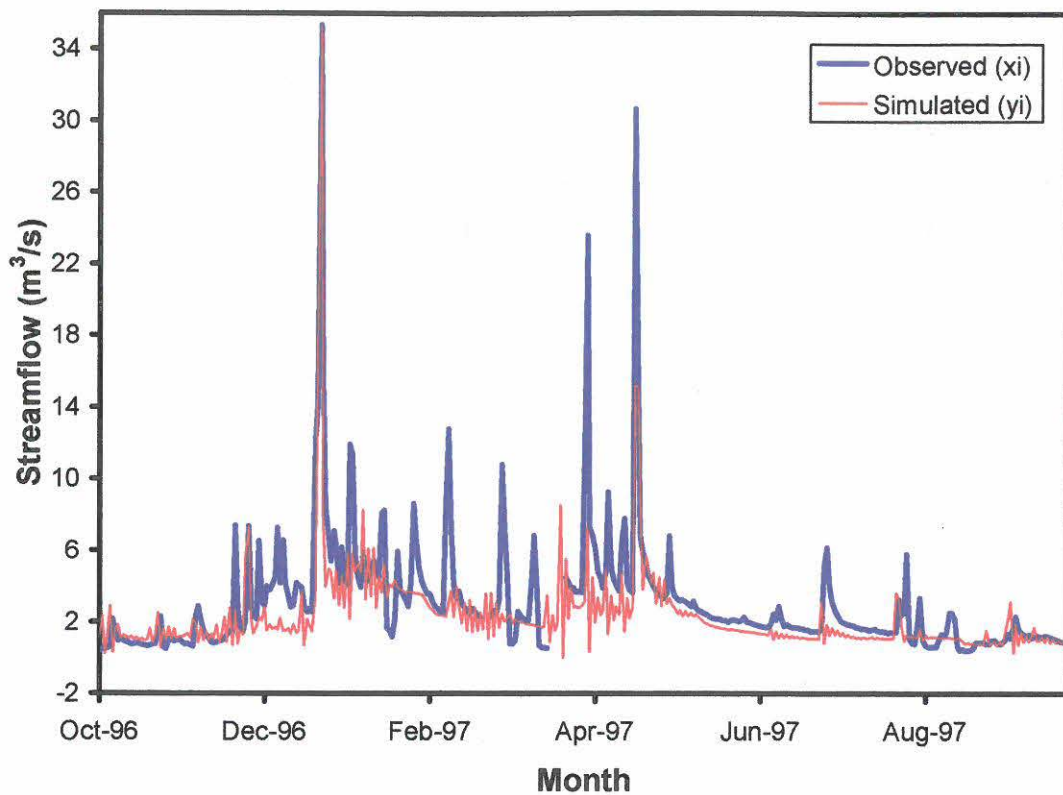


The timing of the simulated hydrographs was correctly simulated throughout the period of calibration. Overall, the simulated interflow recession corresponds with the observed data.

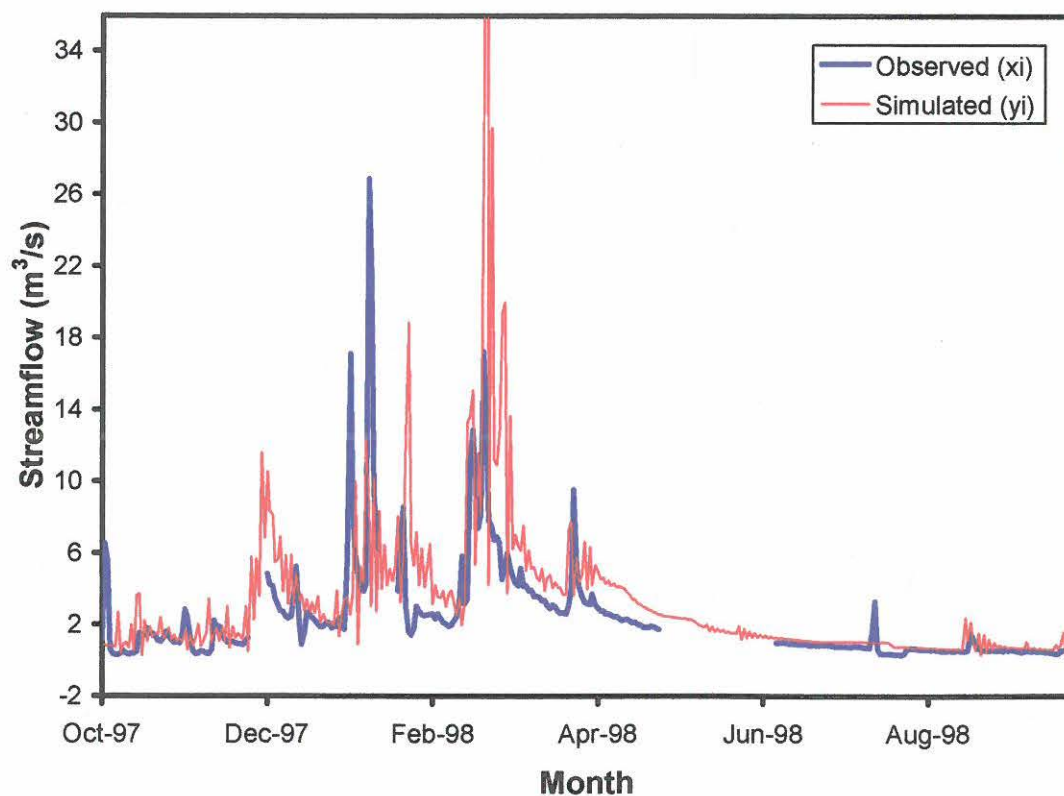
The comparison of observed- and simulated daily streamflow at U2H058 for each hydrological year is shown in Figures 6.14 to 6.16.



**Figure 6.14:** Comparison of observed and simulated daily streamflow: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1995-1996



**Figure 6.15:** Comparison of observed and simulated daily streamflow: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1996-1997



**Figure 6.16:** Comparison of observed and simulated daily streamflow: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1997-1998

### *U2H041, Msunduzi River at Hampstead Park, Moto-X:*

The wet period was over-simulated by 24%, characterised by the over-simulation of single storm events. The dry period was over-simulated by 5%. The baseflow agreed well with the observed data, except during the first part of the hydrological year 1998 to 1999.

The timing of the simulated hydrographs was accurately simulated throughout the period of calibration; thus the final INTFW values were well calibrated. The hydrographs at U2H041 and U2H058 mimic each other, although the simulated streamflow values at U2H041 are higher overall, and the annual water balance at U2H041 is over-simulated. The final calibrated values of the five parameters, which determines the seasonal and annual water balance in sub-catchments 15 to 30 are listed in Table 6.10.

**Table 6.10:** Summary of seasonal and annual water balance parameters in sub-catchments 15 to 30

Main land-use group	LZETP	UZSN (mm)	LZSN (mm)	AGWRC	INFILT (mm/h)
Forest	0.80	29	300	0.93	1.00
Open spaces	0.60	17	300		
Grassland	0.50	17	300		
CBD and industrial	0.10	3	300		
Medium-density residential	0.20	7	300		
Low-density residential	0.40	10	300		
High-density residential	0.15	4	300		
Wetland	0.75	10	300		

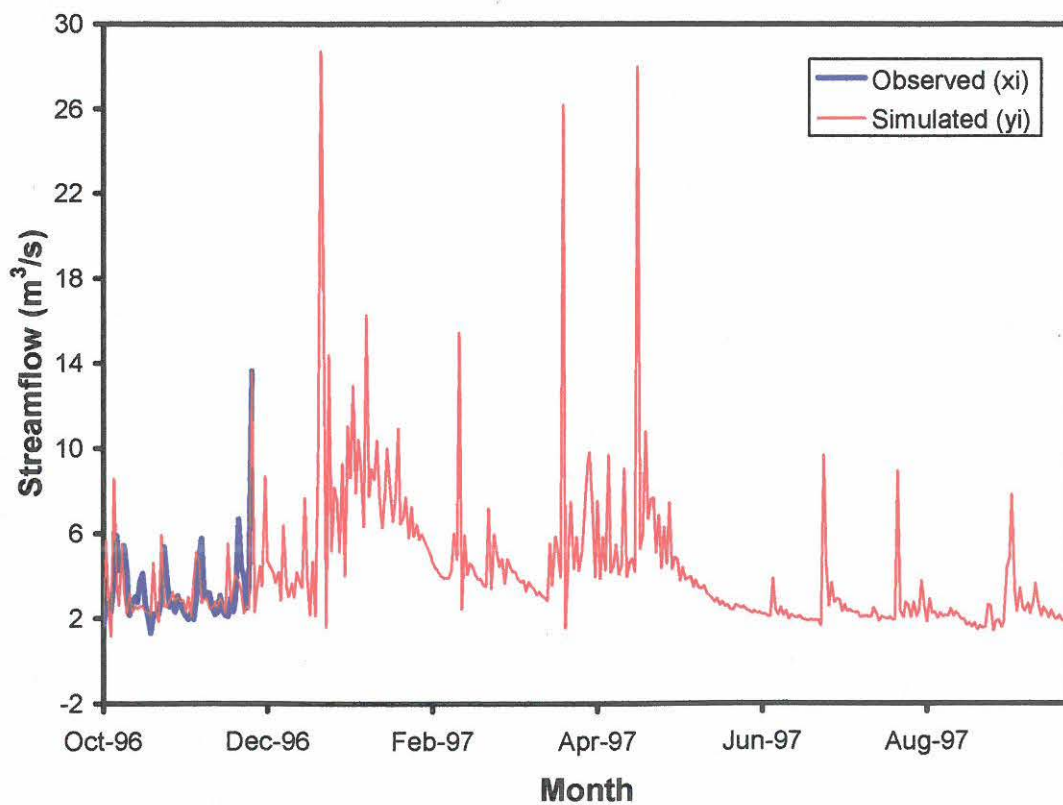
The final calibrated values of INTFW and IRC are listed in Table 6.11.



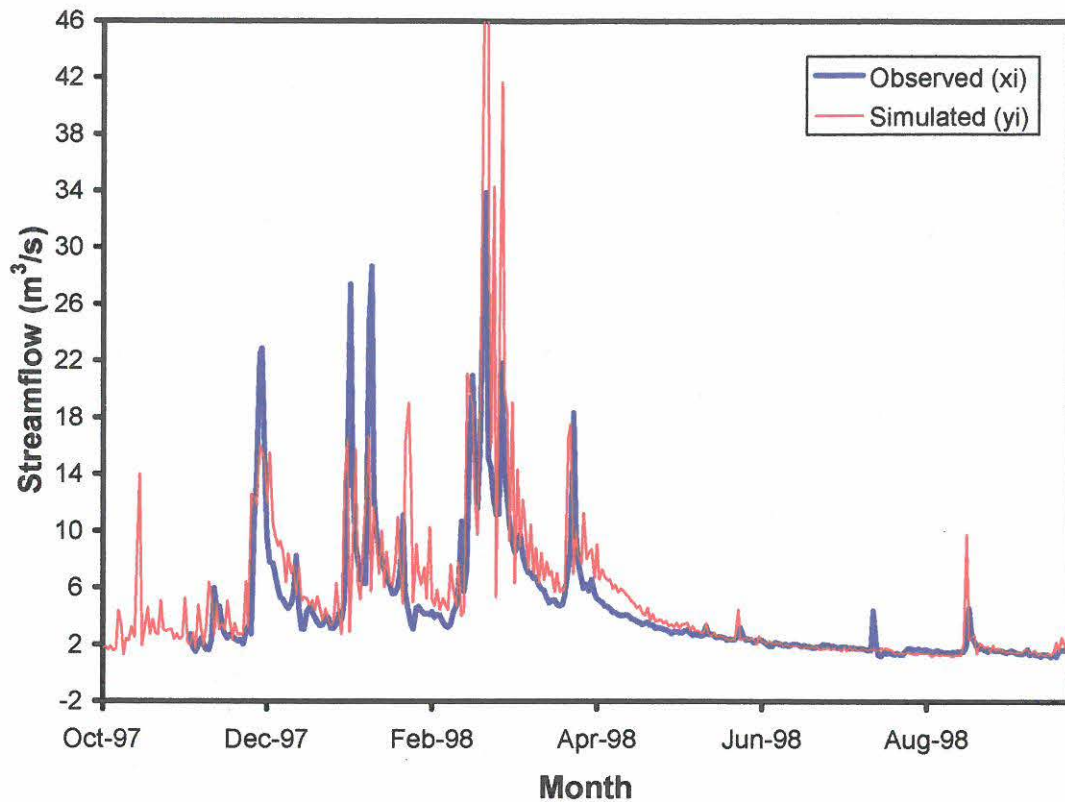
**Table 6.11:** Summary of hydrograph shape parameters in sub-catchments 15 to 30

Main land-use group	INTFW	IRC
Forest	2.5	0.70
Open spaces	3.0	
Grassland	2.0	
CBD and industrial	1.0	
Medium-density residential	1.5	
Low-density residential	1.8	
High-density residential	1.0	
Wetland	2.0	

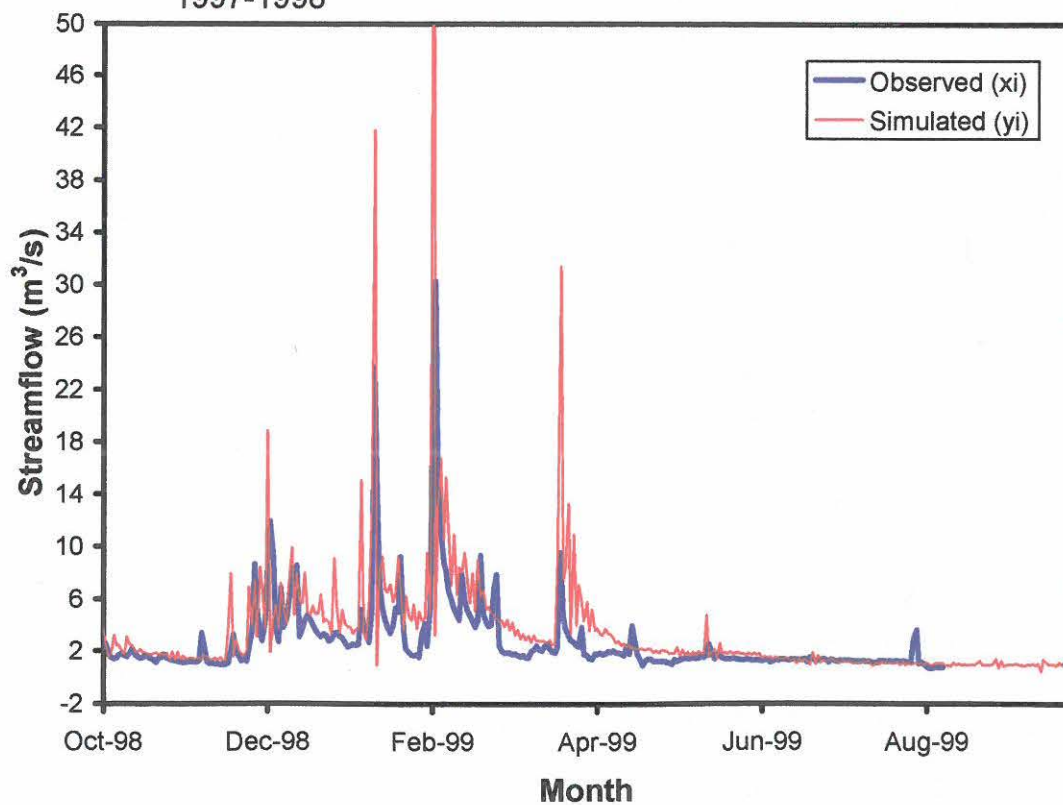
The comparison of observed and simulated daily streamflow at U2H041 for each hydrological year is shown in Figures 6.17 to 6.19.



**Figure 6.17:** Comparison of observed and simulated daily streamflow: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological year: 1996-1997



**Figure 6.18:** Comparison of observed and simulated daily streamflow: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological year: 1997-1998



**Figure 6.19:** Comparison of observed and simulated daily streamflow: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological year: 1998-1999

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*U2H022, Msunduzi River at Nomfihlelo:*

The period of calibration at this hydrological gauging station is for the hydrological years of 1992 to 1998. The wet- and dry period were over-simulated by 14% and 2% respectively. Overall, the annual water balance at U2H022 was over-simulated.

The higher streamflow peaks were over-simulated on various events, especially on 1993/10/06, 1993/12/27, 1994/02/15, 1994/12/27, 1995/12/26-27, 1996/07/09, 1998/02/22 and 1998/02/23.

The baseflow was correctly simulated throughout the period of calibration, except during the hydrological years of 1992 to 1994 and 1997 to 1998. In both instances, the autumn baseflow at the recession limbs of the hydrographs was over-simulated.

The observed data of 1995/12/25 to 1995/12/28 are questionable, because a constant streamflow of  $36.49 \text{ m}^3/\text{second}$  was recorded. This constant streamflow could possibly be ascribed to faulty electronic data-logger readings or to the entanglement of the Type-X mechanical recorder cable system in the recorder well. Taking the hydrological responsiveness of these sub-catchments, which are prone to a spatial variation in precipitation data due to thunderstorm activities into consideration, a constant hydrograph limb in the descending phase is quite irregular. Furthermore it is important to note that the upper limit of accurately measured data at this hydrological gauging station is  $36.50 \text{ m}^3/\text{second}$ .



The final calibrated values of the five parameters, which determine the seasonal and annual water balance in sub-catchments 31 to 42 are listed in Table 6.12.

**Table 6.12:** Summary of seasonal and annual water balance parameters in sub-catchments 31 to 42

Main land-use group	LZETP	UZSN (mm)	LZSN (mm)	AGWRC	INFILT (mm/h)
Forest	0.80	29	320	0.93	1.00
Open spaces	0.60	17	320		
Grassland	0.50	17	320		
CBD and industrial	0.10	3	320		
Medium-density residential	0.20	7	320		
Low-density residential	0.40	10	320		
High-density residential	0.15	4	320		
Wetland	0.75	10	320		

In the final stage of the hydrological calibration of U2H022, the shape of the individual hydrographs was adjusted by the parameters INTFW and IRC. The final calibrated values of these two parameters are listed in Table 6.13.

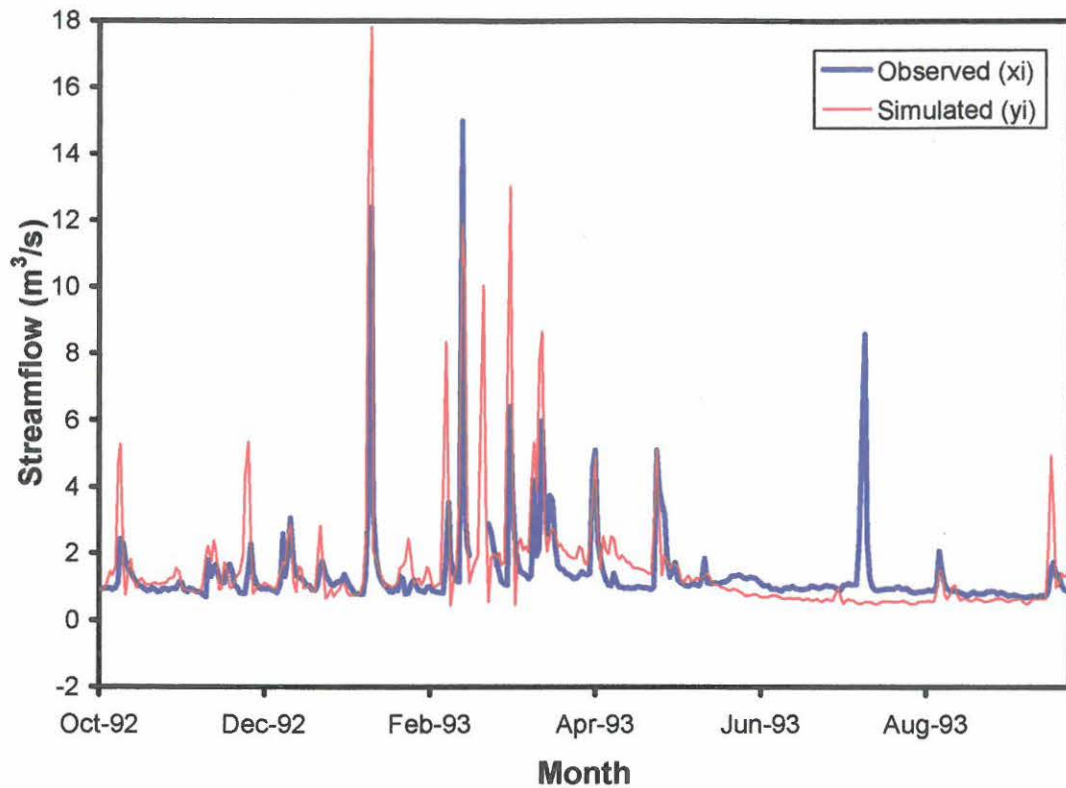
**Table 6.13:** Summary of hydrograph shape parameters in sub-catchments 31 to 42

Main land-use Group	INTFW	IRC
Forest	2.5	0.7
Open spaces	3.0	
Grassland	2.0	
CBD and industrial	1.0	
Medium-density residential	1.5	
Low-density residential	1.8	
High-density residential	1.0	
Wetland	2.0	

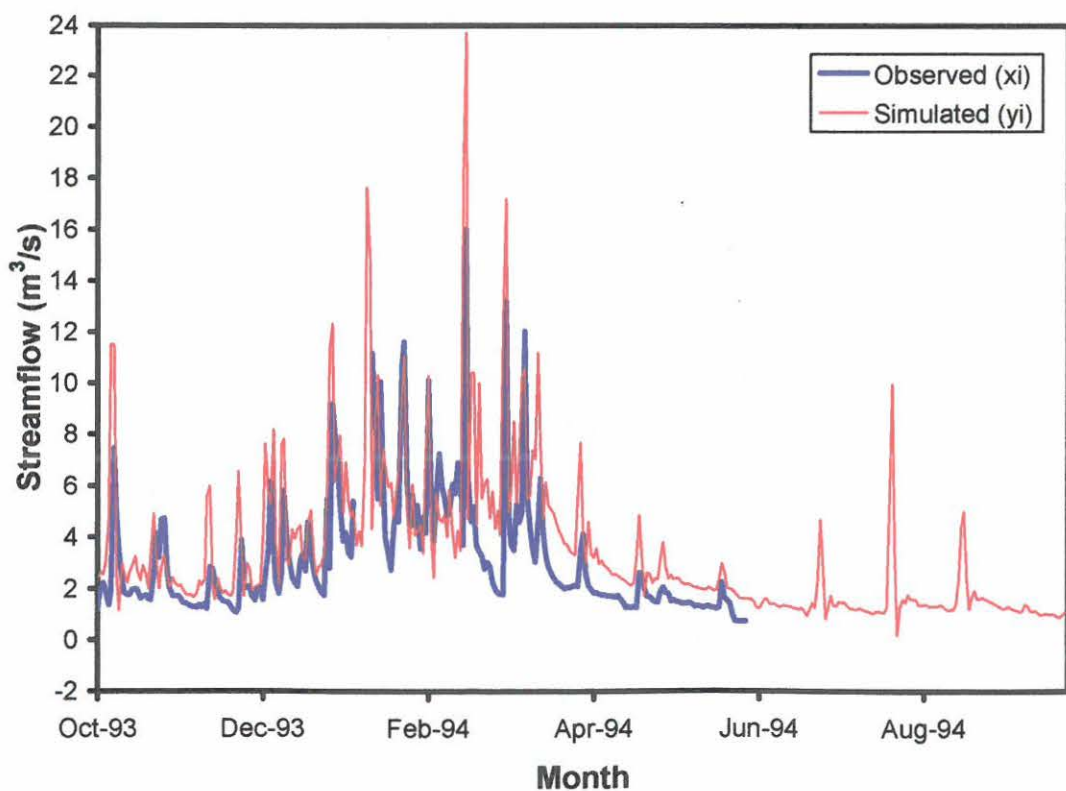
The final INTFW values of the simulated hydrographs were accurately simulated. Simulated interflow recession was incorrectly simulated at the end of the wet period of the hydrological year 1992 to 1993, as well as the next hydrological year. Thereafter, it was correctly simulated during the period of October 1994 to May 1997. The dry period up to June 1997 was incorrectly simulated, resulting in a lower baseflow up to the end of September 1997. Over-simulation of autumn baseflow at the descending limb of the hydrograph was the most consistent error during the last hydrological year.

The daily calibration at U2H022 is the best among the four hydrological gauging stations. This could be ascribed to the fact that the available precipitation data's areal distribution was much more representative and reasonably accounted for the spatial variation in precipitation and storm distributions. The hydrographs at U2H022 and U2H041 mimic each other, especially the timing of the hydrographs, not so specifically the magnitudes of the higher streamflow peaks.

The comparison of observed- and simulated daily streamflow at U2H022 for each hydrological year is shown in Figures 6.20 to 6.26. A summary of the annual water balance at the four hydrological gauging stations is listed in Table 6.14.

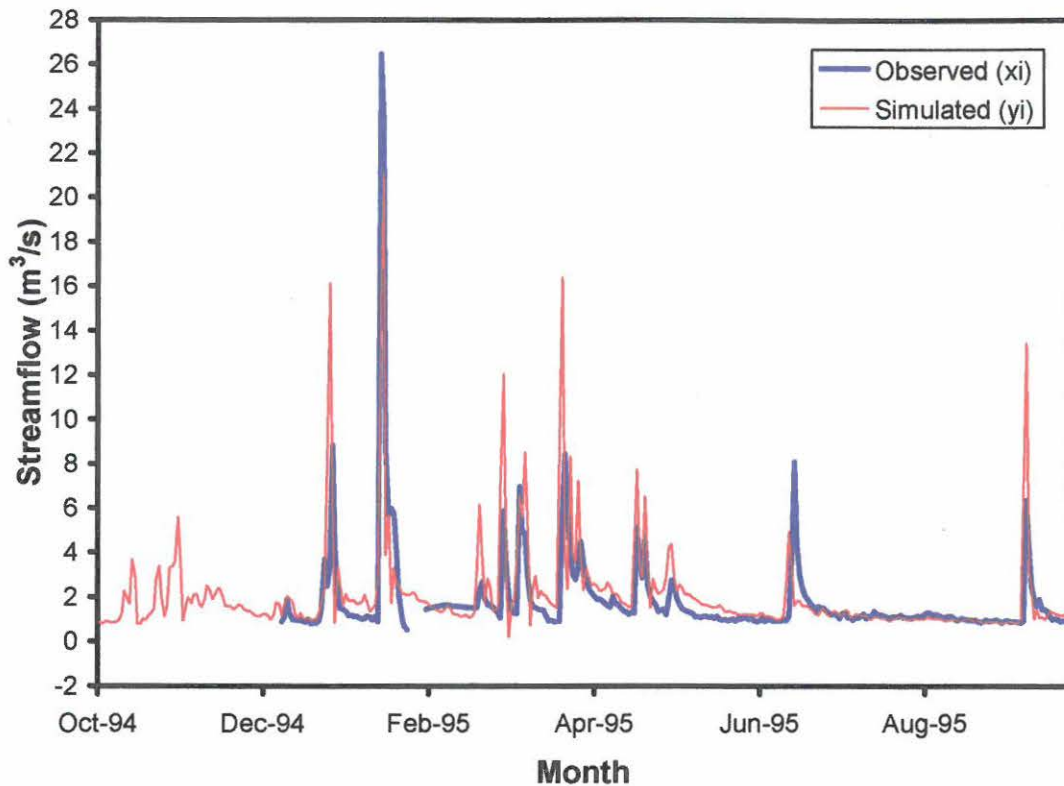


**Figure 6.20:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1992-1993

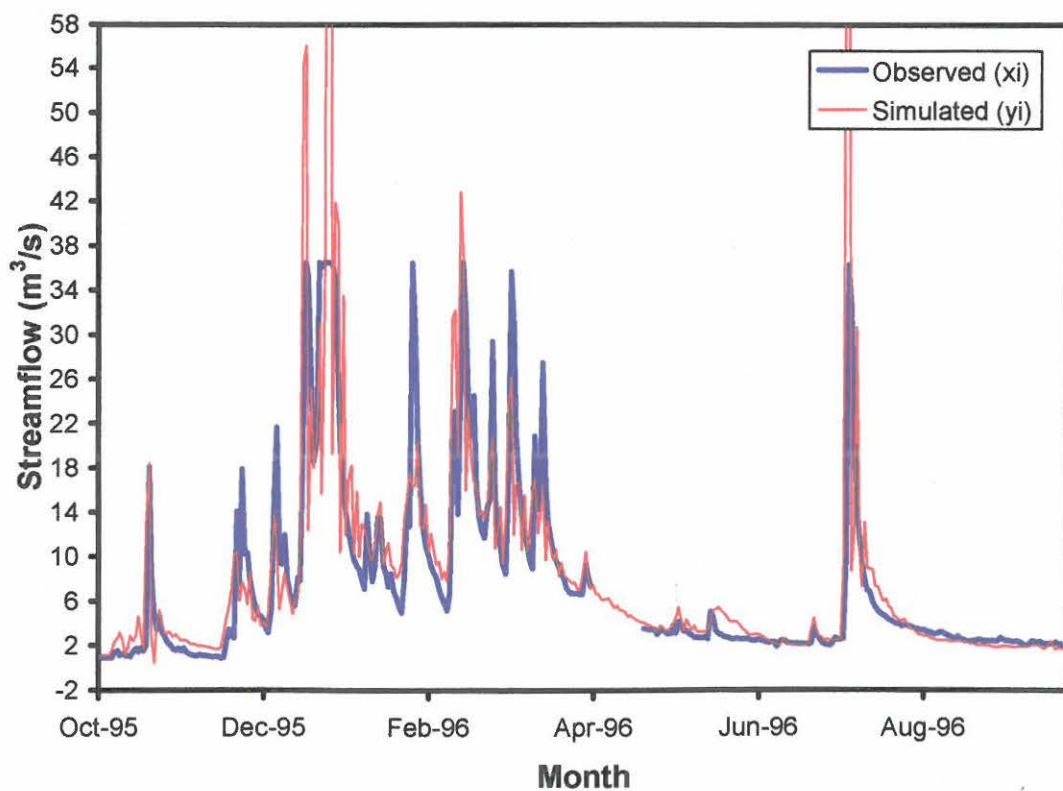


**Figure 6.21:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1993-1994

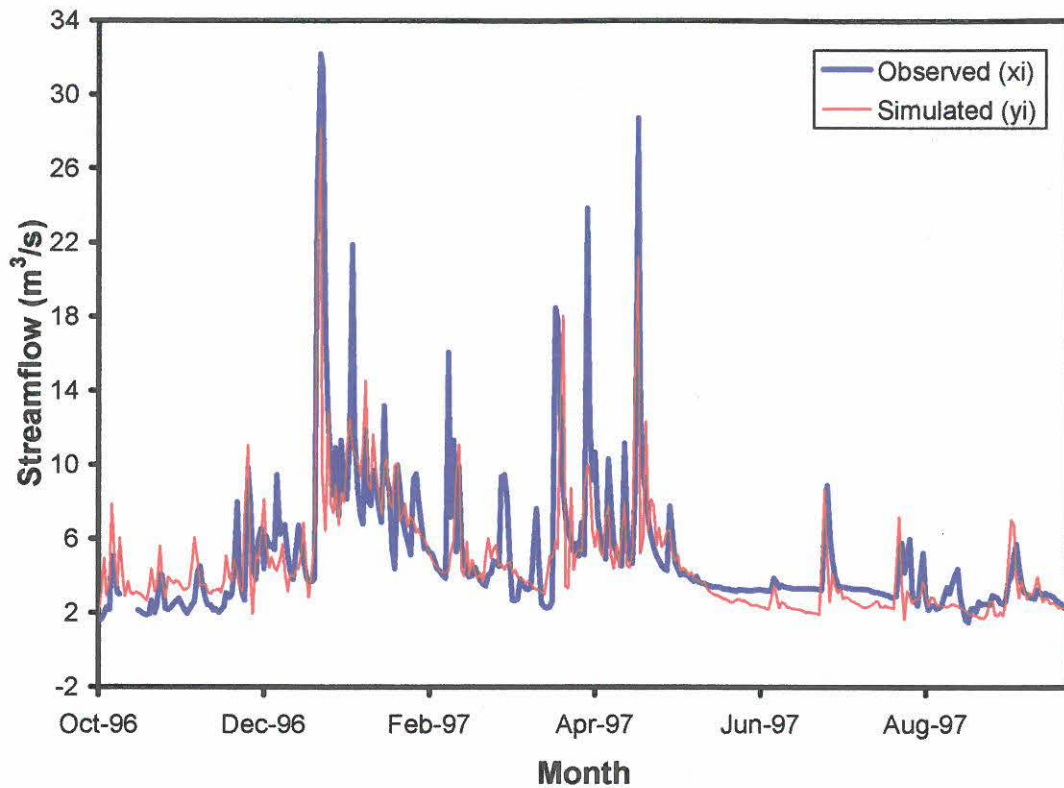




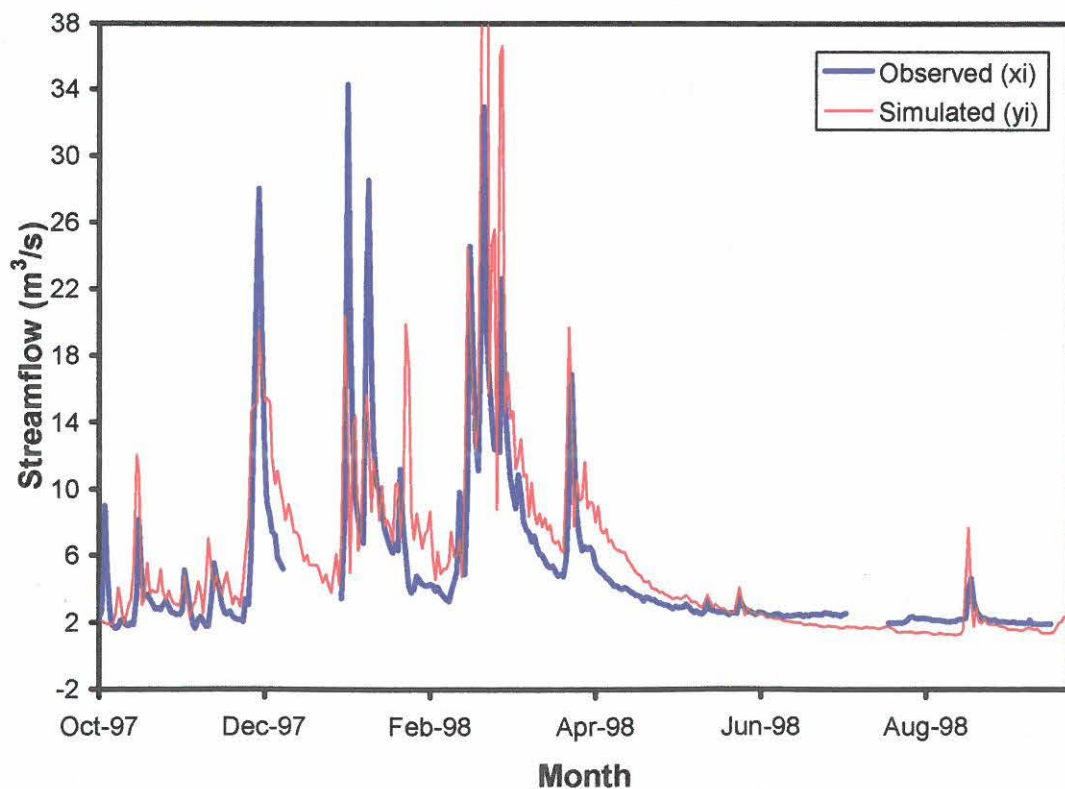
**Figure 6.22:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1994-1995



**Figure 6.23:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1995-1996



**Figure 6.24:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1996-1997



**Figure 6.25:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1997-1998

**Table 6.14:** Summary of the annual water balance at the hydrological gauging stations during calibration. ( $\pm$  Indicates either over- or under-simulation)

Station number	Period of calibration	Annual water balance (Observed $x_i$ , $\times 10^6 \text{ m}^3$ )	Annual water balance (Simulated $y_i$ , $\times 10^6 \text{ m}^3$ )	Percentage difference (%)
U2H011	1993-1996	49	50	+2
U2H058	1995-1998	313	285	-9
U2H041	1996-1999	226	286	+21
U2H022	1992-1998	682	769	+11

### 6.3.3 Verification of Streamflow Simulations

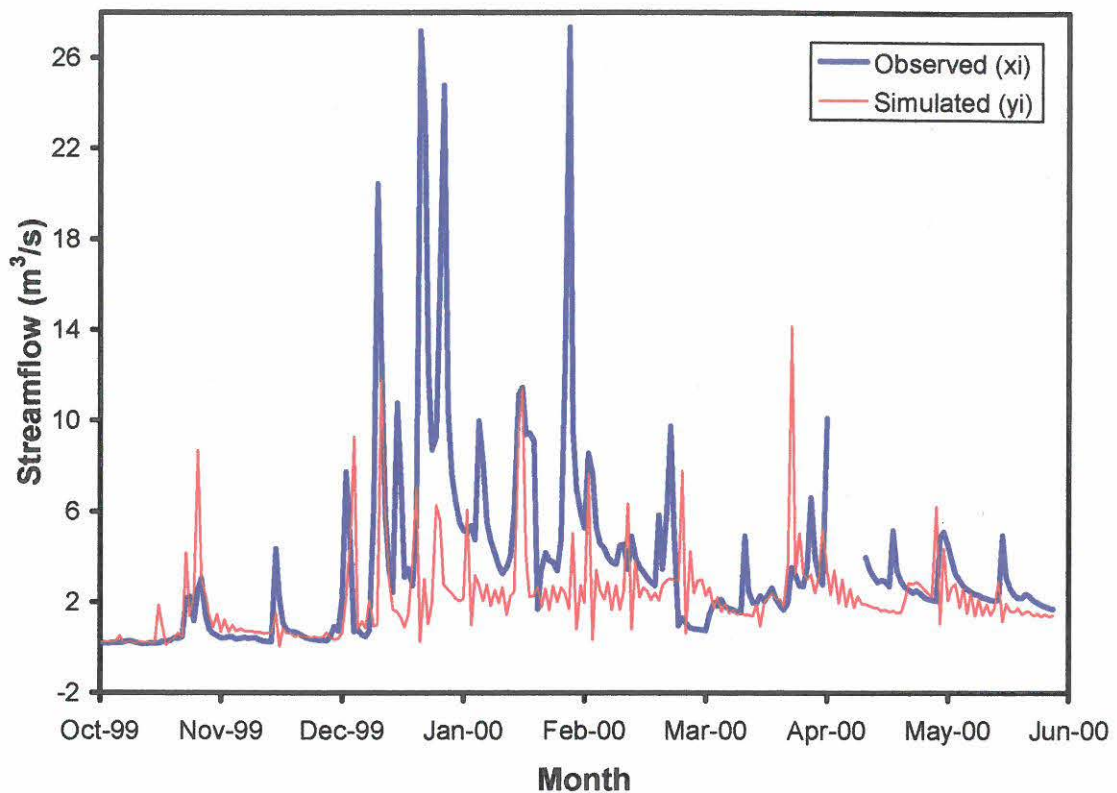
The verification of the hydrological simulations at a daily level was an extension of the calibration process. The verification assures that the calibrated data used in the HSPF model properly assesses all the variables and conditions, which can affect the final modelling results. Therefore only a portion of the record of observed data was used for finalising the parameters during calibration. The remaining records of observed data were then used for verification.

However, due to a lack of good observed data at U2H011, the entire observed record was used for calibration. The periods used for the verification of the four hydrological gauging stations are summarised in Table 6.15.

**Table 6.15:** Summary of verification data

Station description (DWAf number and name)	Period of verification
U2H011 Msunduzi River at Henley Dam	None
U2H058 Msunduzi River at Mason's Mill	1998/10/01-2000/06/01
U2H041 Msunduzi River at Hampstead Park, Moto-X	1999/10/01-2000/06/01
U2H022 Msunduzi River at Nomfihlelo	1998/10/01-2000/06/01





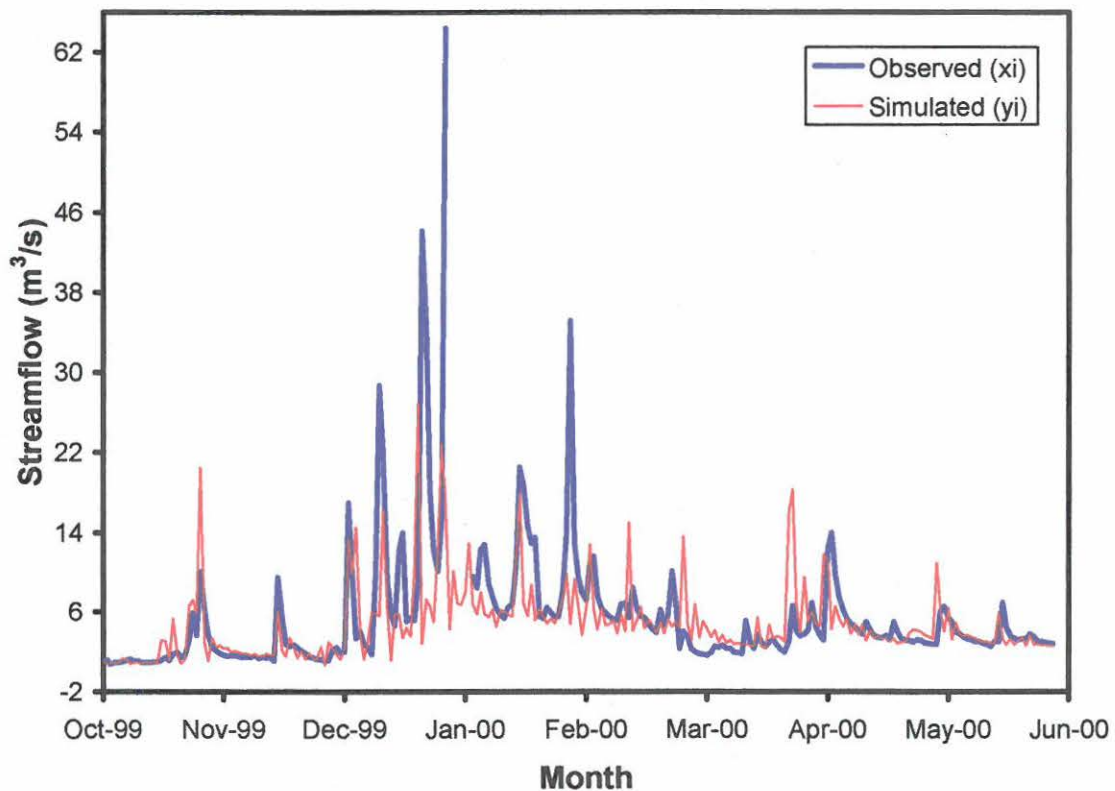
**Figure 6.27:** Comparison of observed and simulated daily streamflow: U2H058: Msunduzi River at Mason's Mill. Verification year: 1999-2000

*U2H041, Msunduzi River at Hampstead Park, Moto-X:*

The verification period was only for one year. The results were average to poor. The wet period was under-simulated by 17%, while the dry period was over-simulated by 2%, resulting in under-simulation of the annual water balance.

In the verification year 1999 to 2000, low streamflow peaks in the order of 12 to 14 m<sup>3</sup>/second were over-simulated, while the higher streamflow peaks were under-simulated. This could be indicative that the model was not responding rapidly to the precipitation input in order to contribute streamflow of this magnitude.

The timing of the hydrograph peaks was incorrectly simulated during certain months; suggesting possible improvements to the INTFW parameter. Overall, the interflow recession of the largest hydrographs was correctly simulated. The comparison of observed and simulated daily streamflow at U2H041 is shown in Figure 6.28.



**Figure 6.28:** Comparison of observed and simulated daily streamflow: U2H041: Msunduzi River at Hampstead Park, Moto-X. Verification year: 1999-2000

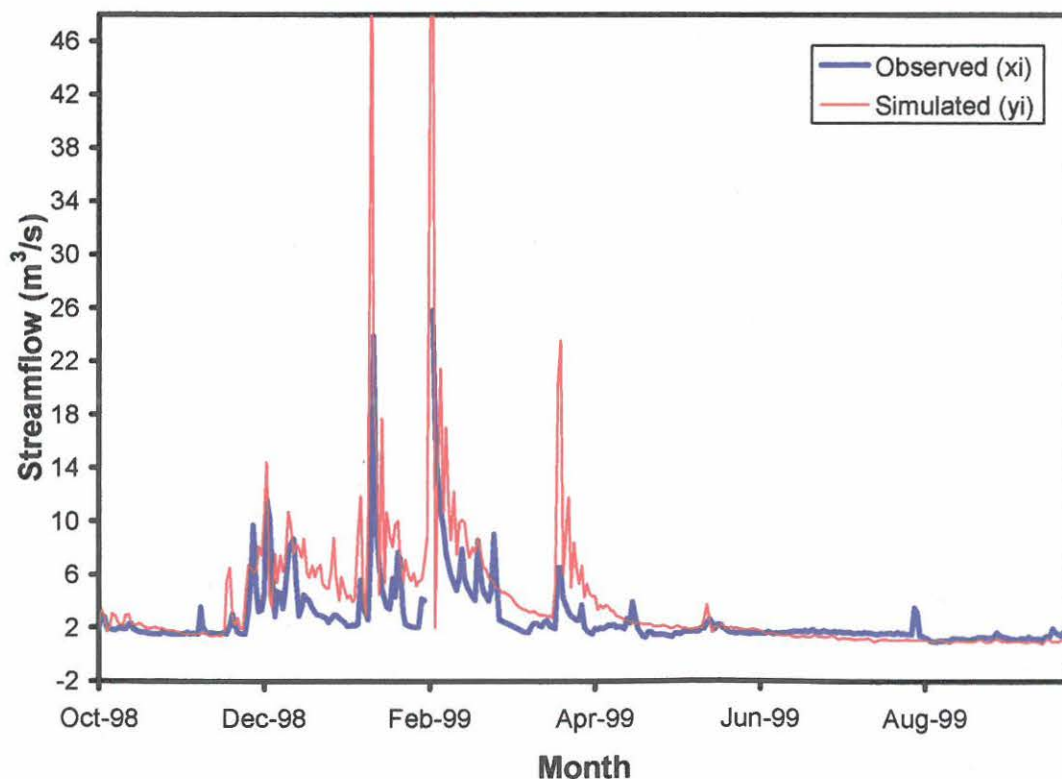
*U2H022, Msunduzi River at Nomfihlelo:*

The verification at U2H022 was the best among the three hydrological gauging stations. This could be ascribed to the fact that the available precipitation data's areal distribution was much more representative. The wet period was over-simulated by 7% and the dry period was under-simulated by 10%. The annual water balance was over-simulated.

The hydrograph shape of the simulated hydrographs was accurately simulated throughout the period of verification. Interflow recession was inaccurately simulated for the first year of verification, thereafter it was satisfactorily simulated. A summary of the annual water balance at the three hydrological gauging stations is listed in Table 6.16. The comparison of observed and simulated daily streamflow at U2H022 for each verification year is shown in Figures 6.29 to 6.30.

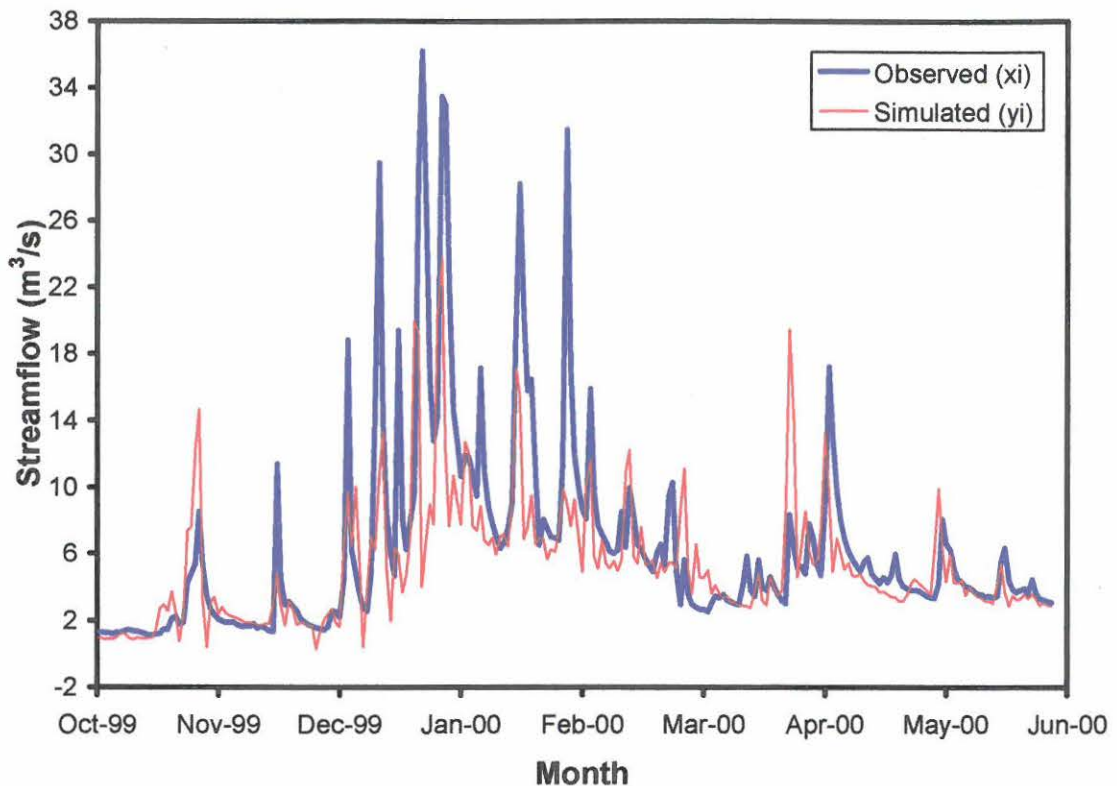
**Table 6.16:** Summary of the annual water balance at the hydrological gauging stations during verification ( $\pm$  Indicates either over- or under-simulation)

Station number	Period of verification	Annual water balance (Observed $x_i$ , $\times 10^6 \text{ m}^3$ )	Annual water balance (Simulated $y_i$ , $\times 10^6 \text{ m}^3$ )	Percentage difference (%)
U2H058	1998-2000	107	101	-6
U2H041	1999-2000	114	96	-16
U2H022	1998-2000	221	233	+5



**Figure 6.29:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Verification year: 1998-1999





**Figure 6.30:** Comparison of observed and simulated daily streamflow: U2H022: Msunduzi River at Nomfihlelo. Verification year: 1999-2000

#### 6.4 ANALYSIS OF MODEL PERFORMANCE

The visual comparison of simulation results against observed data can be highly subjective. Therefore, the pairs of observed data and simulated values of the HSPF model were compared and evaluated using the array of conservation- and regression statistics discussed in Chapter 4.

According to Donigian *et al.* (1984: 114) the performance of hydrology and hydraulic models at an annual- or monthly level with percentage variations of less than 10% are considered very good, between 10 – 15% are good and between 15 – 25% are fair. Munson (1998) stated that several publications on HSPF model performance indicated that a “good” calibration has an  $r^2$  value of 0.9, at the annual level, 0.8 seasonally and 0.6 daily.

The extent and reliability of the meteorological data, especially precipitation, was the biggest limitation in achieving this kind of results. On the other hand, it is a common mistake to accept all observed data as being absolutely accurate. In fact, any measurement obtained under field conditions will usually contain at least a five to 10% variation from the actual value (Donigian *et al.*, 1984: 112).

#### 6.4.1 Graphical Analysis and Scatter Plots

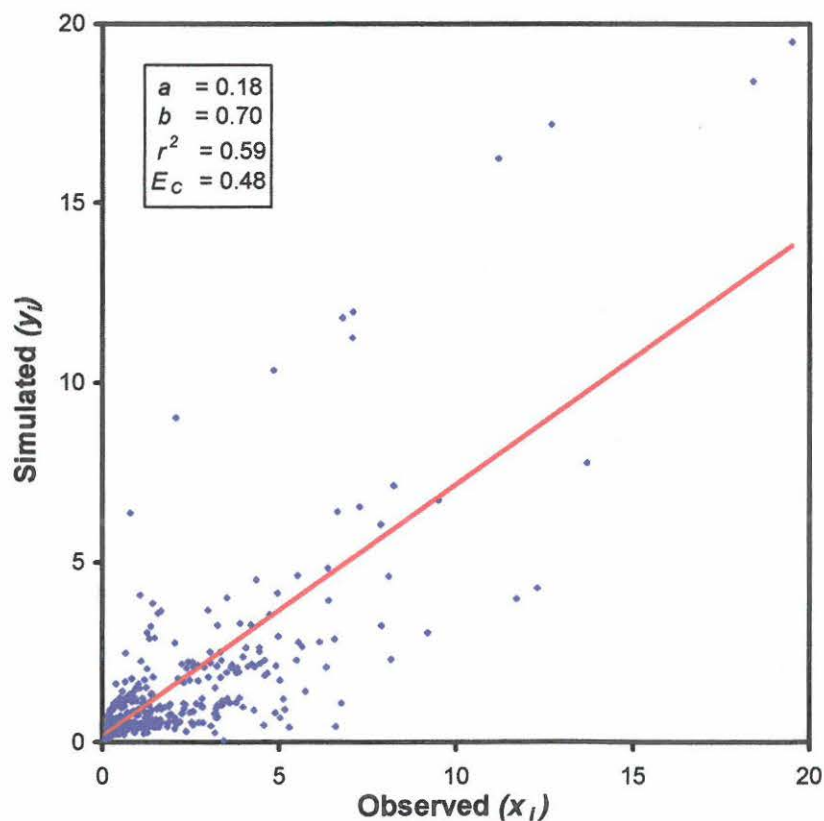
In addition to the model performance measures, scatter plots of the model-simulated values against observed data of the four hydrological gauging stations used to calibrate the model, were obtained to provide a visual measure of model performance.

The scatter plots and model performance measures for the four hydrological gauging stations are shown in Figures 6.32 to 6.46 and Table 6.17, respectively. Results for both calibration- and verification periods are presented, except where a lack of observed data precluded this, such as at U2H011. Values of the y-intercept ( $a$ ), slope ( $b$ ), coefficients of determination ( $r^2$ ) and efficiency ( $E_C$ ) for the complete simulation periods are reported in Table 6.17, but values for individual hydrological years are indicated on the scatter plots, where appropriate. These results provide quantitative amplification of the results discussed in Section 6.3.

The best overall results were obtained for U2H022 ( $r^2=0.65$  and  $E_C=0.59$ ) and the worst overall results at U2H058 ( $r^2=0.37$  and  $E_C=0.05$ ), with U2H041 only marginally better.

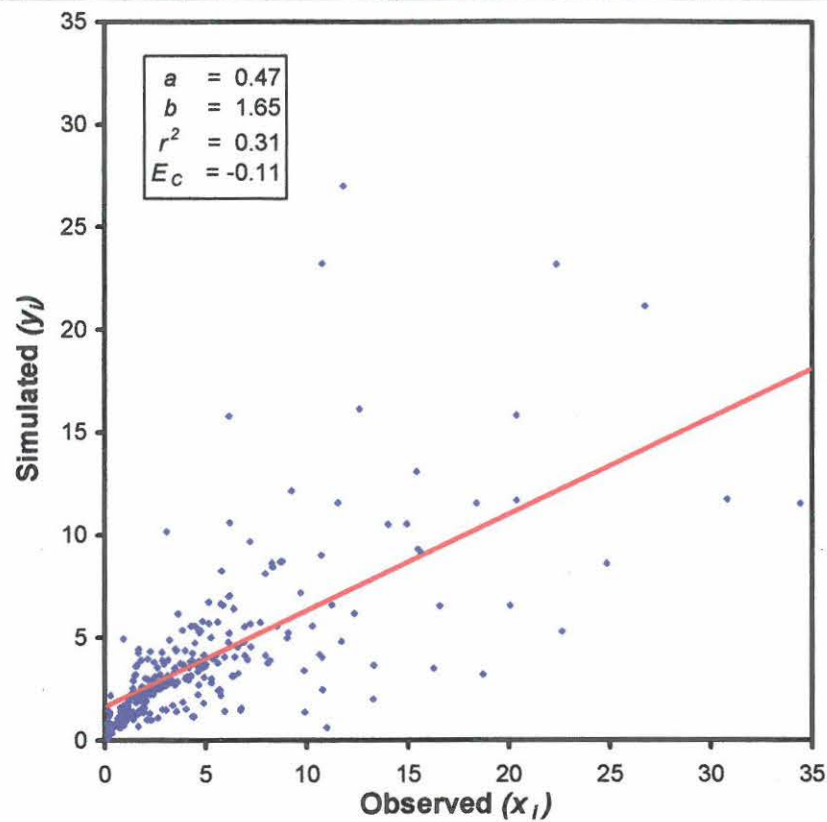
**Table 6.17:** Summary of goodness-of-fit-statistics at the four hydrological gauging stations

Conservation statistics	U2H011	U2H058	U2H041	U2H022
Observed mean ( $\bar{x}$ )	0.97	2.89	4.20	4.14
Simulated mean ( $\bar{y}$ )	0.86	2.79	4.62	4.41
Percentage-difference (%)	11.15	3.55	-9.89	-6.57
Observed standard deviation ( $S_x$ )	1.84	4.95	5.04	5.07
Simulated standard deviation ( $S_y$ )	1.67	4.24	4.80	4.91
Percentage-difference (%)	9.19	14.23	4.81	3.26
Regression statistics	U2H011	U2H058	U2H041	U2H022
Base constant / y-intercept ( $a$ )	0.18	1.29	2.15	1.19
Slope ( $b$ )	0.70	0.52	0.59	0.78
Coefficient of determination ( $r^2$ )	0.59	0.37	0.38	0.65
Coefficient of efficiency ( $E_c$ )	0.48	0.05	0.18	0.59

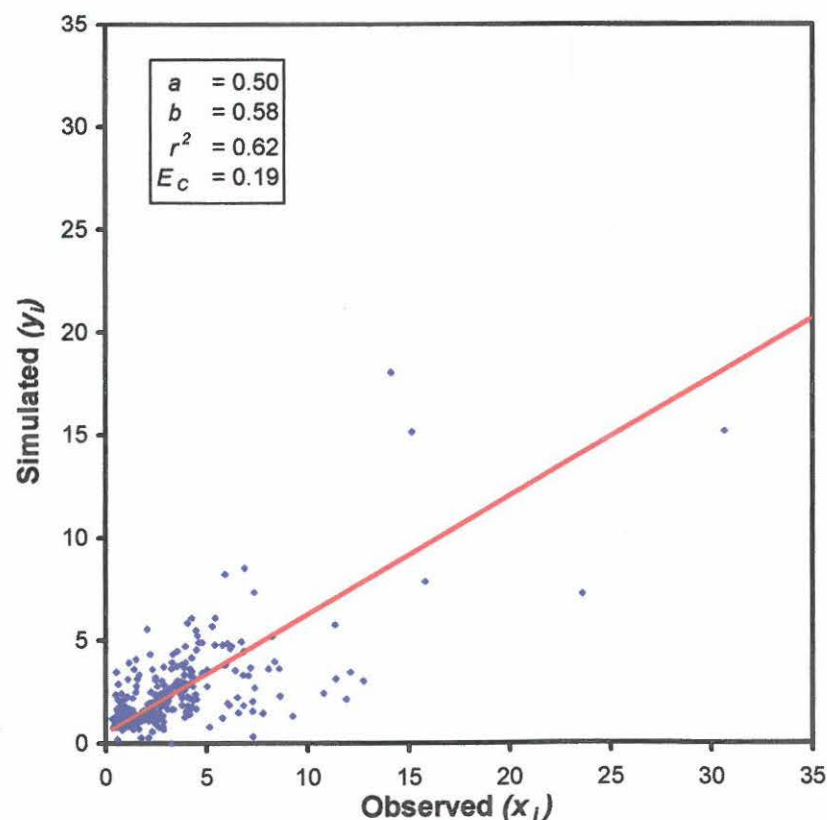


**Figure 6.31:** Scatter plot: U2H011: Msunduzi River at Henley Dam. Hydrological years: 1993-1996

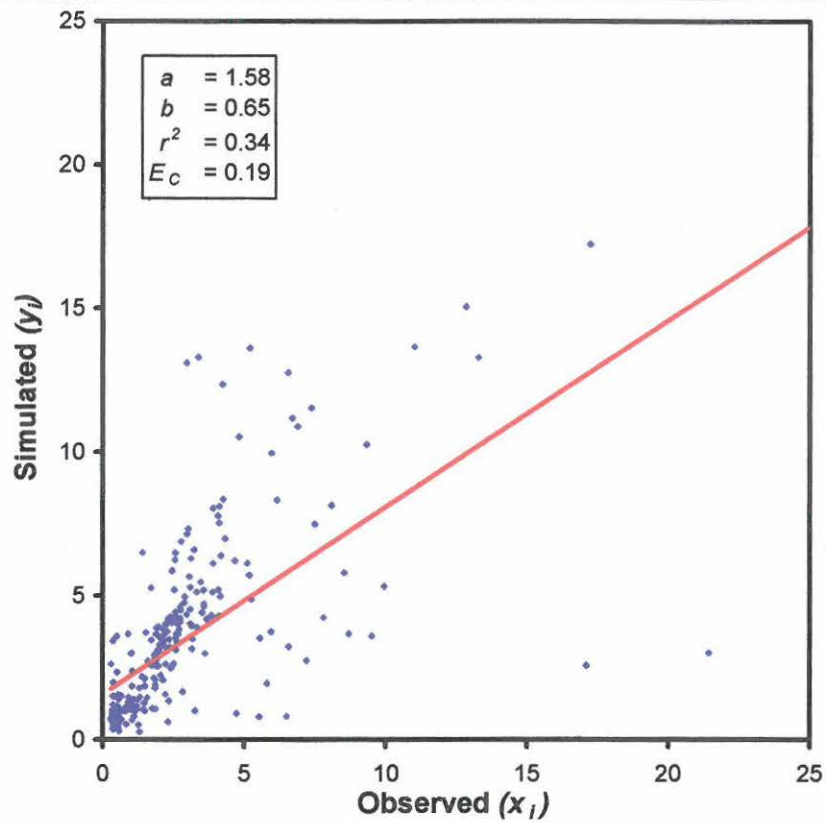




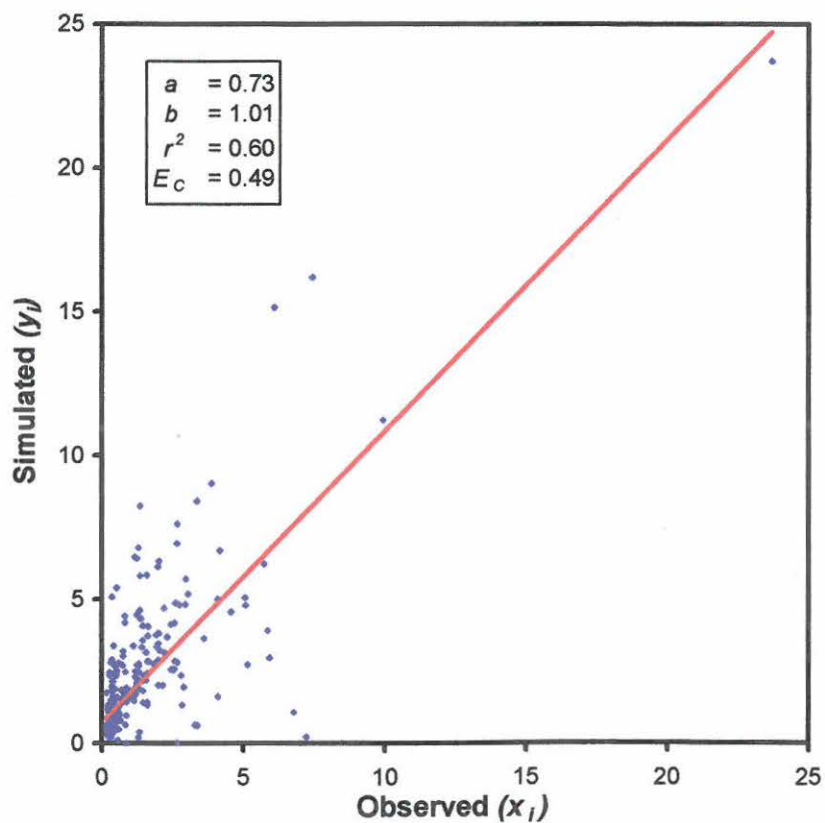
**Figure 6.32:** Scatter plot: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1995-1996



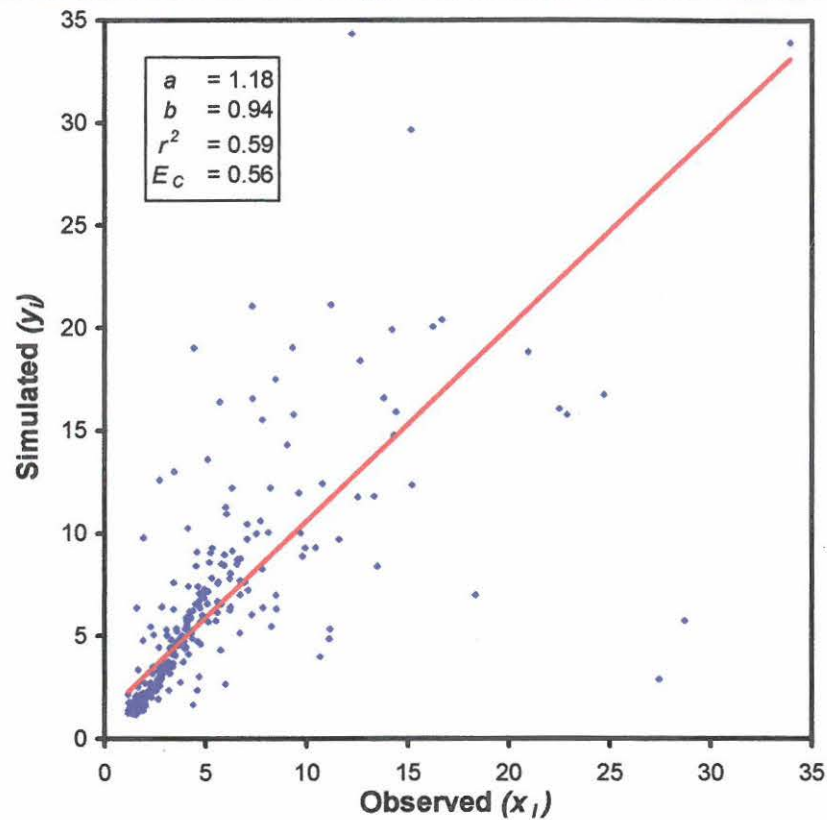
**Figure 6.33:** Scatter plot: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1996-1997



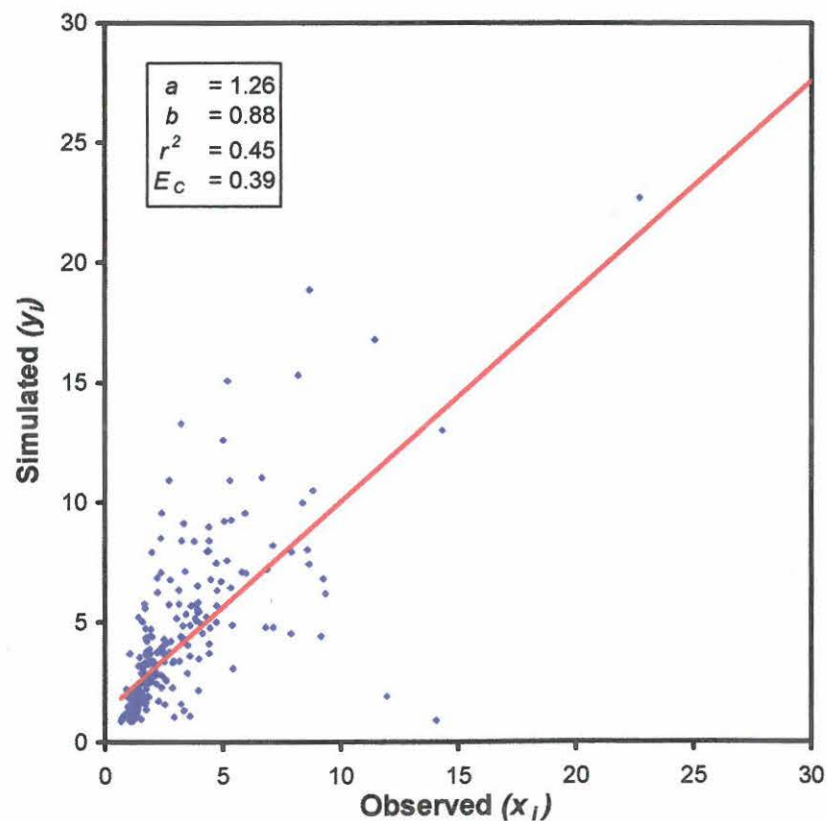
**Figure 6.34:** Scatter plot: U2H058: Msunduzi River at Mason's Mill. Hydrological year: 1997-1998



**Figure 6.35:** Scatter plot: U2H058: Msunduzi River at Mason's Mill. Verification year: 1998-1999

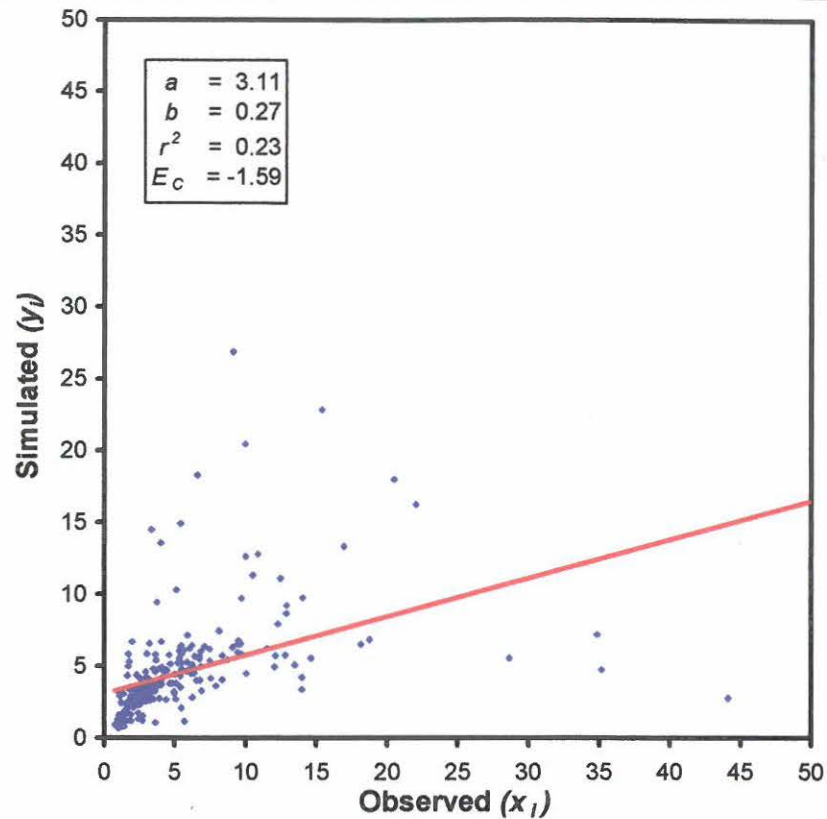


**Figure 6.36:** Scatter plot: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological year: 1997-1998

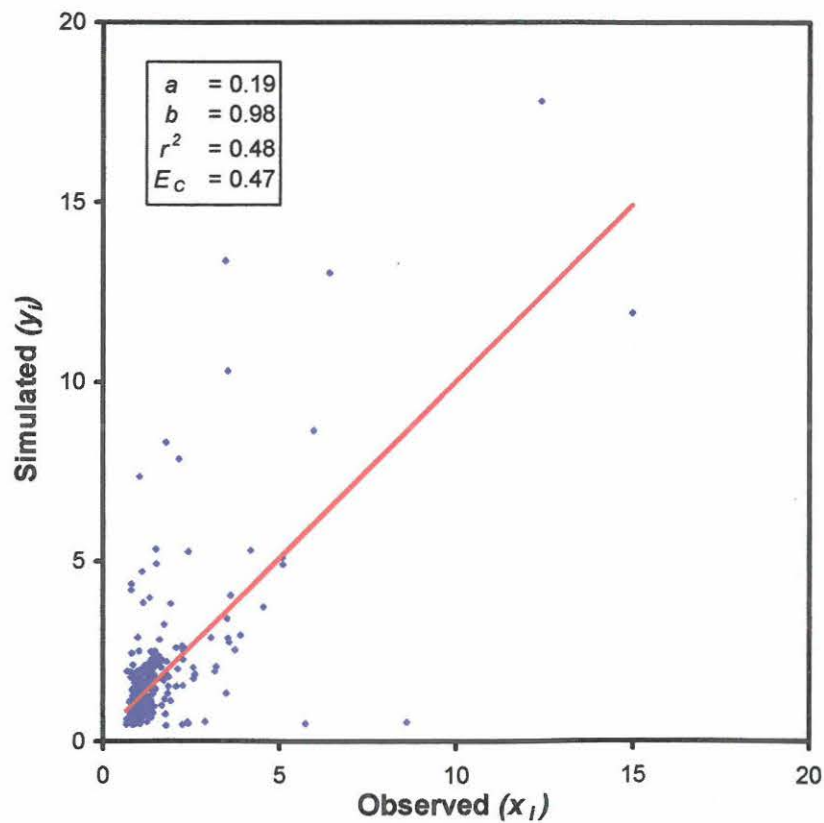


**Figure 6.37:** Scatter plot: U2H041: Msunduzi River at Hampstead Park, Moto-X. Hydrological year: 1998-1999





**Figure 6.38:** Scatter plot: U2H041: Msunduzi River at Hampstead Park, Moto-X. Verification year: 1999-2000



**Figure 6.39:** Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1992-1993

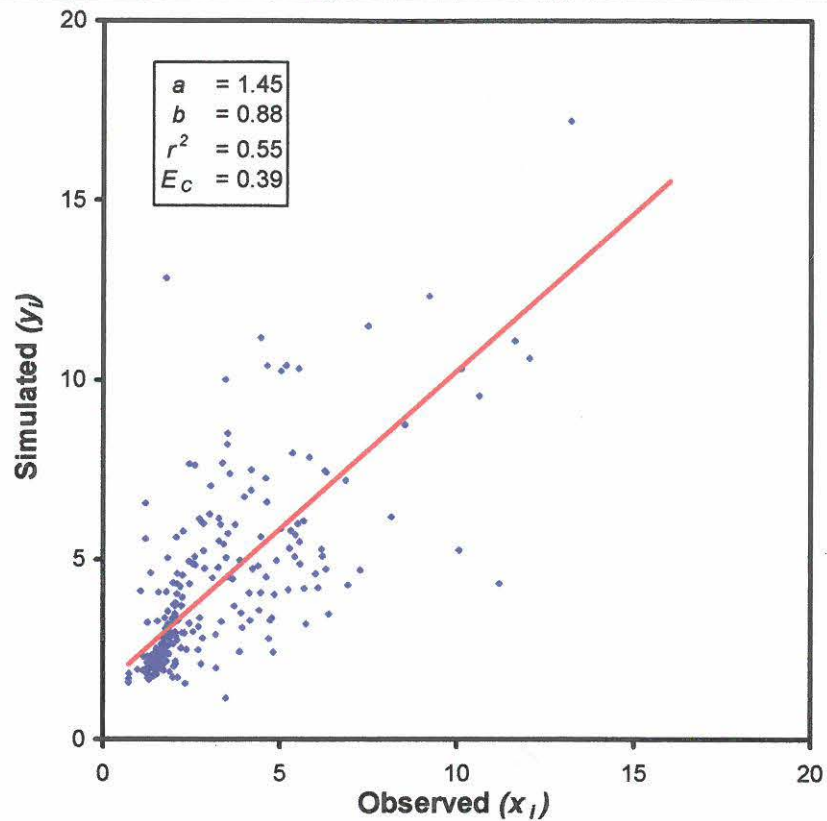


Figure 6.40: Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1993-1994

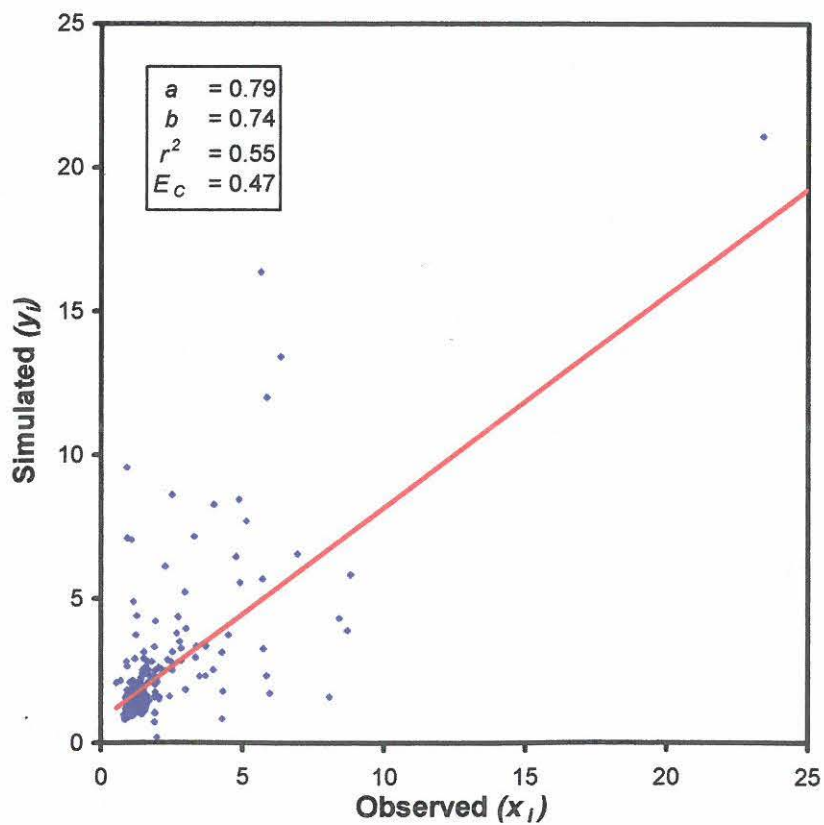


Figure 6.41: Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1994-1995

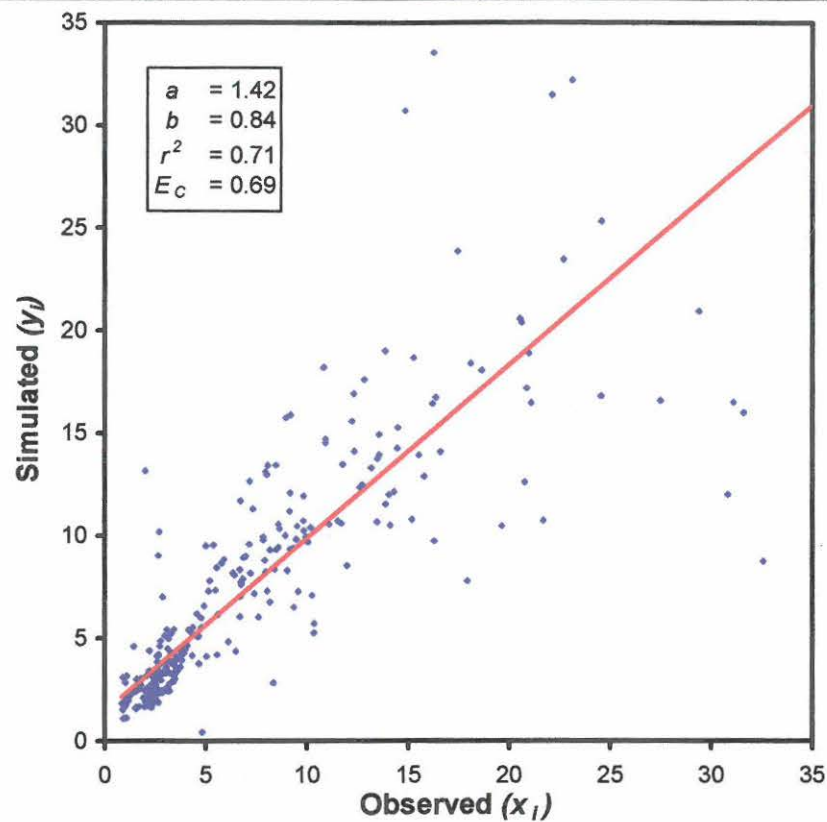


Figure 6.42: Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1995-1996

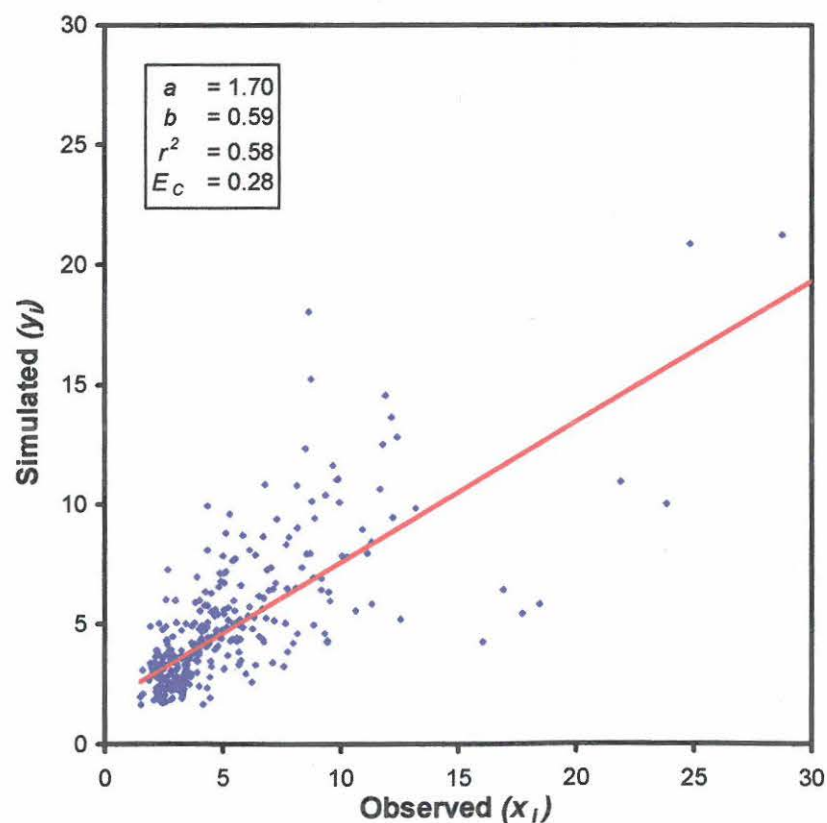
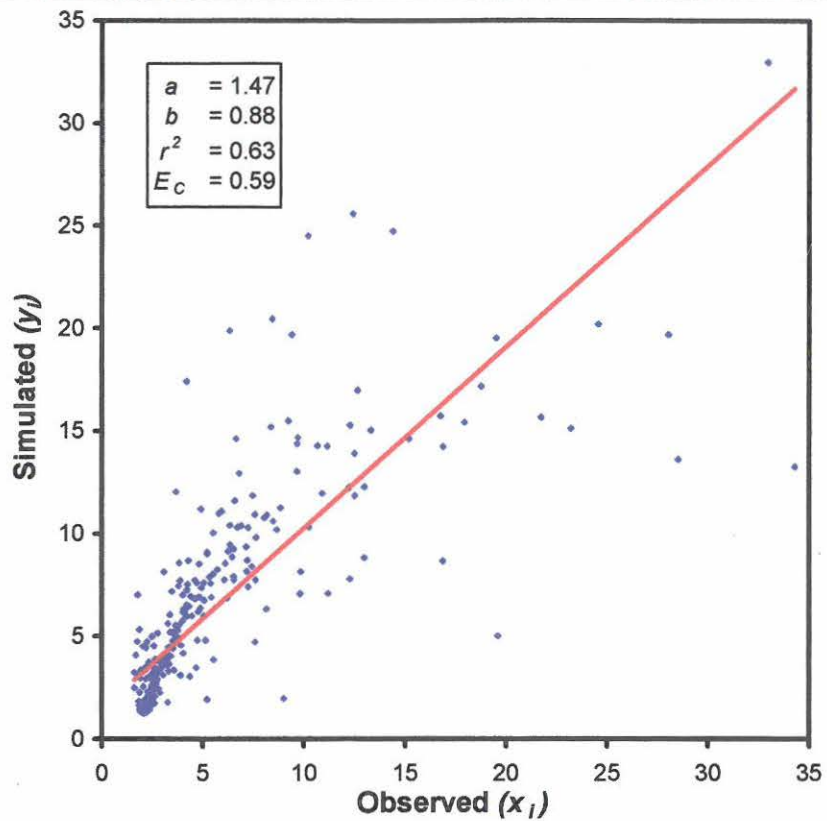
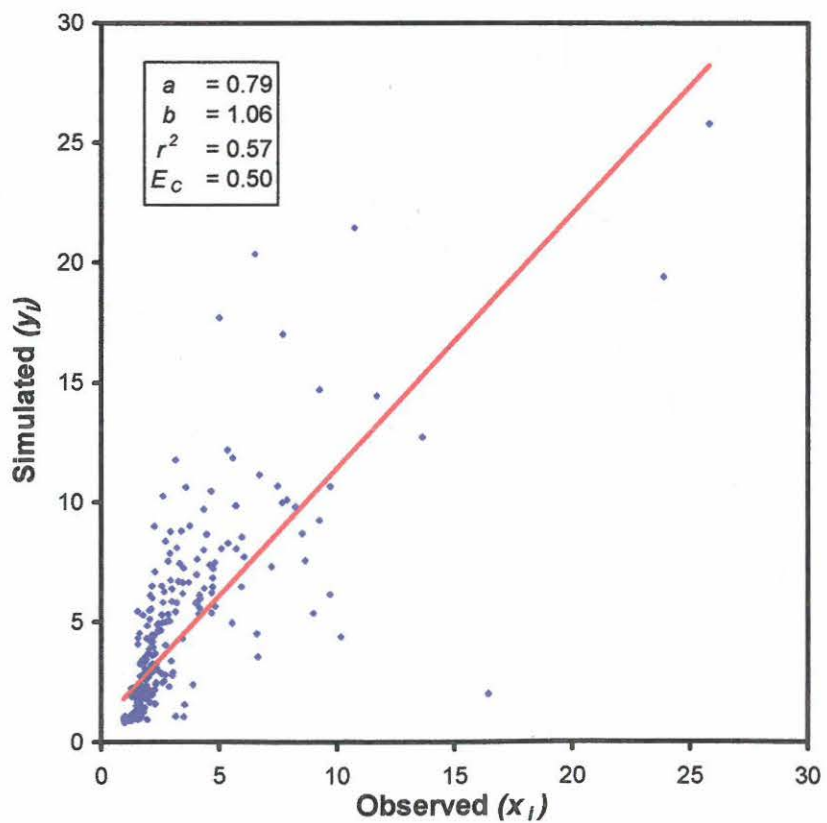


Figure 6.43: Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1996-1997

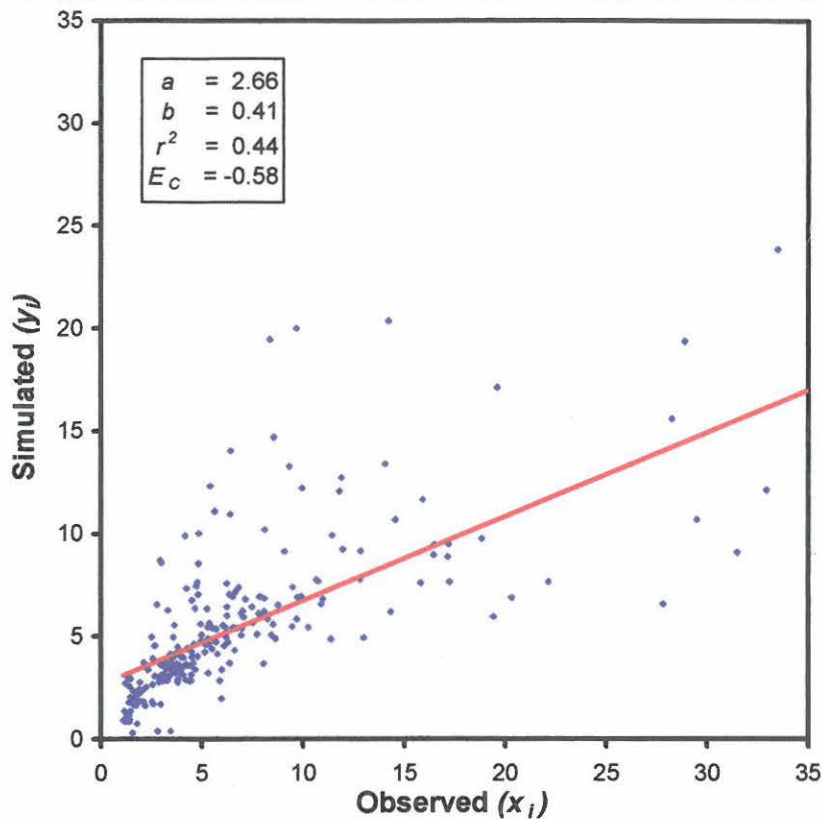




**Figure 6.44:** Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Hydrological year: 1997-1998



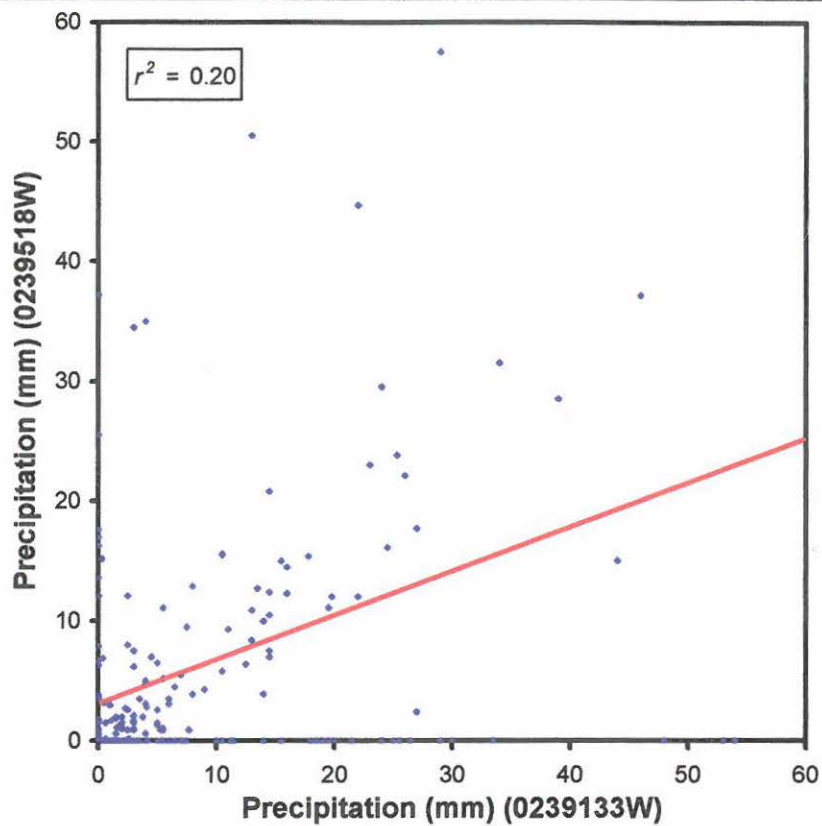
**Figure 6.45:** Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Verification year: 1998-1999



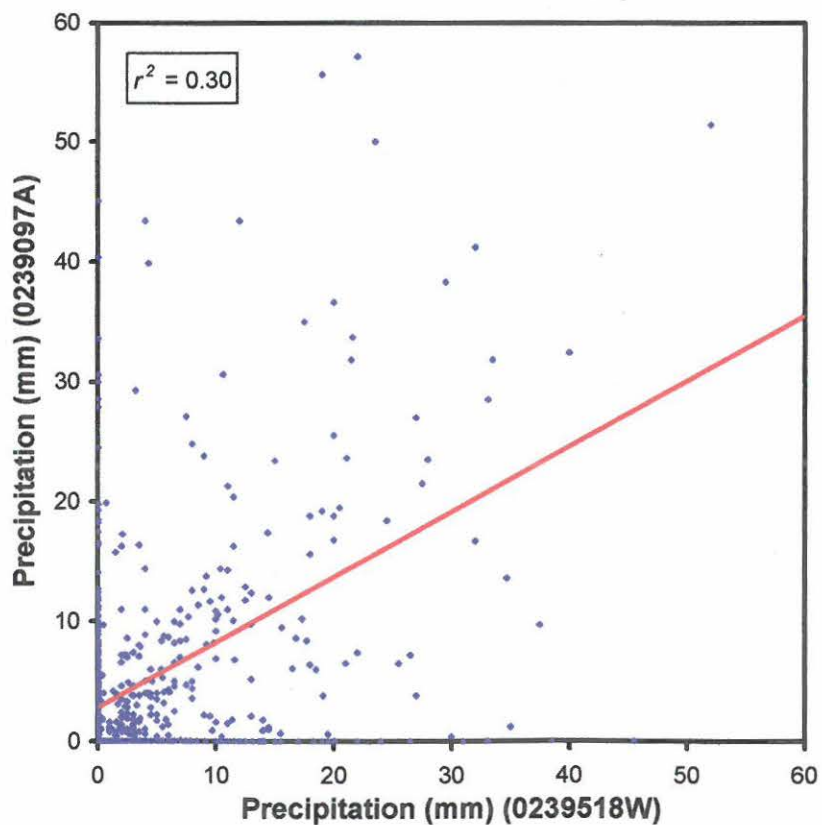
**Figure 6.46:** Scatter plot: U2H022: Msunduzi River at Nomfihlelo. Verification year: 1999-2000

## 6.5 ANALYSIS OF PRECIPITATION DATA

The reliability, along with the poor representation and assumption of uniform areal distribution of precipitation data were the key factors, which influenced the calibration and model performance at a daily level. In order to substantiate this statement, the coefficient of determination ( $r^2$ ) and scatter plots were used to evaluate the correlation between two or more of the precipitation stations used at each hydrological gauging station. Only the original precipitation records, without the infilling as discussed in Chapter 5 were used. The duration of the period of evaluation was therefore in most instances shorter than the period of calibration and verification. The scatter plots and  $r^2$ -values at each of the precipitation stations used for the various sub-catchments are shown in Figures 6.47 to 6.50.

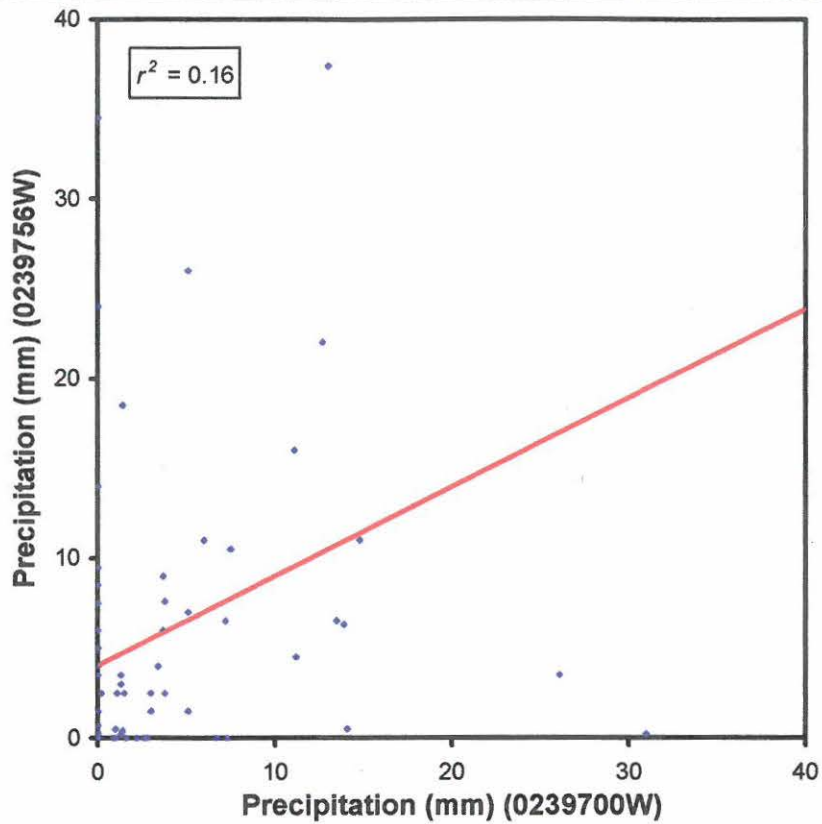


**Figure 6.47:** Scatter plot: Vaucluse (0239133W) versus Edendale (0239518W) used for sub-catchments one to five (upstream of U2H011)

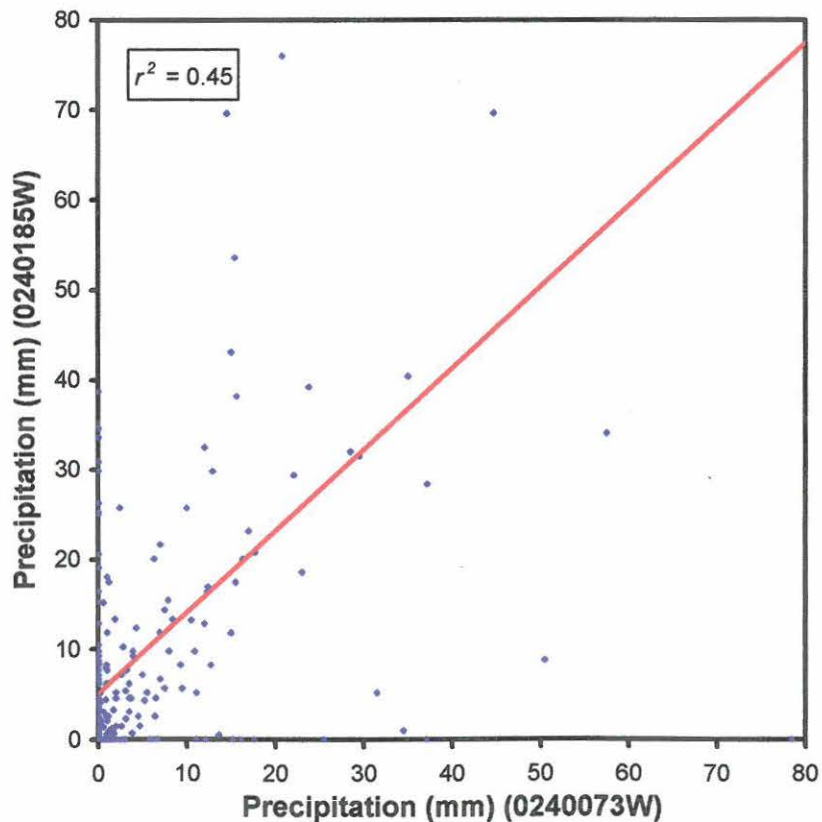


**Figure 6.48:** Scatter plot: Edendale (0239518W) versus Elandshoek (0239097A) used for sub-catchments six to 14 (upstream of U2H058)





**Figure 6.49:** Scatter plot: Ukulinga (0239700W) versus Pietermaritzburg purification works (0239756W) used for sub-catchments 15 to 30 (upstream of U2H041)



**Figure 6.50:** Scatter plot: Camperdown (0240073W) versus Nagle (0240185W) used for sub-catchments 31 to 42 (upstream of U2H022)

The results of the scatter plots indicate a low correlation between the different precipitation data sets, thus emphasising the spatial variation in precipitation- and storm distributions. Individual hydrographs were best simulated at U2H022 possibly because of more representative precipitation data (highest  $r^2$ -values) within those sub-catchments.

Even when the precipitation stations have a low degree of correlation, they are still representative of the parts of the catchment for which they provide precipitation distribution as determined by the Thiessen polygons. The question is really whether the precipitation stations are representative of the precipitation distribution suggested by the Thiessen polygons. Since there is bound to be variation in areal distribution in precipitation within a given polygon, given the small number of precipitation stations, a single station is considered to be a poor representative of the entire polygon. The only solution to this problem is a more representative distribution of precipitation stations, or using an alternative method to represent precipitation distribution.

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## **CONCLUSIONS & RECOMMENDATIONS**

### **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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## 7. CONCLUSIONS AND RECOMMENDATIONS

The *primary objective* of this modelling process was the application of the streamflow generation component of the HSPF model in order to provide hydrological information essential to those responsible for planning, development and management of the Msunduzi River Catchment. The *focus* of this modelling process was on the development and implementation of all input data and the testing of HSPF's continuous modelling system to correctly represent the hydrological components of the hydrological cycle.

The specific objectives were to:

- Collect and process input data and information for the modelling system.
- Develop and establish a Geographical Information System (GIS) containing the relevant spatial information for the Msunduzi River Catchment.
- Develop the hydrological modelling system to simulate all hydrological processes of the catchment successfully in terms of water quantity. This effort included discretisation of the entire catchment, extensive preparation of hydrological- and meteorological data and development of the stage-discharge relationships of the rivers.
- Verify the streamflow simulations against observed data at a daily level where available and evaluate the performance of the modelling system by making use of an array of goodness-of-fit-statistics.

## **7.1 DATA COLLECTION AND PREPARATION**

HSPF demands a vast amount of data, especially when water quality simulations are being considered. Data collection was considerably simplified by having access to the CCWR computer system and software developed at the CCWR.

The stand-alone programs ANNIE (Interactive Hydrological Analyses and Data Management) and IOWDM (Input and Output for Watershed Data Management File), which were developed by the USGS for loading and manipulating data in the WDM file can be tedious to use. This is especially true when dealing with large numbers of time series. A great deal of automation can be achieved by automating the processes with general purpose tools such as Unix “awk” and “sed” commands and shell scripts, which were used extensively during the data preparation phase. Fortunately, both ANNIE and IOWDM lend themselves toward this kind of automation.

## **7.2 STREAMFLOW SIMULATION AND CALIBRATION**

The areal distribution of the main land-use groups contributing to the different hydrological gauging stations within each of the sub-catchments played an important role in distributing the runoff.

Model parameters should not account for major errors in input and output time series. Streamflow measurements are only accurate below the upper limit of accurately measured data of hydrological gauging stations. Any part of the hydrographs above these limits should therefore be discounted. Observed data measured during prevailing conditions of submergence must be verified with conventional current gaugings.

The greatest difficulty in obtaining accurate streamflow simulations was due to the lack of good streamflow records, especially for U2H011. The lack of data on water releases from Henley Dam also made good simulation at this hydrological gauging station difficult.

The margin of error in the simulated annual water balance varied between two and 11% during calibration, while it varied between five and 16% during the period of verification. There is a slight bias in the seasonal calibration for under-estimating the streamflow in the upper sub-catchments during the wet period and over-estimating the baseflow during autumn and follow-on dry periods. In the lower sub-catchments, the wet periods were over-simulated and the baseflow was correctly simulated during the dry period.

The over-simulation of the various single storm events (higher streamflow peaks) was the most consistent error that occurred during the period of calibration. This can probably be ascribed to the poor representation and areal distribution of precipitation data, which did not account accurately for the spatial variation in precipitation- and storm distributions. The results of the scatter plots indicated that there is a low correlation between the different precipitation data sets, thus emphasising the spatial variation in precipitation- and storm distributions. Individual hydrographs were best simulated at U2H022 and this might be linked to the more representative precipitation data (highest  $r^2$ -values) within those sub-catchments.



Even when the precipitation stations have a low degree of correlation, they are still representative of the parts of the catchment for which they provide precipitation distribution as determined by the Thiessen polygons. The question is really whether the precipitation stations are representative of the precipitation distribution suggested by the Thiessen polygons. Since there is bound to be variation in areal distribution in precipitation within a given polygon, given the small number of precipitation stations, a single station is considered to be a poor representative of the entire polygon. The only solution to this problem is a more representative distribution of precipitation stations, or using an alternative method to represent precipitation distribution.

The over-estimation of baseflows could be due to inadequate representation of the effects of riparian vegetation in the model. The parameter, which is used to account for riparian vegetation is BASETP. Alternatively, the KVARV parameter, which was considered to be constant in this study, could play a role.

The response time and interflow recession rates of the hydrographs were accurately simulated, except for some cases during autumn where the interflow recession rate was incorrectly simulated. Therefore, AGWRC and IRC must be decreased in order to accomplish faster recession in the recession tails of the simulated storm hydrographs in autumn- or follow-on winter baseflow. Decreasing the IRC value should “steepen” the recession and increase peak flows.

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### 7.3 ANALYSIS OF MODEL PERFORMANCE

The reported value ranges of the coefficient of determination ( $r^2$ ) for other HSPF applications in the literature were used as a guideline of model performance analysis. The objective was to maximise the coefficient of determination ( $r^2$ ) to unity, or at least to the proposed value of 0.6 at a daily level.

As in the case of calibration and verification, the best model performance was associated with the best hydrological- and meteorological data. The model performance at U2H022 is a typical example, because the best input data, compared to the other hydrological gauging stations, were available at U2H022. The final  $r^2$  values of U2H022 were within the acceptable range of 0.6 at a daily level.

### 7.4 RECOMMENDATIONS

Recommendations for future refinements of the Msunduzi River Catchment HSPF model are as follows:

- Collect more representative river geometry data in order to improve the stage-discharge relationships, especially when in-depth water quality studies are to be performed in future. Many water quality processes depend on river depth. The river cross-sections used to compute the stage-discharge relationships in HSPF were based on very limited data, viz. GIS data, topographical maps, orthographical photographs and topographical- and section surveys. Although the hydrological calibration is not very sensitive to river geometry.

- 
- ❑ Further streamflow calibration by adjusting the BASETP parameters and extended examination of areal photographs, GIS data and field visits to determine the areas most likely to be affected by riparian vegetation.
  - ❑ Collect hourly meteorological data throughout the catchment. New precipitation estimation techniques such as spatial radar precipitation estimation and improved real-time precipitation data can be incorporated to improve the precipitation data. This would also permit simulation of processes, which are sensitive to diurnal changes, such as water temperature and other quality constituents. It would also allow for more accurate simulation of precipitation events in smaller catchments, such as the Slang Spruit.
  - ❑ The finer discretisation of sub-catchments based on the resolution of accurate precipitation estimation and the hydrological response of the different land-uses.
  - ❑ The use of PEST, a model-independent, non-linear parameter estimator to improve the final calibrated values (Doherty, Brebber & Whyte, 1994). PEST also allows confidence limits to be set on parameter estimates, allowing for a more reliable application of the model as a predictive tool.
  - ❑ The simulation of water quality constituents to assess the impact of point source- and diffuse sources of pollution.
  - ❑ Human water use. This model incorporates the effects of water abstractions from Henley Dam and the discharge of Darvill wastewater treatment plant. However, if the model is used to simulate the effects of increased pumping or wastewater discharge, water consumption of all the urban- and informal areas of Pietermaritzburg, Cato Ridge, Vulindlela and other townships will need to be incorporated.



- 
- The automation and improvement of integration between the GIS and HSPF by incorporating stand alone processes in the GIS or modelling system, utilising the GIS for the display of simulation results and further development to enable interactive scenario assessment. Many of these goals can be achieved by the implementation of BASINS, a multipurpose environmental analysis system, which provides an integrated catchment framework, to support and standardise all the processes of hydrological- and water quality modelling. The GENSCN tool can also be used for integration of GIS information and HSPF (Kittle, Lumb, Hummel, Duda, & Gray, 1998).

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### HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT

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# **ANNEXURES**

## **HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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## 9. ANNEXURES

**ANNEXURE A: PLATES 1 to 9: SPATIAL GIS DATA OF THE MSUNDUZI RIVER CATCHMENT**

**ANNEXURE B: PLATES 10 to 24 b: PHOTOGRAPHS OF THE MSUNDUZI RIVER CATCHMENT**

**ANNEXURE C: LIST OF USER'S CONTROL INPUT FILES AND INPUT DATA OF THE MSUNDUZI RIVER CATCHMENT**

This annexure lists the final User's Control Input files developed for the Msunduzi River Catchment. Two separate files were used to simulate the upper- and lower catchments.



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# **SPATIAL GIS DATA OF THE MSUNDUZI RIVER CATCHMENT**

**HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

# MSUNDUZI RIVER CATCHMENT Management Sub-Catchments

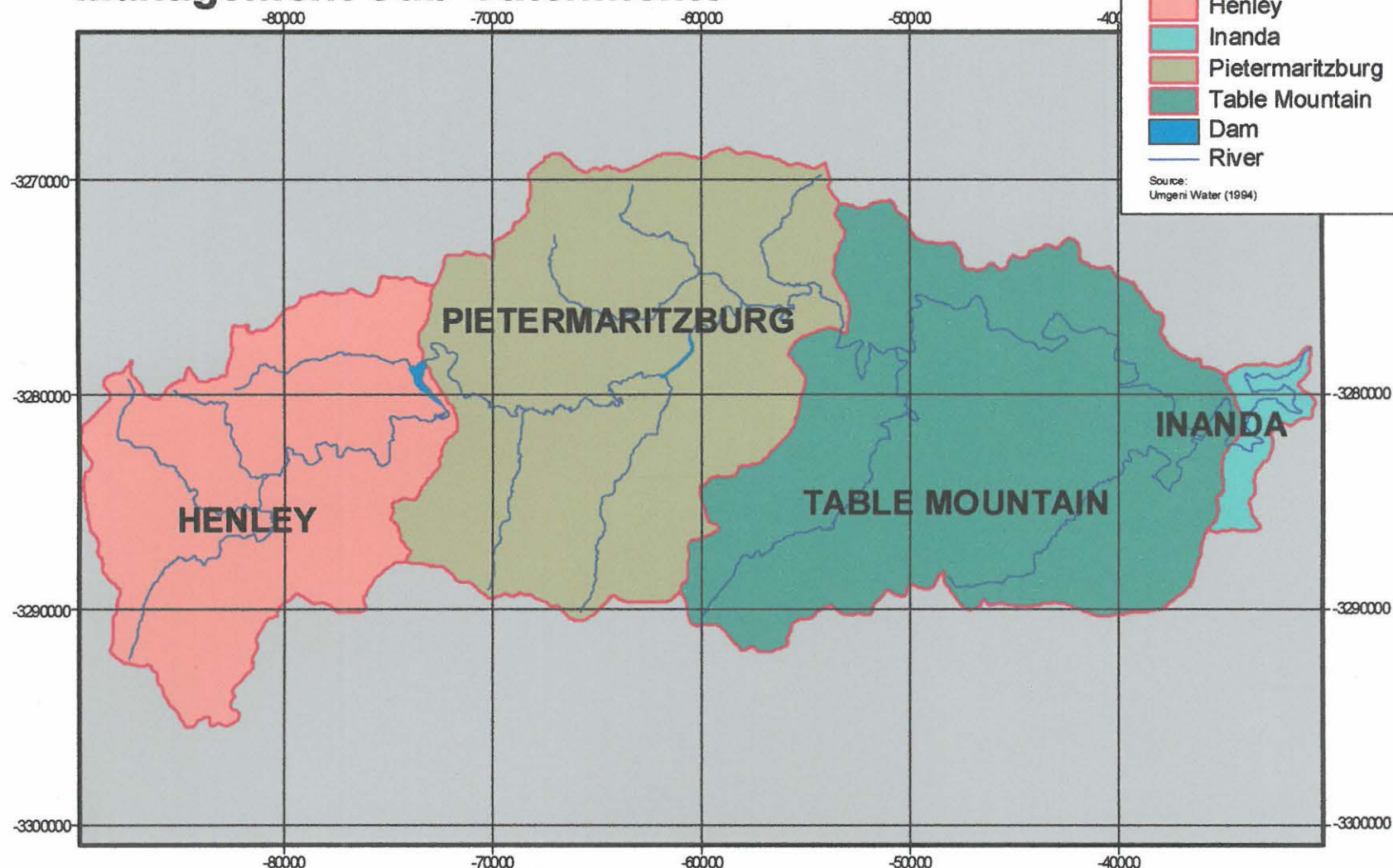
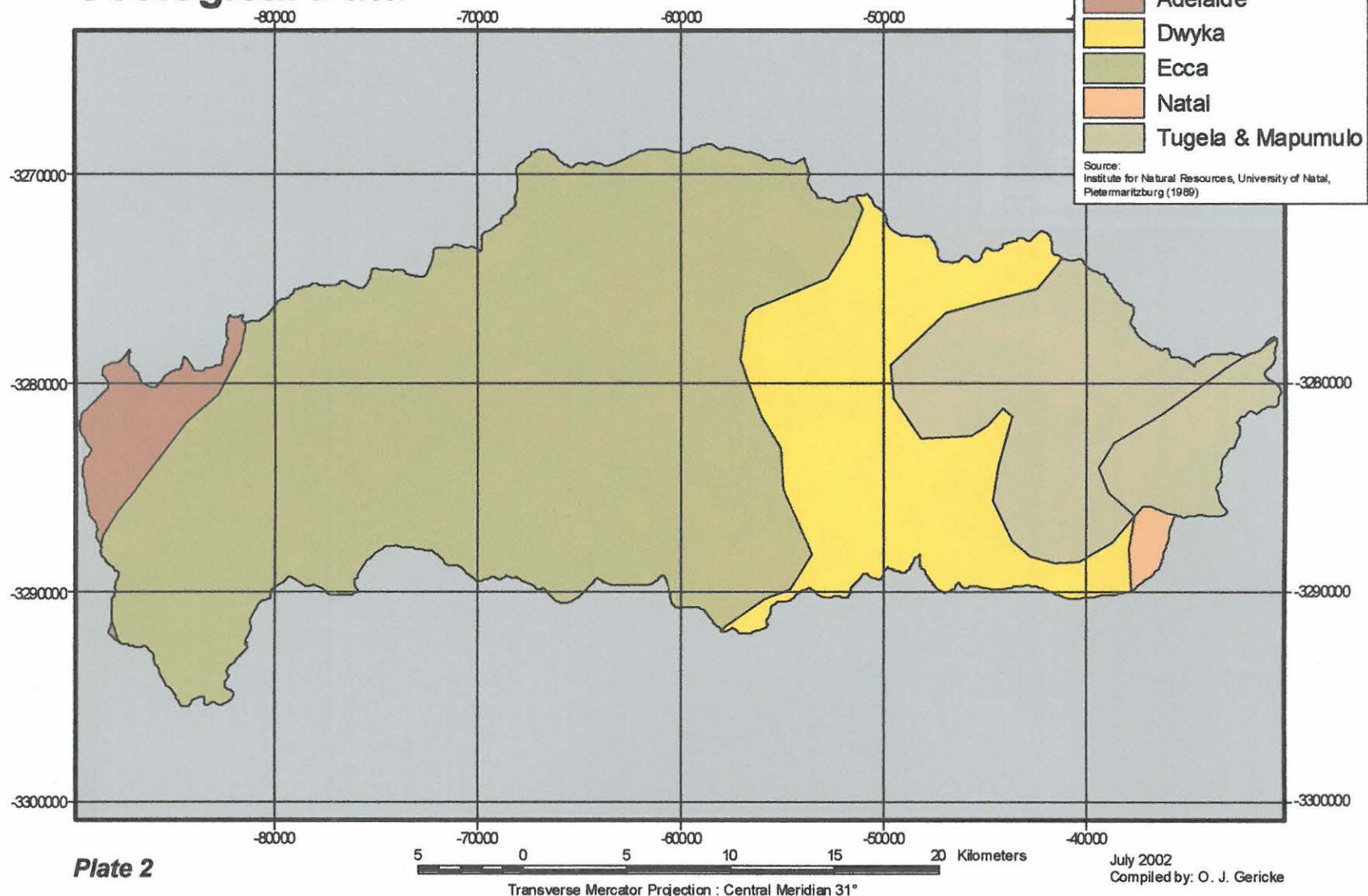


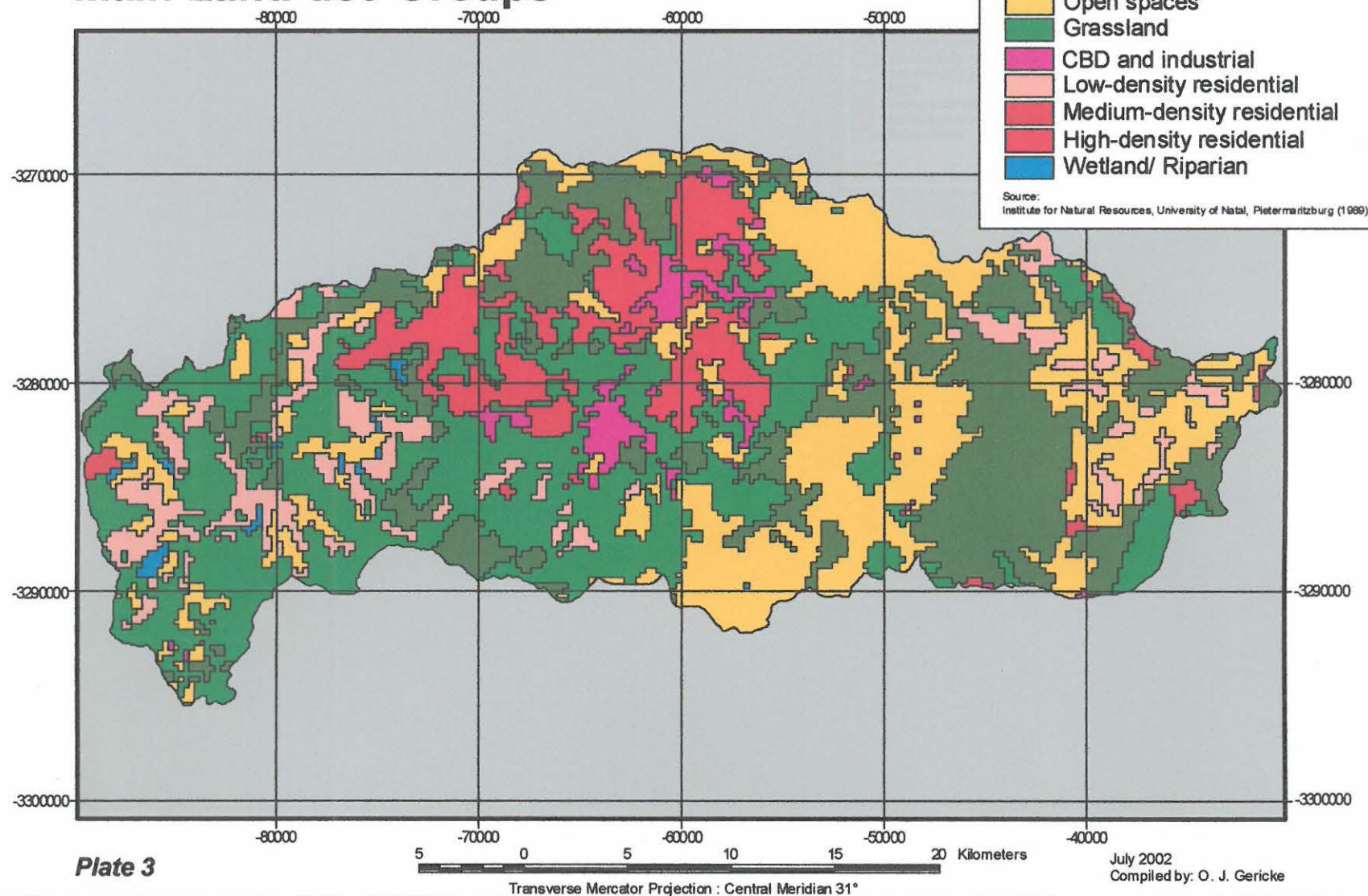
Plate 1

# MSUNDUZI RIVER CATCHMENT Geological Data



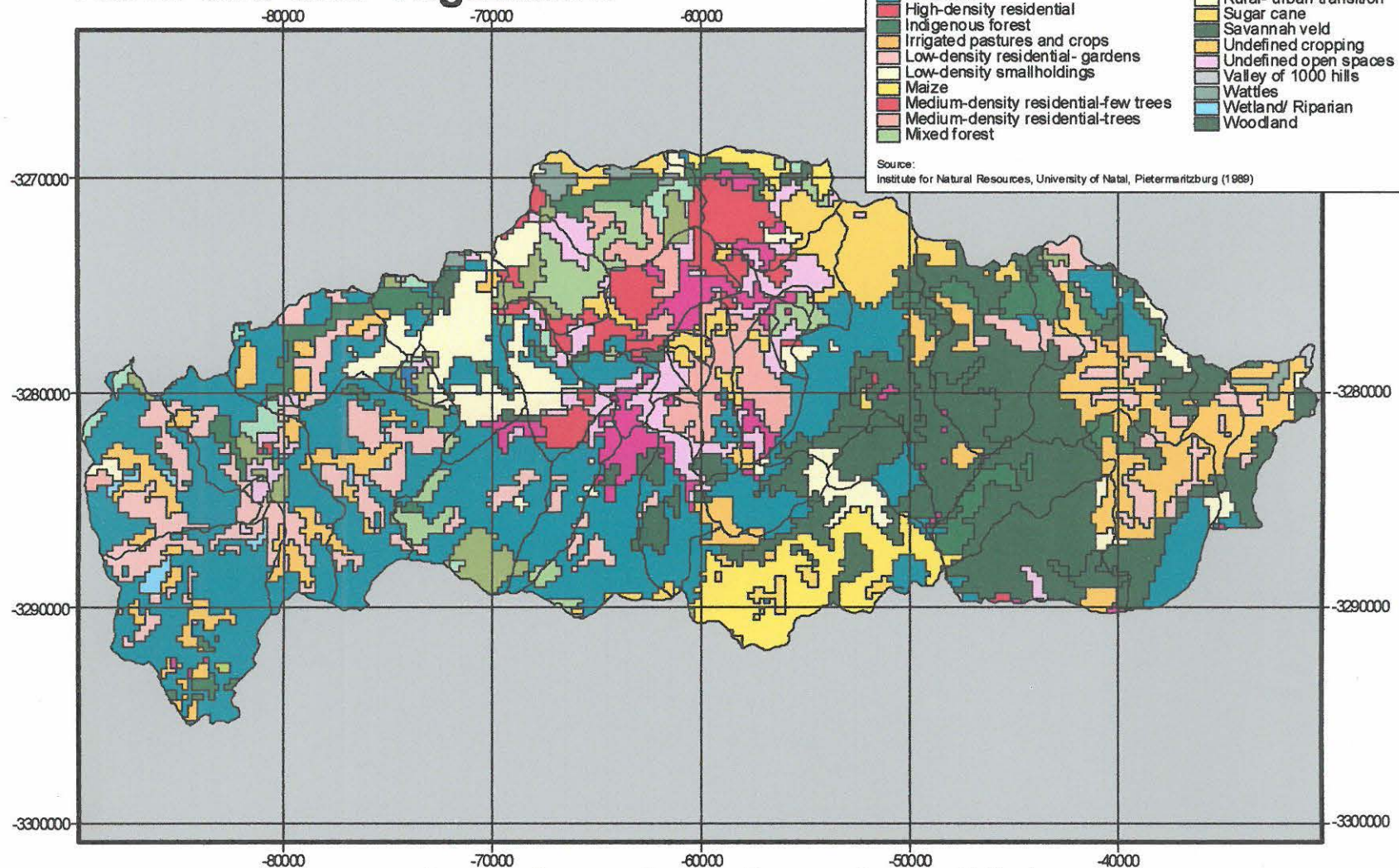


# MSUNDUZI RIVER CATCHMENT Main Land-use Groups

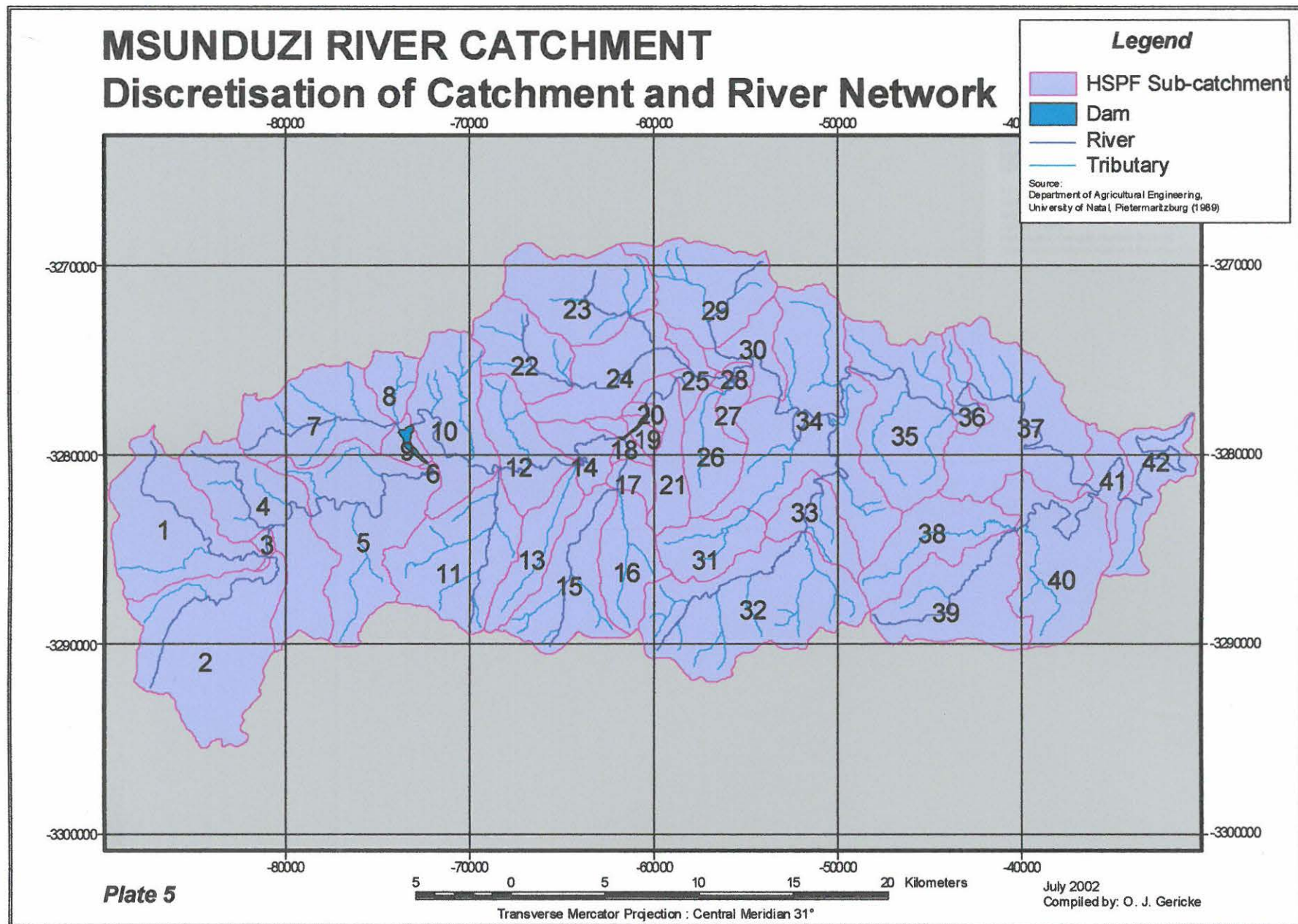




# MSUNDUZI RIVER CATCHMENT Land-use and Vegetation

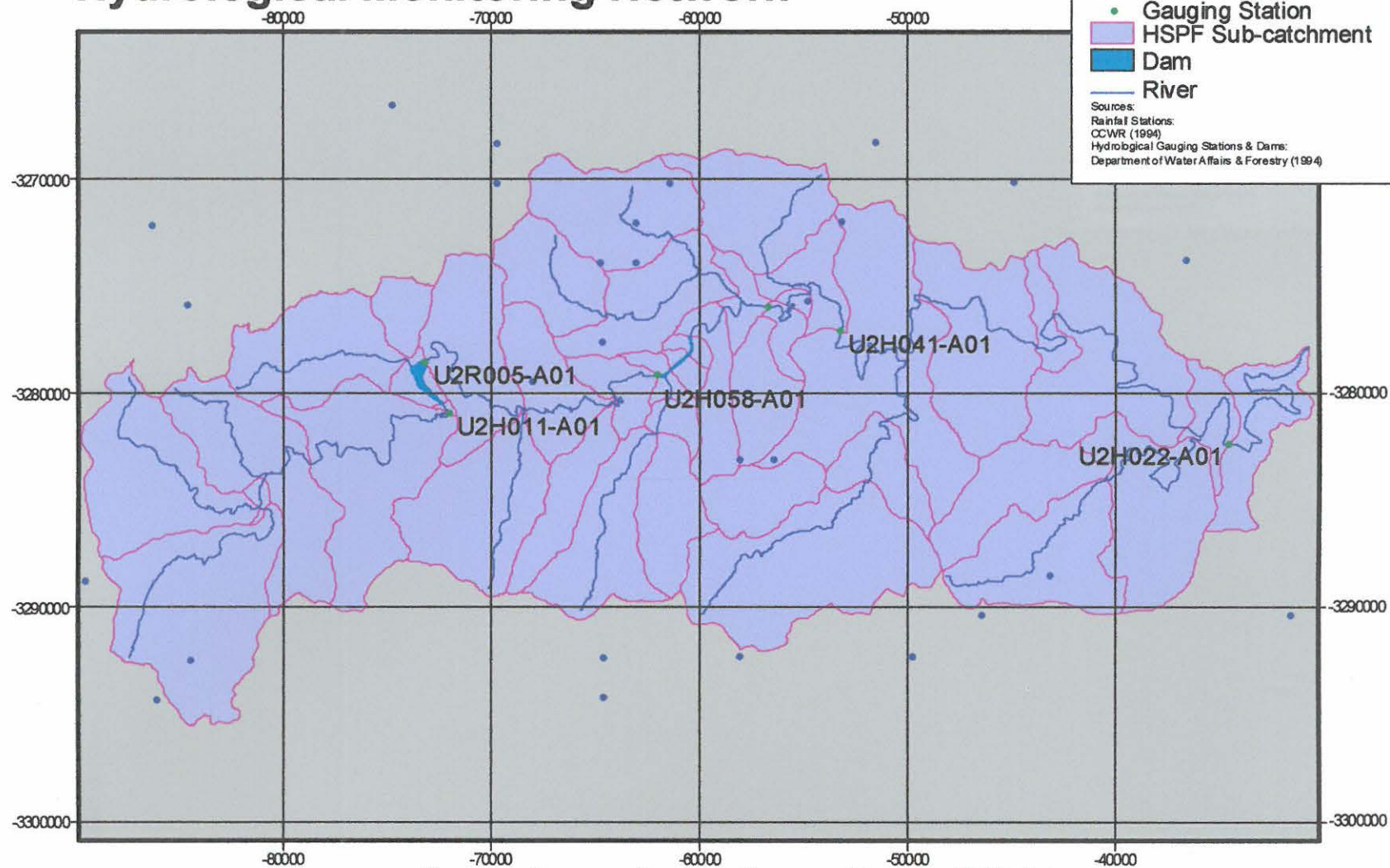




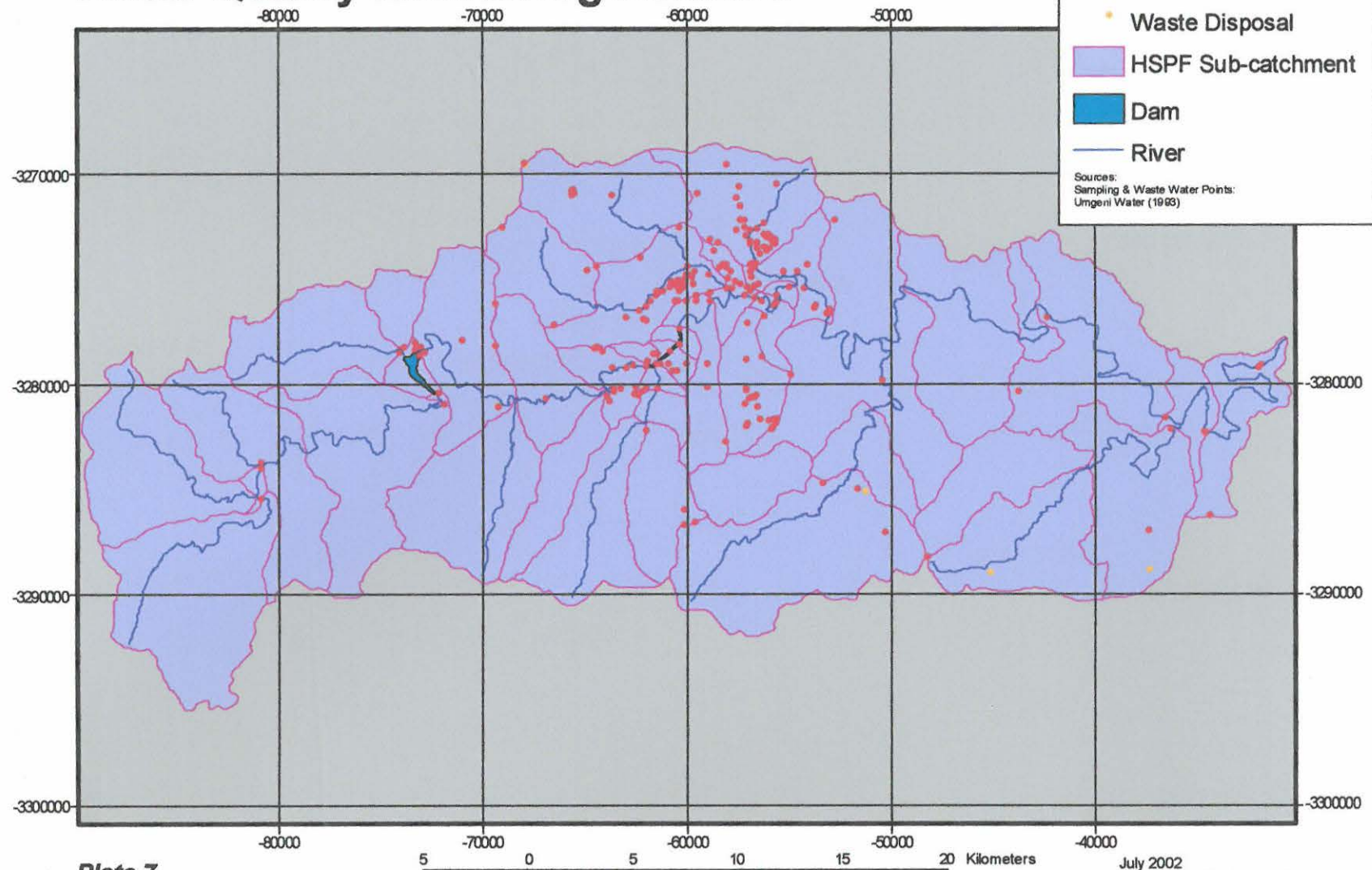




# MSUNDUZI RIVER CATCHMENT Hydrological Monitoring Network



# MSUNDUZI RIVER CATCHMENT Water Quality Monitoring Network





# MSUNDUZI RIVER CATCHMENT Thiessen Polygons

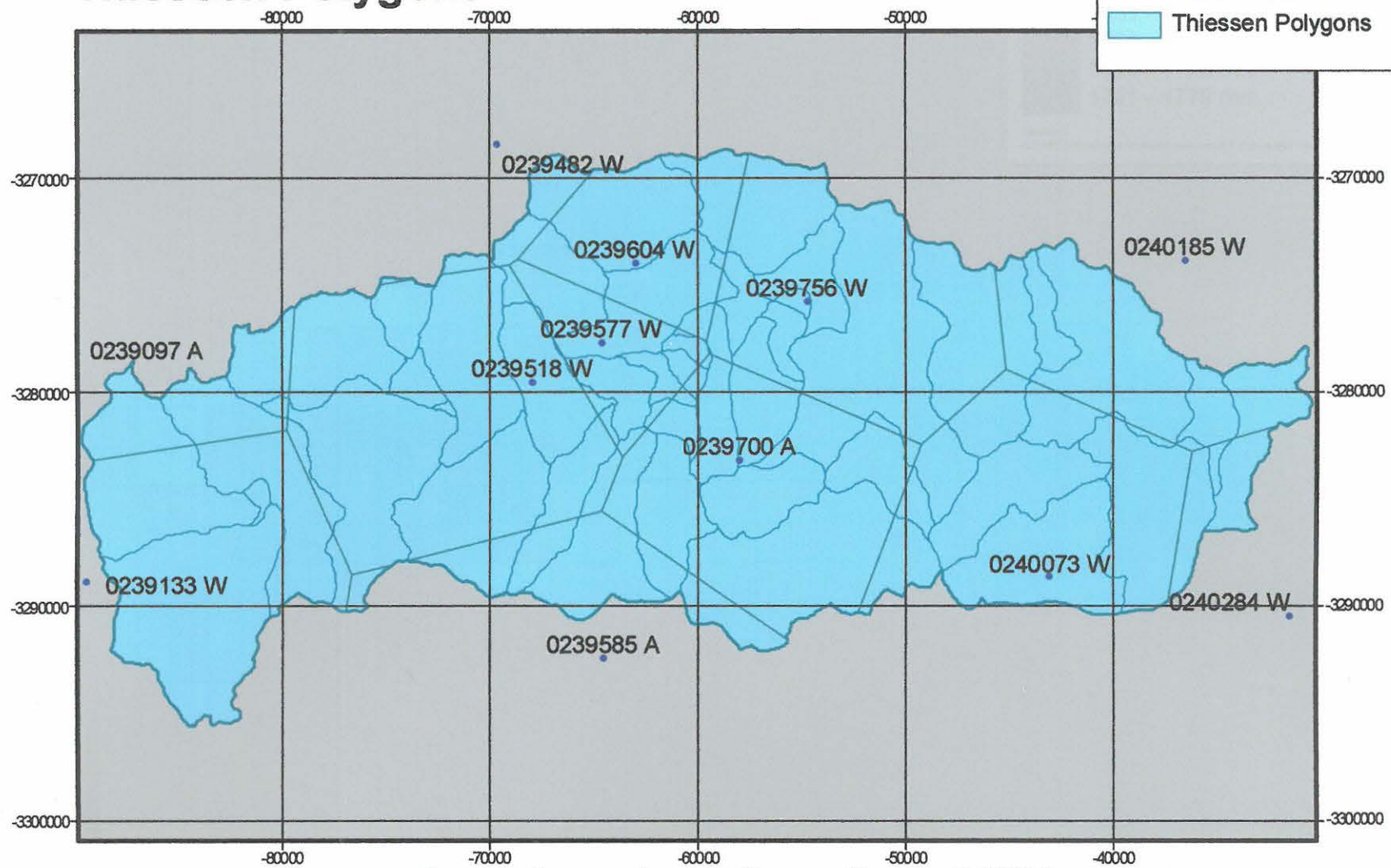
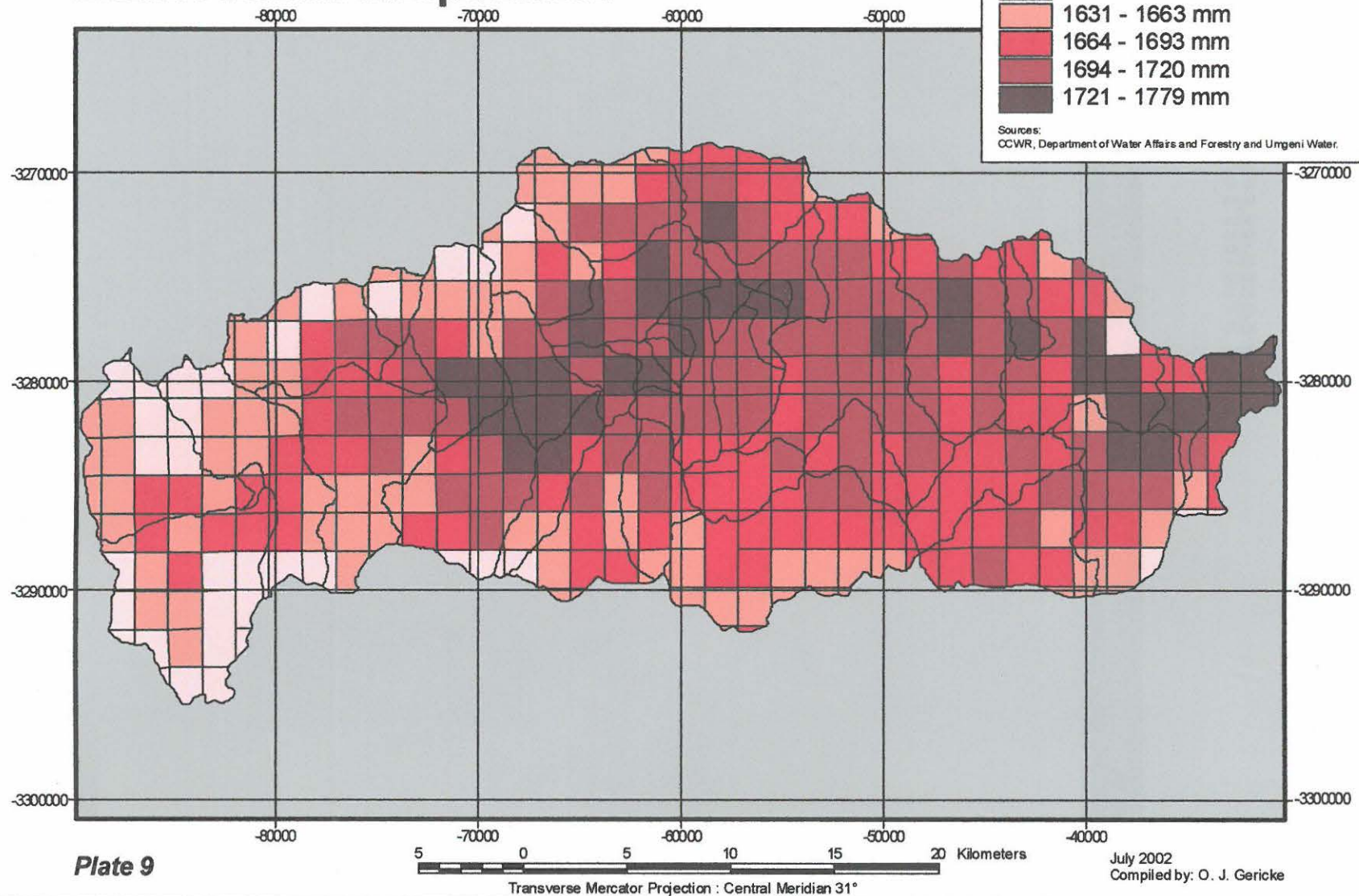


Plate 8

July 2002  
Compiled by: O. J. Gericke



# MSUNDUZI RIVER CATCHMENT Mean Annual Evaporation

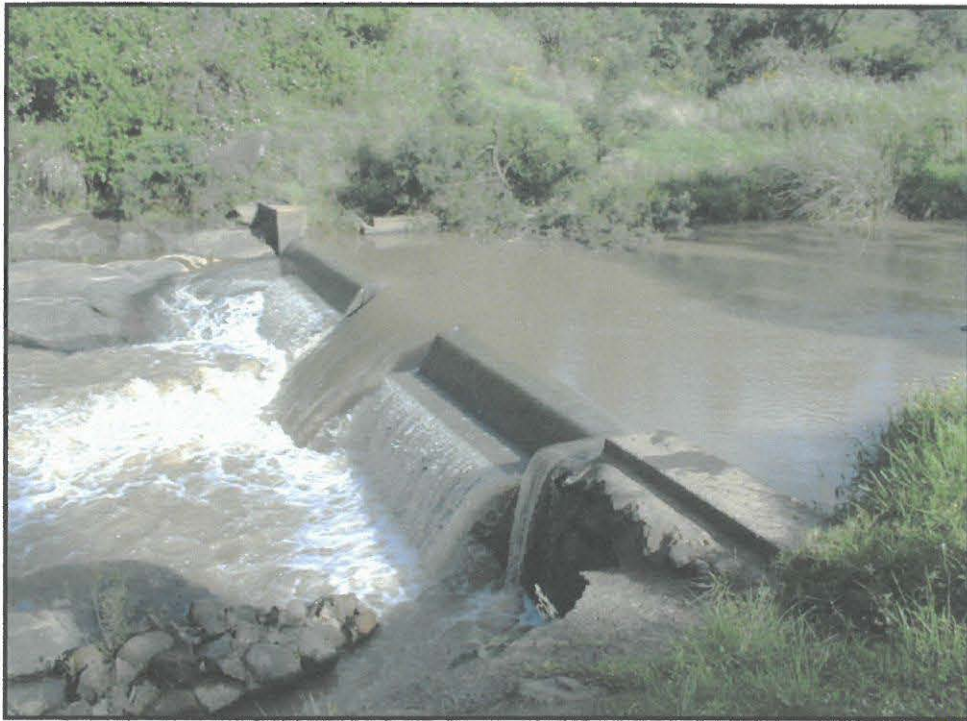


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# **PHOTOGRAPHS OF THE MSUNDUZI RIVER CATCHMENT**

**HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**





**Plate 10: U2H011 Msunduzi River at Henley Dam: Sub-catchment 5**

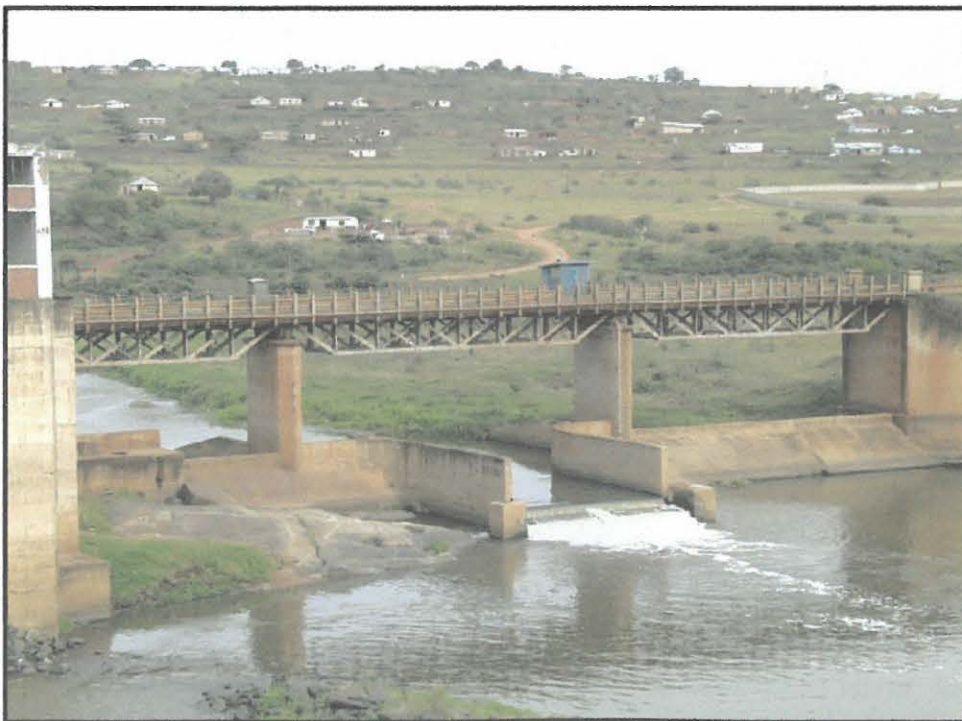


**Plate 11: U2H058 Msunduzi River at Mason's Mill: Sub-catchment 14**





**Plate 12: U2H041 Msunduzi River at Hampstead Park, Moto-X: Sub-catchment 30**



**Plate 13: U2H022 Msunduzi River at Nomfihlelo: Sub-catchment 42**



**Plate 14:** Silt trapping weir upstream of Camp's Drift bridge: Sub-catchment 18. Msunduzi River



**Plate 15:** Camp's Drift Road: Sub-catchment 19. Msunduzi River. Upstream view of canalised recreational area above Camp's Drift weir towards silt trapping weir





**Plate 16 a: Camp's Drift weir at College Road: Sub-catchment 20. Msunduzi River. Multiple notches. Ogee structure with an overflow of approximately 65 to 70 m**



**Plate 16 b: Camp's Drift weir at College Road: Sub-catchment 20. Msunduzi River. Typical downstream conditions are a defined river section of approximately 57 m**





**Plate 17: Railway bridge at Ritchie Road:** Sub-catchment 21. Reaches of the Foxhill Spruit. Uniform channel, defined flow path covered with grass and reeds. Approximate width of 20.6 m



**Plate 18 a: Alexandra Park pedestrian bridge, near Gutridge- and Prince Alfred Streets:** Sub-catchment 21. Msunduzi River. Upstream view. Uniform channel with trapezoidal section. Approximate top width of 30 m and bottom width of 14 m. Rocky bed conditions with contractions upstream

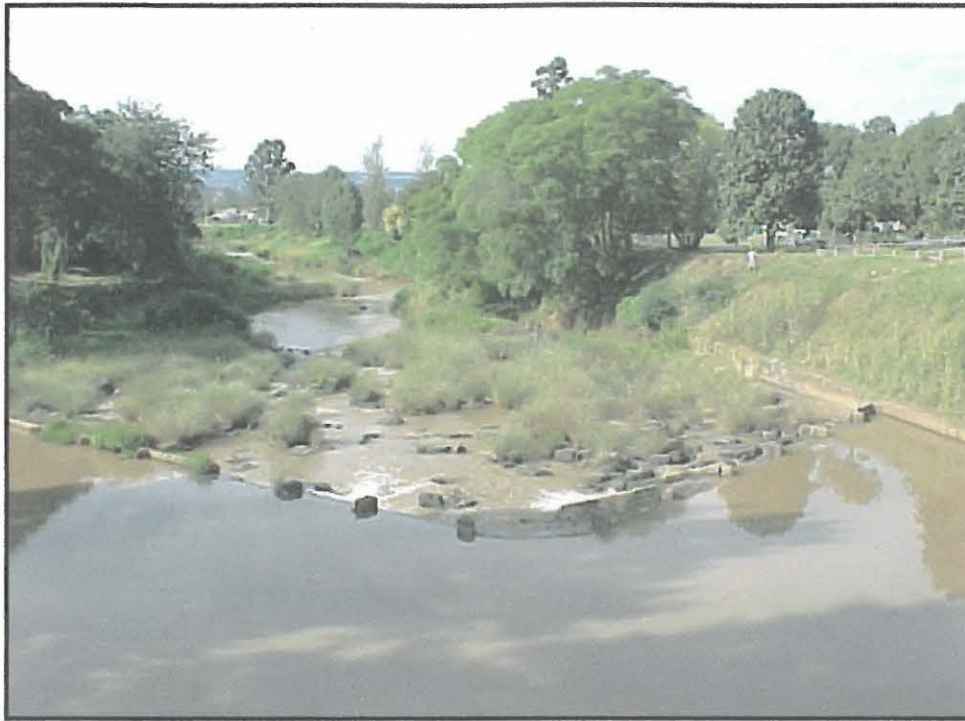




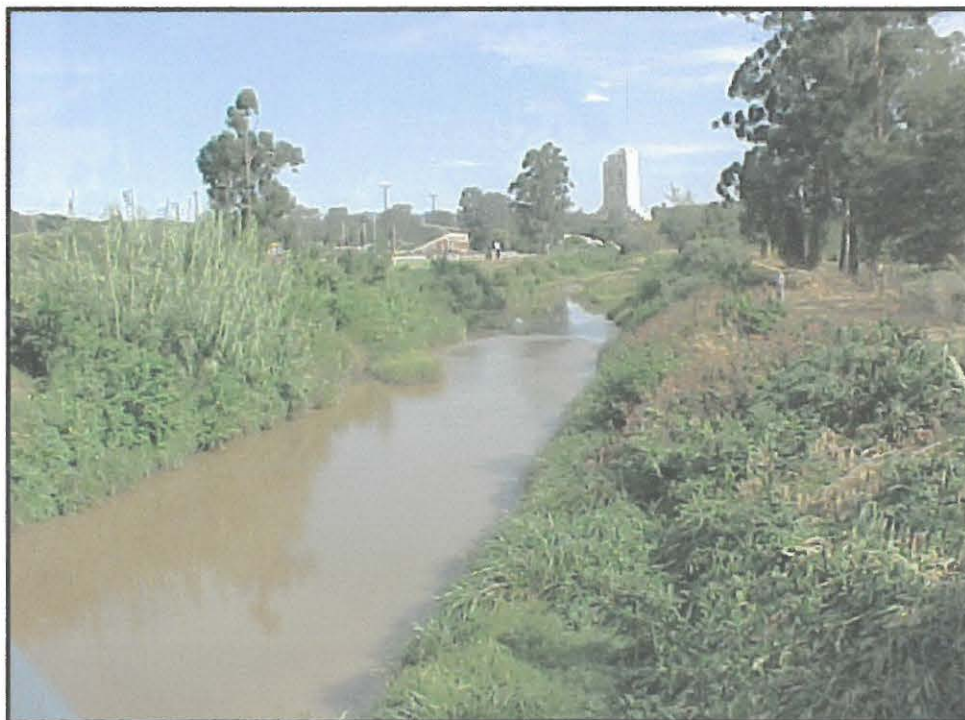
**Plate 18 b: Alexandra Park pedestrian bridge, near Gutridge- and Prince Alfred Streets:** Sub-catchment 21. Msunduzi River. Downstream view. Uniform channel with trapezoidal section. Approximate top width of 30 m and bottom width of 14 m



**Plate 19 a: Durban Road bridge at Durban Road near Kershaw Park:** Sub-catchment 21. Msunduzi River. Uniform channel with trapezoidal section. Approximate top width of 39.5 m and bottom width of 29 m. Upstream section



**Plate 19 b: Durban Road bridge at Durban Road near Kershaw Park: Sub-catchment 21. Msunduzi River. Downstream section**



**Plate 20 a: Daniel Lindley bridge: Boshoff Street: Sub-catchment 21. Msunduzi River. Downstream of Durban Road bridge. Upstream section**





**Plate 20 b: Daniel Lindley bridge: Boshoff Street: Sub-catchment 21. Msunduzi River. Upstream section**



**Plate 21: Upstream of Howick Road bridge, near Royal Show grounds: Sub-catchment 24. Lower reaches of the Dorp Spruit. Uniform channel with trapezoidal section. Approximate top width of 10 m and bottom width of 7 m**



**Plate 22:** Upstream view from Connor Road bridge: Sub-catchment 24. Reaches of the Chase Valley Stream. Uniform channel with trapezoidal section. Approximate top width of 17 m and bottom width of 8 m



**Plate 23 a:** Low-water bridge between Woodhouse- and Promed Roads: Sub-catchment 27. Reaches of the Msunduzi River. Uniform channel with trapezoidal section. Approximate top width of 35 m and bottom width of 25 m. Upstream section





**Plate 23 b: Low-water bridge between Woodhouse- and Promed Roads:**  
Sub-catchment 27. Downstream section



**Plate 24 a: Low-water bridge at Grimthorpe Avenue near Hampstead Park:**  
Sub-catchment 34. Reaches of the Msunduzi River. Uniform  
channel with trapezoidal section. Approximate top width of 25 m  
and bottom width of 14 m. Upstream section





**Plate 24 b: Low-water bridge at Grimthorpe Avenue near Hampstead Park:**  
Sub-catchment 34. Reaches of the Msunduzi River.  
Downstream section

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# **USER'S CONTROL INPUT FILES AND INPUT DATA OF THE MSUNDUZI RIVER CATCHMENT**

**HSPF MODELLING OF THE MSUNDUZI RIVER CATCHMENT**

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# USER' S CONTROL INPUT FILES

## UPPER SUB-CATCHMENTS (1-20)

RUN

GLOBAL

**HSPF DEMONSTRATION RUN UPSTREAM OF MGENI CONFLUENCE**

**START 1992/01/01 END 2000/06/01**

RUN INTERP OUTPUT LEVEL 4

RESUME 0 RUN 1 TSSFL 0 WDMSFL 0 UNITS 2

END GLOBAL

FILES

<FILE> <UN#>\*\*\*<---FILE NAME----->

WDM 21 msunduzi.wdm

MESSU 22 msunduzi.ech

61 msunduzi.p61

62 msunduzi.p62

63 msunduzi.p63

64 msunduzi.p64

92 msunduzi.plt

END FILES

OPN SEQUENCE

INGRP INDELT 24:00

Catchment 1 \*\*\*\*\*

PERLND 11

PERLND 12

PERLND 13

PERLND 14\*\*\*

IMPLND 14\*\*\*

PERLND 15

IMPLND 15

PERLND 16

PERLND 17\*\*\*

IMPLND 17\*\*\*

PERLND 18

RCHRES 1

Catchment 2 \*\*\*\*\*

PERLND 21

PERLND 22

PERLND 23

PERLND 24

IMPLND 24

PERLND 25\*\*\*

PERLND 26

PERLND 27\*\*\*

PERLND 28

RCHRES 2

Catchment 3 \*\*\*\*\*

PERLND 31

PERLND 32\*\*\*

PERLND 33

PERLND 34\*\*\*

PERLND 35\*\*\*

PERLND 36

PERLND 37\*\*\*

PERLND 38\*\*\*

RCHRES 3

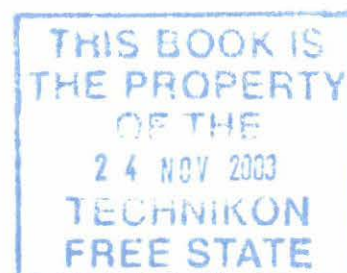
Catchment 4 \*\*\*\*\*

PERLND 41

PERLND 42

PERLND 43

PERLND 44\*\*\*





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PERLND	45***
PERLND	46
PERLND	47
IMPLND	47
PERLND	48
RCHRES	4
Catchment	5 *****
PERLND	51
PERLND	52
PERLND	53
PERLND	54***
PERLND	55
IMPLND	55
PERLND	56
PERLND	57***
PERLND	58
RCHRES	5
Catchment	6 *****
PERLND	61
PERLND	62***
PERLND	63
PERLND	64***
PERLND	65
IMPLND	65
PERLND	66***
PERLND	67***
PERLND	68***
RCHRES	6
Catchment	7 *****
PERLND	71
PERLND	72
PERLND	73
PERLND	74***
PERLND	75
IMPLND	75
PERLND	76
PERLND	77***
PERLND	78***
RCHRES	7
Catchment	8 *****
PERLND	81
PERLND	82
PERLND	83
PERLND	84***
PERLND	85
IMPLND	85
PERLND	86***
PERLND	87***
PERLND	88***
RCHRES	8
Catchment	9 *****
PERLND	91
PERLND	92***
PERLND	93
PERLND	94***
PERLND	95
IMPLND	95
PERLND	96***
PERLND	97***
PERLND	98***
RCHRES	9
COPY	1
Catchment	10 *****
PERLND	101
PERLND	102
PERLND	103
PERLND	104
IMPLND	104

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PERLND	105
IMPLND	105
PERLND	106
PERLND	107
IMPLND	107
PERLND	108***
RCHRES	10
Catchment	11 *****
PERLND	111
PERLND	112***
PERLND	113
PERLND	114
IMPLND	114
PERLND	115
IMPLND	115
PERLND	116
PERLND	117
IMPLND	117
PERLND	118***
RCHRES	11
Catchment	12 *****
PERLND	121
PERLND	122***
PERLND	123
PERLND	124
IMPLND	124
PERLND	125
IMPLND	125
PERLND	126***
PERLND	127
IMPLND	127
PERLND	128***
RCHRES	12
Catchment	13 *****
PERLND	131
PERLND	132***
PERLND	133
PERLND	134
IMPLND	134
PERLND	135
IMPLND	135
PERLND	136
PERLND	137
IMPLND	137
PERLND	138***
RCHRES	13
Catchment	14 *****
PERLND	141
PERLND	142***
PERLND	143
PERLND	144
IMPLND	144
PERLND	145
PERLND	146***
PERLND	147***
PERLND	148***
RCHRES	14
Catchment	15 *****
PERLND	151
PERLND	152
PERLND	153
PERLND	154
IMPLND	154
PERLND	155
IMPLND	155
PERLND	156
PERLND	157***
PERLND	158***

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RCHRES      15
Catchment   16 *****
PERLND      161
PERLND      162
PERLND      163
PERLND      164
IMPLND      164
PERLND      165***
PERLND      166
PERLND      167***
PERLND      168***
RCHRES      16
Catchment   17 *****
PERLND      171***
PERLND      172***
PERLND      173
PERLND      174
IMPLND      174
PERLND      175
IMPLND      175
PERLND      176***
PERLND      177***
PERLND      178***
RCHRES      17
Catchment   18 *****
PERLND      181
PERLND      182***
PERLND      183
PERLND      184
IMPLND      184
PERLND      185
IMPLND      185
PERLND      186***
PERLND      187
IMPLND      187
PERLND      188***
RCHRES      18***
Catchment   19 *****
PERLND      191***
PERLND      192
PERLND      193
PERLND      194***
PERLND      195
IMPLND      195
PERLND      196***
PERLND      197***
PERLND      198***
RCHRES      19***
Catchment   20 *****
PERLND      201
PERLND      202
PERLND      203
PERLND      204***
PERLND      205
IMPLND      205
PERLND      206***
PERLND      207
IMPLND      207
PERLND      208***
RCHRES      20
PLTGEN      1***
END INGRP
END OPN SEQUENCE

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COPY

TIMESERIES  
# # NPT NMN \*\*\*  
1 1  
END TIMESERIES  
END COPY

PERLND

ACTIVITY  
<PLS > Active Sections (1=Active; 0=Inactive) \*\*\*  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*  
11 208 1  
END ACTIVITY

PRINT-INFO  
<PLS > Print-flags \*\*\* PIVL PYR  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*  
11 208 5 5 09  
END PRINT-INFO

GEN-INFO  
<PLS ><-----Name-----> Unit-systems Printer \*\*\*  
# - # t-series Engl Metr \*\*\*  
in out \*\*\*

11	Catch	1, Forest	2	2	0	61
12	Catch	1, Crops	2	2	0	61
13	Catch	1, Grassland	2	2	0	61
14	Catch	1, CBD	2	2	0	61
15	Catch	1, MD Res.	2	2	0	61
16	Catch	1, LD Res.	2	2	0	61
17	Catch	1, HD Res.	2	2	0	61
18	Catch	1, Wetland	2	2	0	61
21	Catch	2, Forest	2	2	0	61
22	Catch	2, Crops	2	2	0	61
23	Catch	2, Grassland	2	2	0	61
24	Catch	2, CBD	2	2	0	61
25	Catch	2, MD Res.	2	2	0	61
26	Catch	2, LD Res.	2	2	0	61
27	Catch	2, HD Res.	2	2	0	61
28	Catch	2, Wetland	2	2	0	61
31	Catch	3, Forest	2	2	0	61
32	Catch	3, Crops	2	2	0	61
33	Catch	3, Grassland	2	2	0	61
34	Catch	3, CBD	2	2	0	61
35	Catch	3, MD Res.	2	2	0	61
36	Catch	3, LD Res.	2	2	0	61
37	Catch	3, HD Res.	2	2	0	61
38	Catch	3, Wetland	2	2	0	61
41	Catch	4, Forest	2	2	0	61
42	Catch	4, Crops	2	2	0	61
43	Catch	4, Grassland	2	2	0	61
44	Catch	4, CBD	2	2	0	61
45	Catch	4, MD Res.	2	2	0	61
46	Catch	4, LD Res.	2	2	0	61
47	Catch	4, HD Res.	2	2	0	61
48	Catch	4, Wetland	2	2	0	61
51	Catch	5, Forest	2	2	0	61
52	Catch	5, Crops	2	2	0	61
53	Catch	5, Grassland	2	2	0	61
54	Catch	5, CBD	2	2	0	61
55	Catch	5, MD Res.	2	2	0	61
56	Catch	5, LD Res.	2	2	0	61
57	Catch	5, HD Res.	2	2	0	61
58	Catch	5, Wetland	2	2	0	61
61	Catch	6, Forest	2	2	0	61
62	Catch	6, Crops	2	2	0	61
63	Catch	6, Grassland	2	2	0	61

64	Catch 6, CBD	2	2	0	61
65	Catch 6, MD Res.	2	2	0	61
66	Catch 6, LD Res.	2	2	0	61
67	Catch 6, HD Res.	2	2	0	61
68	Catch 6, Wetland	2	2	0	61
71	Catch 7, Forest	2	2	0	61
72	Catch 7, Crops	2	2	0	61
73	Catch 7, Grassland	2	2	0	61
74	Catch 7, CBD	2	2	0	61
75	Catch 7, MD Res.	2	2	0	61
76	Catch 7, LD Res.	2	2	0	61
77	Catch 7, HD Res.	2	2	0	61
78	Catch 7, Wetland	2	2	0	61
81	Catch 8, Forest	2	2	0	61
82	Catch 8, Crops	2	2	0	61
83	Catch 8, Grassland	2	2	0	61
84	Catch 8, CBD	2	2	0	61
85	Catch 8, MD Res.	2	2	0	61
86	Catch 8, LD Res.	2	2	0	61
87	Catch 8, HD Res.	2	2	0	61
88	Catch 8, Wetland	2	2	0	61
91	Catch 9, Forest	2	2	0	61
92	Catch 9, Crops	2	2	0	61
93	Catch 9, Grassland	2	2	0	61
94	Catch 9, CBD	2	2	0	61
95	Catch 9, MD Res.	2	2	0	61
96	Catch 9, LD Res.	2	2	0	61
97	Catch 9, HD Res.	2	2	0	61
98	Catch 9, Wetland	2	2	0	61
101	Catch 10, Forest	2	2	0	61
102	Catch 10, Crops	2	2	0	61
103	Catch 10, Grassland	2	2	0	61
104	Catch 10, CBD	2	2	0	61
105	Catch 10, MD Res.	2	2	0	61
106	Catch 10, LD Res.	2	2	0	61
107	Catch 10, HD Res.	2	2	0	61
108	Catch 10, Wetland	2	2	0	61
111	Catch 11, Forest	2	2	0	61
112	Catch 11, Crops	2	2	0	61
113	Catch 11, Grassland	2	2	0	61
114	Catch 11, CBD	2	2	0	61
115	Catch 11, MD Res.	2	2	0	61
116	Catch 11, LD Res.	2	2	0	61
117	Catch 11, HD Res.	2	2	0	61
118	Catch 11, Wetland	2	2	0	61
121	Catch 12, Forest	2	2	0	61
122	Catch 12, Crops	2	2	0	61
123	Catch 12, Grassland	2	2	0	61
124	Catch 12, CBD	2	2	0	61
125	Catch 12, MD Res.	2	2	0	61
126	Catch 12, LD Res.	2	2	0	61
127	Catch 12, HD Res.	2	2	0	61
128	Catch 12, Wetland	2	2	0	61
131	Catch 13, Forest	2	2	0	61
132	Catch 13, Crops	2	2	0	61
133	Catch 13, Grassland	2	2	0	61
134	Catch 13, CBD	2	2	0	61
135	Catch 13, MD Res.	2	2	0	61
136	Catch 13, LD Res.	2	2	0	61
137	Catch 13, HD Res.	2	2	0	61
138	Catch 13, Wetland	2	2	0	61
141	Catch 14, Forest	2	2	0	61
142	Catch 14, Crops	2	2	0	61
143	Catch 14, Grassland	2	2	0	61
144	Catch 14, CBD	2	2	0	61
145	Catch 14, MD Res.	2	2	0	61
146	Catch 14, LD Res.	2	2	0	61
147	Catch 14, HD Res.	2	2	0	61

148	Catch 14, Wetland	2	2	0	61
151	Catch 15, Forest	2	2	0	61
152	Catch 15, Crops	2	2	0	61
153	Catch 15, Grassland	2	2	0	61
154	Catch 15, CBD	2	2	0	61
155	Catch 15, MD Res.	2	2	0	61
156	Catch 15, LD Res.	2	2	0	61
157	Catch 15, HD Res.	2	2	0	61
158	Catch 15, Wetland	2	2	0	61
161	Catch 16, Forest	2	2	0	61
162	Catch 16, Crops	2	2	0	61
163	Catch 16, Grassland	2	2	0	61
164	Catch 16, CBD	2	2	0	61
165	Catch 16, MD Res.	2	2	0	61
166	Catch 16, LD Res.	2	2	0	61
167	Catch 16, HD Res.	2	2	0	61
168	Catch 16, Wetland	2	2	0	61
171	Catch 17, Forest	2	2	0	61
172	Catch 17, Crops	2	2	0	61
173	Catch 17, Grassland	2	2	0	61
174	Catch 17, CBD	2	2	0	61
175	Catch 17, MD Res.	2	2	0	61
176	Catch 17, LD Res.	2	2	0	61
177	Catch 17, HD Res.	2	2	0	61
178	Catch 17, Wetland	2	2	0	61
181	Catch 18, Forest	2	2	0	61
182	Catch 18, Crops	2	2	0	61
183	Catch 18, Grassland	2	2	0	61
184	Catch 18, CBD	2	2	0	61
185	Catch 18, MD Res.	2	2	0	61
186	Catch 18, LD Res.	2	2	0	61
187	Catch 18, HD Res.	2	2	0	61
188	Catch 18, Wetland	2	2	0	61
191	Catch 19, Forest	2	2	0	61
192	Catch 19, Crops	2	2	0	61
193	Catch 19, Grassland	2	2	0	61
194	Catch 19, CBD	2	2	0	61
195	Catch 19, MD Res.	2	2	0	61
196	Catch 19, LD Res.	2	2	0	61
197	Catch 19, HD Res.	2	2	0	61
198	Catch 19, Wetland	2	2	0	61
201	Catch 20, Forest	2	2	0	61
202	Catch 20, Crops	2	2	0	61
203	Catch 20, Grassland	2	2	0	61
204	Catch 20, CBD	2	2	0	61
205	Catch 20, MD Res.	2	2	0	61
206	Catch 20, LD Res.	2	2	0	61
207	Catch 20, HD Res.	2	2	0	61
208	Catch 20, Wetland	2	2	0	61

END GEN-INFO

PWAT-PARM1

<PLS > PWATER variable monthly parameter value flags											***		
#	-	#	CSNO	RTOP	UZFG	VCS	VUZ	VNN	VIFW	VIRC	VLE	IFFC	***
12			0	0	0	1	0	0	0	0	1		
13			0	0	0	1	0	0	0	0	1		
22			0	0	0	1	0	0	0	0	1		
23			0	0	0	1	0	0	0	0	1		
32			0	0	0	1	0	0	0	0	1		
33			0	0	0	1	0	0	0	0	1		
42			0	0	0	1	0	0	0	0	1		
43			0	0	0	1	0	0	0	0	1		
52			0	0	0	1	0	0	0	0	1		
53			0	0	0	1	0	0	0	0	1		
62			0	0	0	1	0	0	0	0	1		
63			0	0	0	1	0	0	0	0	1		
72			0	0	0	1	0	0	0	0	1		
73			0	0	0	1	0	0	0	0	1		



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82      0      0      0      1      0      0      0      0      1
83      0      0      0      1      0      0      0      0      1
92      0      0      0      1      0      0      0      0      1
93      0      0      0      1      0      0      0      0      1
102     0      0      0      1      0      0      0      0      1
103     0      0      0      1      0      0      0      0      1
112     0      0      0      1      0      0      0      0      1
113     0      0      0      1      0      0      0      0      1
122     0      0      0      1      0      0      0      0      1
123     0      0      0      1      0      0      0      0      1
132     0      0      0      1      0      0      0      0      1
133     0      0      0      1      0      0      0      0      1
142     0      0      0      1      0      0      0      0      1
143     0      0      0      1      0      0      0      0      1
152     0      0      0      1      0      0      0      0      1
153     0      0      0      1      0      0      0      0      1
162     0      0      0      1      0      0      0      0      1
163     0      0      0      1      0      0      0      0      1
172     0      0      0      1      0      0      0      0      1
173     0      0      0      1      0      0      0      0      1
182     0      0      0      1      0      0      0      0      1
183     0      0      0      1      0      0      0      0      1
192     0      0      0      1      0      0      0      0      1
193     0      0      0      1      0      0      0      0      1
202     0      0      0      1      0      0      0      0      1
203     0      0      0      1      0      0      0      0      1
END PWAT-PARM1

```

PWAT-PARM2

<PLS > \*\*\* PWATER input info: Part 2

#	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
11		0.000	380.0	0.60	83.0	0.117	0.0	0.99
12		0.000	380.0	0.60	83.0	0.117	0.0	0.99
13		0.000	380.0	0.60	83.0	0.117	0.0	0.99
14		0.000	380.0	0.60	83.0	0.117	0.0	0.99
15		0.000	380.0	0.60	83.0	0.117	0.0	0.99
16		0.000	380.0	0.60	83.0	0.117	0.0	0.99
17		0.000	380.0	0.60	83.0	0.117	0.0	0.99
18		0.000	380.0	0.60	83.0	0.117	0.0	0.99
21		0.000	380.0	0.60	63.0	0.147	0.0	0.99
22		0.000	380.0	0.60	63.0	0.147	0.0	0.99
23		0.000	380.0	0.60	63.0	0.147	0.0	0.99
24		0.000	380.0	0.60	63.0	0.147	0.0	0.99
25		0.000	380.0	0.60	63.0	0.147	0.0	0.99
26		0.000	380.0	0.60	63.0	0.147	0.0	0.99
27		0.000	380.0	0.60	63.0	0.147	0.0	0.99
28		0.000	380.0	0.60	63.0	0.147	0.0	0.99
31		0.000	380.0	0.60	99.0	0.092	0.0	0.99
32		0.000	380.0	0.60	99.0	0.092	0.0	0.99
33		0.000	380.0	0.60	99.0	0.092	0.0	0.99
34		0.000	380.0	0.60	99.0	0.092	0.0	0.99
35		0.000	380.0	0.60	99.0	0.092	0.0	0.99
36		0.000	380.0	0.60	99.0	0.092	0.0	0.99
37		0.000	380.0	0.60	99.0	0.092	0.0	0.99
38		0.000	380.0	0.60	99.0	0.092	0.0	0.99
41		0.000	380.0	0.60	64.0	0.146	0.0	0.99
42		0.000	380.0	0.60	64.0	0.146	0.0	0.99
43		0.000	380.0	0.60	64.0	0.146	0.0	0.99
44		0.000	380.0	0.60	64.0	0.146	0.0	0.99
45		0.000	380.0	0.60	64.0	0.146	0.0	0.99
46		0.000	380.0	0.60	64.0	0.146	0.0	0.99
47		0.000	380.0	0.60	64.0	0.146	0.0	0.99
48		0.000	380.0	0.60	64.0	0.146	0.0	0.99
51		0.000	380.0	0.60	54.0	0.187	0.0	0.99
52		0.000	380.0	0.60	54.0	0.187	0.0	0.99
53		0.000	380.0	0.60	54.0	0.187	0.0	0.99
54		0.000	380.0	0.60	54.0	0.187	0.0	0.99
55		0.000	380.0	0.60	54.0	0.187	0.0	0.99

56	0.000	380.0	0.60	54.0	0.187	0.0	0.99
57	0.000	380.0	0.60	54.0	0.187	0.0	0.99
58	0.000	380.0	0.60	54.0	0.187	0.0	0.99
61	0.000	280.0	1.00	61.0	0.149	0.0	0.95
62	0.000	280.0	1.00	61.0	0.149	0.0	0.95
63	0.000	280.0	1.00	61.0	0.149	0.0	0.95
64	0.000	280.0	1.00	61.0	0.149	0.0	0.95
65	0.000	280.0	1.00	61.0	0.149	0.0	0.95
66	0.000	280.0	1.00	61.0	0.149	0.0	0.95
67	0.000	280.0	1.00	61.0	0.149	0.0	0.95
68	0.000	280.0	1.00	61.0	0.149	0.0	0.95
71	0.000	280.0	1.00	48.0	0.217	0.0	0.95
72	0.000	280.0	1.00	48.0	0.217	0.0	0.95
73	0.000	280.0	1.00	48.0	0.217	0.0	0.95
74	0.000	280.0	1.00	48.0	0.217	0.0	0.95
75	0.000	280.0	1.00	48.0	0.217	0.0	0.95
76	0.000	280.0	1.00	48.0	0.217	0.0	0.95
77	0.000	280.0	1.00	48.0	0.217	0.0	0.95
78	0.000	280.0	1.00	48.0	0.217	0.0	0.95
81	0.000	280.0	1.00	31.0	0.300	0.0	0.95
82	0.000	280.0	1.00	31.0	0.300	0.0	0.95
83	0.000	280.0	1.00	31.0	0.300	0.0	0.95
84	0.000	280.0	1.00	31.0	0.300	0.0	0.95
85	0.000	280.0	1.00	31.0	0.300	0.0	0.95
86	0.000	280.0	1.00	31.0	0.300	0.0	0.95
87	0.000	280.0	1.00	31.0	0.300	0.0	0.95
88	0.000	280.0	1.00	31.0	0.300	0.0	0.95
91	0.000	280.0	1.00	58.0	0.165	0.0	0.95
92	0.000	280.0	1.00	58.0	0.165	0.0	0.95
93	0.000	280.0	1.00	58.0	0.165	0.0	0.95
94	0.000	280.0	1.00	58.0	0.165	0.0	0.95
95	0.000	280.0	1.00	58.0	0.165	0.0	0.95
96	0.000	280.0	1.00	58.0	0.165	0.0	0.95
97	0.000	280.0	1.00	58.0	0.165	0.0	0.95
98	0.000	280.0	1.00	58.0	0.165	0.0	0.95
101	0.000	280.0	1.00	34.0	0.284	0.0	0.95
102	0.000	280.0	1.00	34.0	0.284	0.0	0.95
103	0.000	280.0	1.00	34.0	0.284	0.0	0.95
104	0.000	280.0	1.00	34.0	0.284	0.0	0.95
105	0.000	280.0	1.00	34.0	0.284	0.0	0.95
106	0.000	280.0	1.00	34.0	0.284	0.0	0.95
107	0.000	280.0	1.00	34.0	0.284	0.0	0.95
108	0.000	280.0	1.00	34.0	0.284	0.0	0.95
111	0.000	280.0	1.00	31.0	0.300	0.0	0.95
112	0.000	280.0	1.00	31.0	0.300	0.0	0.95
113	0.000	280.0	1.00	31.0	0.300	0.0	0.95
114	0.000	280.0	1.00	31.0	0.300	0.0	0.95
115	0.000	280.0	1.00	31.0	0.300	0.0	0.95
116	0.000	280.0	1.00	31.0	0.300	0.0	0.95
117	0.000	280.0	1.00	31.0	0.300	0.0	0.95
118	0.000	280.0	1.00	31.0	0.300	0.0	0.95
121	0.000	280.0	1.00	51.0	0.202	0.0	0.95
122	0.000	280.0	1.00	51.0	0.202	0.0	0.95
123	0.000	280.0	1.00	51.0	0.202	0.0	0.95
124	0.000	280.0	1.00	51.0	0.202	0.0	0.95
125	0.000	280.0	1.00	51.0	0.202	0.0	0.95
126	0.000	280.0	1.00	51.0	0.202	0.0	0.95
127	0.000	280.0	1.00	51.0	0.202	0.0	0.95
128	0.000	280.0	1.00	51.0	0.202	0.0	0.95
131	0.000	280.0	1.00	55.0	0.181	0.0	0.95
132	0.000	280.0	1.00	55.0	0.181	0.0	0.95
133	0.000	280.0	1.00	55.0	0.181	0.0	0.95
134	0.000	280.0	1.00	55.0	0.181	0.0	0.95
135	0.000	280.0	1.00	55.0	0.181	0.0	0.95
136	0.000	280.0	1.00	55.0	0.181	0.0	0.95
137	0.000	280.0	1.00	55.0	0.181	0.0	0.95
138	0.000	280.0	1.00	55.0	0.181	0.0	0.95
141	0.000	280.0	1.00	84.0	0.115	0.0	0.95

142	0.000	280.0	1.00	84.0	0.115	0.0	0.95
143	0.000	280.0	1.00	84.0	0.115	0.0	0.95
144	0.000	280.0	1.00	84.0	0.115	0.0	0.95
145	0.000	280.0	1.00	84.0	0.115	0.0	0.95
146	0.000	280.0	1.00	84.0	0.115	0.0	0.95
147	0.000	280.0	1.00	84.0	0.115	0.0	0.95
148	0.000	280.0	1.00	84.0	0.115	0.0	0.95
151	0.000	300.0	1.00	72.0	0.133	0.0	0.95
152	0.000	300.0	1.00	72.0	0.133	0.0	0.95
153	0.000	300.0	1.00	72.0	0.133	0.0	0.95
154	0.000	300.0	1.00	72.0	0.133	0.0	0.95
155	0.000	300.0	1.00	72.0	0.133	0.0	0.95
156	0.000	300.0	1.00	72.0	0.133	0.0	0.95
157	0.000	300.0	1.00	72.0	0.133	0.0	0.95
158	0.000	300.0	1.00	72.0	0.133	0.0	0.95
161	0.000	300.0	1.00	77.0	0.126	0.0	0.95
162	0.000	300.0	1.00	77.0	0.126	0.0	0.95
163	0.000	300.0	1.00	77.0	0.126	0.0	0.95
164	0.000	300.0	1.00	77.0	0.126	0.0	0.95
165	0.000	300.0	1.00	77.0	0.126	0.0	0.95
166	0.000	300.0	1.00	77.0	0.126	0.0	0.95
167	0.000	300.0	1.00	77.0	0.126	0.0	0.95
168	0.000	300.0	1.00	77.0	0.126	0.0	0.95
171	0.000	300.0	1.00	126.0	0.050	0.0	0.95
172	0.000	300.0	1.00	126.0	0.050	0.0	0.95
173	0.000	300.0	1.00	126.0	0.050	0.0	0.95
174	0.000	300.0	1.00	126.0	0.050	0.0	0.95
175	0.000	300.0	1.00	126.0	0.050	0.0	0.95
176	0.000	300.0	1.00	126.0	0.050	0.0	0.95
177	0.000	300.0	1.00	126.0	0.050	0.0	0.95
178	0.000	300.0	1.00	126.0	0.050	0.0	0.95
181	0.000	300.0	1.00	101.0	0.089	0.0	0.95
182	0.000	300.0	1.00	101.0	0.089	0.0	0.95
183	0.000	300.0	1.00	101.0	0.089	0.0	0.95
184	0.000	300.0	1.00	101.0	0.089	0.0	0.95
185	0.000	300.0	1.00	101.0	0.089	0.0	0.95
186	0.000	300.0	1.00	101.0	0.089	0.0	0.95
187	0.000	300.0	1.00	101.0	0.089	0.0	0.95
188	0.000	300.0	1.00	101.0	0.089	0.0	0.95
191	0.000	300.0	1.00	133.0	0.039	0.0	0.95
192	0.000	300.0	1.00	133.0	0.039	0.0	0.95
193	0.000	300.0	1.00	133.0	0.039	0.0	0.95
194	0.000	300.0	1.00	133.0	0.039	0.0	0.95
195	0.000	300.0	1.00	133.0	0.039	0.0	0.95
196	0.000	300.0	1.00	133.0	0.039	0.0	0.95
197	0.000	300.0	1.00	133.0	0.039	0.0	0.95
198	0.000	300.0	1.00	133.0	0.039	0.0	0.95
201	0.000	300.0	1.00	136.0	0.035	0.0	0.95
202	0.000	300.0	1.00	136.0	0.035	0.0	0.95
203	0.000	300.0	1.00	136.0	0.035	0.0	0.95
204	0.000	300.0	1.00	136.0	0.035	0.0	0.95
205	0.000	300.0	1.00	136.0	0.035	0.0	0.95
206	0.000	300.0	1.00	136.0	0.035	0.0	0.95
207	0.000	300.0	1.00	136.0	0.035	0.0	0.95
208	0.000	300.0	1.00	136.0	0.035	0.0	0.95

END PWAT-PARM2

PWAT-PARM3

<PLS > \*\*\* PWATER input info: Part 3

#	-	#	***PETMAX	PETMIN	INFEXP	INFILD	DEEPPFR	BASETP	AGWETP
11					2.0	2.0	0.00	0.0	0.00
12					2.0	2.0	0.00	0.0	0.00
13					2.0	2.0	0.00	0.0	0.00
14					2.0	2.0	0.00	0.0	0.00
15					2.0	2.0	0.00	0.0	0.00
16					2.0	2.0	0.00	0.0	0.00
17					2.0	2.0	0.00	0.0	0.00
18					2.0	2.0	0.00	0.0	0.00



21	2.0	2.0	0.00	0.0	0.00
22	2.0	2.0	0.00	0.0	0.00
23	2.0	2.0	0.00	0.0	0.00
24	2.0	2.0	0.00	0.0	0.00
25	2.0	2.0	0.00	0.0	0.00
26	2.0	2.0	0.00	0.0	0.00
27	2.0	2.0	0.00	0.0	0.00
28	2.0	2.0	0.00	0.0	0.00
31	2.0	2.0	0.00	0.0	0.00
32	2.0	2.0	0.00	0.0	0.00
33	2.0	2.0	0.00	0.0	0.00
34	2.0	2.0	0.00	0.0	0.00
35	2.0	2.0	0.00	0.0	0.00
36	2.0	2.0	0.00	0.0	0.00
37	2.0	2.0	0.00	0.0	0.00
38	2.0	2.0	0.00	0.0	0.00
41	2.0	2.0	0.00	0.0	0.00
42	2.0	2.0	0.00	0.0	0.00
43	2.0	2.0	0.00	0.0	0.00
44	2.0	2.0	0.00	0.0	0.00
45	2.0	2.0	0.00	0.0	0.00
46	2.0	2.0	0.00	0.0	0.00
47	2.0	2.0	0.00	0.0	0.00
48	2.0	2.0	0.00	0.0	0.00
51	2.0	2.0	0.00	0.0	0.00
52	2.0	2.0	0.00	0.0	0.00
53	2.0	2.0	0.00	0.0	0.00
54	2.0	2.0	0.00	0.0	0.00
55	2.0	2.0	0.00	0.0	0.00
56	2.0	2.0	0.00	0.0	0.00
57	2.0	2.0	0.00	0.0	0.00
58	2.0	2.0	0.00	0.0	0.00
61	2.0	2.0	0.00	0.0	0.00
62	2.0	2.0	0.00	0.0	0.00
63	2.0	2.0	0.00	0.0	0.00
64	2.0	2.0	0.00	0.0	0.00
65	2.0	2.0	0.00	0.0	0.00
66	2.0	2.0	0.00	0.0	0.00
67	2.0	2.0	0.00	0.0	0.00
68	2.0	2.0	0.00	0.0	0.00
71	2.0	2.0	0.00	0.0	0.00
72	2.0	2.0	0.00	0.0	0.00
73	2.0	2.0	0.00	0.0	0.00
74	2.0	2.0	0.00	0.0	0.00
75	2.0	2.0	0.00	0.0	0.00
76	2.0	2.0	0.00	0.0	0.00
77	2.0	2.0	0.00	0.0	0.00
78	2.0	2.0	0.00	0.0	0.00
81	2.0	2.0	0.00	0.0	0.00
82	2.0	2.0	0.00	0.0	0.00
83	2.0	2.0	0.00	0.0	0.00
84	2.0	2.0	0.00	0.0	0.00
85	2.0	2.0	0.00	0.0	0.00
86	2.0	2.0	0.00	0.0	0.00
87	2.0	2.0	0.00	0.0	0.00
88	2.0	2.0	0.00	0.0	0.00
91	2.0	2.0	0.00	0.0	0.00
92	2.0	2.0	0.00	0.0	0.00
93	2.0	2.0	0.00	0.0	0.00
94	2.0	2.0	0.00	0.0	0.00
95	2.0	2.0	0.00	0.0	0.00
96	2.0	2.0	0.00	0.0	0.00
97	2.0	2.0	0.00	0.0	0.00
98	2.0	2.0	0.00	0.0	0.00
101	2.0	2.0	0.00	0.0	0.00
102	2.0	2.0	0.00	0.0	0.00
103	2.0	2.0	0.00	0.0	0.00
104	2.0	2.0	0.00	0.0	0.00

105	2.0	2.0	0.00	0.0	0.00
106	2.0	2.0	0.00	0.0	0.00
107	2.0	2.0	0.00	0.0	0.00
108	2.0	2.0	0.00	0.0	0.00
111	2.0	2.0	0.00	0.0	0.00
112	2.0	2.0	0.00	0.0	0.00
113	2.0	2.0	0.00	0.0	0.00
114	2.0	2.0	0.00	0.0	0.00
115	2.0	2.0	0.00	0.0	0.00
116	2.0	2.0	0.00	0.0	0.00
117	2.0	2.0	0.00	0.0	0.00
118	2.0	2.0	0.00	0.0	0.00
121	2.0	2.0	0.00	0.0	0.00
122	2.0	2.0	0.00	0.0	0.00
123	2.0	2.0	0.00	0.0	0.00
124	2.0	2.0	0.00	0.0	0.00
125	2.0	2.0	0.00	0.0	0.00
126	2.0	2.0	0.00	0.0	0.00
127	2.0	2.0	0.00	0.0	0.00
128	2.0	2.0	0.00	0.0	0.00
131	2.0	2.0	0.00	0.0	0.00
132	2.0	2.0	0.00	0.0	0.00
133	2.0	2.0	0.00	0.0	0.00
134	2.0	2.0	0.00	0.0	0.00
135	2.0	2.0	0.00	0.0	0.00
136	2.0	2.0	0.00	0.0	0.00
137	2.0	2.0	0.00	0.0	0.00
138	2.0	2.0	0.00	0.0	0.00
141	2.0	2.0	0.00	0.0	0.00
142	2.0	2.0	0.00	0.0	0.00
143	2.0	2.0	0.00	0.0	0.00
144	2.0	2.0	0.00	0.0	0.00
145	2.0	2.0	0.00	0.0	0.00
146	2.0	2.0	0.00	0.0	0.00
147	2.0	2.0	0.00	0.0	0.00
148	2.0	2.0	0.00	0.0	0.00
151	2.0	2.0	0.00	0.0	0.00
152	2.0	2.0	0.00	0.0	0.00
153	2.0	2.0	0.00	0.0	0.00
154	2.0	2.0	0.00	0.0	0.00
155	2.0	2.0	0.00	0.0	0.00
156	2.0	2.0	0.00	0.0	0.00
157	2.0	2.0	0.00	0.0	0.00
158	2.0	2.0	0.00	0.0	0.00
161	2.0	2.0	0.00	0.0	0.00
162	2.0	2.0	0.00	0.0	0.00
163	2.0	2.0	0.00	0.0	0.00
164	2.0	2.0	0.00	0.0	0.00
165	2.0	2.0	0.00	0.0	0.00
166	2.0	2.0	0.00	0.0	0.00
167	2.0	2.0	0.00	0.0	0.00
168	2.0	2.0	0.00	0.0	0.00
171	2.0	2.0	0.00	0.0	0.00
172	2.0	2.0	0.00	0.0	0.00
173	2.0	2.0	0.00	0.0	0.00
174	2.0	2.0	0.00	0.0	0.00
175	2.0	2.0	0.00	0.0	0.00
176	2.0	2.0	0.00	0.0	0.00
177	2.0	2.0	0.00	0.0	0.00
178	2.0	2.0	0.00	0.0	0.00
181	2.0	2.0	0.00	0.0	0.00
182	2.0	2.0	0.00	0.0	0.00
183	2.0	2.0	0.00	0.0	0.00
184	2.0	2.0	0.00	0.0	0.00
185	2.0	2.0	0.00	0.0	0.00
186	2.0	2.0	0.00	0.0	0.00
187	2.0	2.0	0.00	0.0	0.00
188	2.0	2.0	0.00	0.0	0.00

191	2.0	2.0	0.00	0.0	0.00
192	2.0	2.0	0.00	0.0	0.00
193	2.0	2.0	0.00	0.0	0.00
194	2.0	2.0	0.00	0.0	0.00
195	2.0	2.0	0.00	0.0	0.00
196	2.0	2.0	0.00	0.0	0.00
197	2.0	2.0	0.00	0.0	0.00
198	2.0	2.0	0.00	0.0	0.00
201	2.0	2.0	0.00	0.0	0.00
202	2.0	2.0	0.00	0.0	0.00
203	2.0	2.0	0.00	0.0	0.00
204	2.0	2.0	0.00	0.0	0.00
205	2.0	2.0	0.00	0.0	0.00
206	2.0	2.0	0.00	0.0	0.00
207	2.0	2.0	0.00	0.0	0.00
208	2.0	2.0	0.00	0.0	0.00

END PWAT-PARM3

PWAT-PARM4

<PLS >		PWATER input info: Part 4					***
# - #	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP	***
11	4.45	19.40	0.40	2.5	0.70	0.80	
21	4.45	19.40	0.40	2.5	0.70	0.80	
31	4.45	19.40	0.40	2.5	0.70	0.80	
41	4.45	19.40	0.40	2.5	0.70	0.80	
51	4.45	19.40	0.40	2.5	0.70	0.80	
61	4.45	29.40	0.40	2.5	0.70	0.80	
71	4.45	29.40	0.40	2.5	0.70	0.80	
81	4.45	29.40	0.40	2.5	0.70	0.80	
91	4.45	29.40	0.40	2.5	0.70	0.80	
101	4.45	29.40	0.40	2.5	0.70	0.80	
111	4.45	29.40	0.40	2.5	0.70	0.80	
121	4.45	29.40	0.40	2.5	0.70	0.80	
131	4.45	29.40	0.40	2.5	0.70	0.80	
141	4.45	29.40	0.40	2.5	0.70	0.80	
151	4.45	29.40	0.40	2.5	0.70	0.80	
161	4.45	29.40	0.40	2.5	0.70	0.80	
171	4.45	29.40	0.40	2.5	0.70	0.80	
181	4.45	29.40	0.40	2.5	0.70	0.80	
191	4.45	29.40	0.40	2.5	0.70	0.80	
201	4.45	29.40	0.40	2.5	0.70	0.80	
12	3.50	16.80	0.25	3.0	0.70	0.60	
22	3.50	16.80	0.25	3.0	0.70	0.60	
32	3.50	16.80	0.25	3.0	0.70	0.60	
42	3.50	16.80	0.25	3.0	0.70	0.60	
52	3.50	16.80	0.25	3.0	0.70	0.60	
62	3.50	16.80	0.25	3.0	0.70	0.60	
72	3.50	16.80	0.25	3.0	0.70	0.60	
82	3.50	16.80	0.25	3.0	0.70	0.60	
92	3.50	16.80	0.25	3.0	0.70	0.60	
102	3.50	16.80	0.25	3.0	0.70	0.60	
112	3.50	16.80	0.25	3.0	0.70	0.60	
122	3.50	16.80	0.25	3.0	0.70	0.60	
132	3.50	16.80	0.25	3.0	0.70	0.60	
142	3.50	16.80	0.25	3.0	0.70	0.60	
152	3.50	16.80	0.25	3.0	0.70	0.60	
162	3.50	16.80	0.25	3.0	0.70	0.60	
172	3.50	16.80	0.25	3.0	0.70	0.60	
182	3.50	16.80	0.25	3.0	0.70	0.60	
192	3.50	16.80	0.25	3.0	0.70	0.60	
202	3.50	16.80	0.25	3.0	0.70	0.60	
13	2.54	5.80	0.20	3.0	0.70	0.50	
23	2.54	5.80	0.20	3.0	0.70	0.50	
33	2.54	5.80	0.20	3.0	0.70	0.50	
43	2.54	5.80	0.20	3.0	0.70	0.50	
53	2.54	5.80	0.20	3.0	0.70	0.50	
63	2.54	16.80	0.20	3.0	0.70	0.50	
73	2.54	16.80	0.20	3.0	0.70	0.50	



83	2.54	16.80	0.20	3.0	0.70	0.50
93	2.54	16.80	0.20	3.0	0.70	0.50
103	2.54	16.80	0.20	3.0	0.70	0.50
113	2.54	16.80	0.20	3.0	0.70	0.50
123	2.54	16.80	0.20	3.0	0.70	0.50
133	2.54	16.80	0.20	3.0	0.70	0.50
143	2.54	16.80	0.20	3.0	0.70	0.50
153	2.54	16.80	0.20	3.0	0.70	0.50
163	2.54	16.80	0.20	3.0	0.70	0.50
173	2.54	16.80	0.20	3.0	0.70	0.50
183	2.54	16.80	0.20	3.0	0.70	0.50
193	2.54	16.80	0.20	3.0	0.70	0.50
203	2.54	16.80	0.20	3.0	0.70	0.50
14	0.25	2.54	0.10	1.5	0.70	0.10
24	0.25	2.54	0.10	1.5	0.70	0.10
34	0.25	2.54	0.10	1.5	0.70	0.10
44	0.25	2.54	0.10	1.5	0.70	0.10
54	0.25	2.54	0.10	1.5	0.70	0.10
64	0.25	2.54	0.10	1.5	0.70	0.10
74	0.25	2.54	0.10	1.5	0.70	0.10
84	0.25	2.54	0.10	1.5	0.70	0.10
94	0.25	2.54	0.10	1.5	0.70	0.10
104	0.25	2.54	0.10	1.5	0.70	0.10
114	0.25	2.54	0.10	1.5	0.70	0.10
124	0.25	2.54	0.10	1.5	0.70	0.10
134	0.25	2.54	0.10	1.5	0.70	0.10
144	0.25	2.54	0.10	1.5	0.70	0.10
154	0.25	2.54	0.10	1.5	0.70	0.10
164	0.25	2.54	0.10	1.5	0.70	0.10
174	0.25	2.54	0.10	1.5	0.70	0.10
184	0.25	2.54	0.10	1.5	0.70	0.10
194	0.25	2.54	0.10	1.5	0.70	0.10
204	0.25	2.54	0.10	1.5	0.70	0.10
15	1.05	6.50	0.14	1.5	0.70	0.20
25	1.05	6.50	0.14	1.5	0.70	0.20
35	1.05	6.50	0.14	1.5	0.70	0.20
45	1.05	6.50	0.14	1.5	0.70	0.20
55	1.05	6.50	0.14	1.5	0.70	0.20
65	1.05	6.50	0.14	1.5	0.70	0.20
75	1.05	6.50	0.14	1.5	0.70	0.20
85	1.05	6.50	0.14	1.5	0.70	0.20
95	1.05	6.50	0.14	1.5	0.70	0.20
105	1.05	6.50	0.14	1.5	0.70	0.20
115	1.05	6.50	0.14	1.5	0.70	0.20
125	1.05	6.50	0.14	1.5	0.70	0.20
135	1.05	6.50	0.14	1.5	0.70	0.20
145	1.05	6.50	0.14	1.5	0.70	0.20
155	1.05	6.50	0.14	1.5	0.70	0.20
165	1.05	6.50	0.14	1.5	0.70	0.20
175	1.05	6.50	0.14	1.5	0.70	0.20
185	1.05	6.50	0.14	1.5	0.70	0.20
195	1.05	6.50	0.14	1.5	0.70	0.20
205	1.05	6.50	0.14	1.5	0.70	0.20
16	1.85	10.00	0.15	1.8	0.70	0.35
26	1.85	10.00	0.15	1.8	0.70	0.35
36	1.85	10.00	0.15	1.8	0.70	0.35
46	1.85	10.00	0.15	1.8	0.70	0.35
56	1.85	10.00	0.15	1.8	0.70	0.35
66	1.85	10.00	0.15	1.8	0.70	0.40
76	1.85	10.00	0.15	1.8	0.70	0.40
86	1.85	10.00	0.15	1.8	0.70	0.40
96	1.85	10.00	0.15	1.8	0.70	0.40
106	1.85	10.00	0.15	1.8	0.70	0.40
116	1.85	10.00	0.15	1.8	0.70	0.40
126	1.85	10.00	0.15	1.8	0.70	0.40
136	1.85	10.00	0.15	1.8	0.70	0.40
146	1.85	10.00	0.15	1.8	0.70	0.40
156	1.85	10.00	0.15	1.8	0.70	0.40

166	1.85	10.00	0.15	1.8	0.70	0.40
176	1.85	10.00	0.15	1.8	0.70	0.40
186	1.85	10.00	0.15	1.8	0.70	0.40
196	1.85	10.00	0.15	1.8	0.70	0.40
206	1.85	10.00	0.15	1.8	0.70	0.40
17	0.76	3.50	0.12	1.0	0.70	0.15
27	0.76	3.50	0.12	1.0	0.70	0.15
37	0.76	3.50	0.12	1.0	0.70	0.15
47	0.76	3.50	0.12	1.0	0.70	0.15
57	0.76	3.50	0.12	1.0	0.70	0.15
67	0.76	3.50	0.12	1.0	0.70	0.15
77	0.76	3.50	0.12	1.0	0.70	0.15
87	0.76	3.50	0.12	1.0	0.70	0.15
97	0.76	3.50	0.12	1.0	0.70	0.15
107	0.76	3.50	0.12	1.0	0.70	0.15
117	0.76	3.50	0.12	1.0	0.70	0.15
127	0.76	3.50	0.12	1.0	0.70	0.15
137	0.76	3.50	0.12	1.0	0.70	0.15
147	0.76	3.50	0.12	1.0	0.70	0.15
157	0.76	3.50	0.12	1.0	0.70	0.15
167	0.76	3.50	0.12	1.0	0.70	0.15
177	0.76	3.50	0.12	1.0	0.70	0.15
187	0.76	3.50	0.12	1.0	0.70	0.15
197	0.76	3.50	0.12	1.0	0.70	0.15
207	0.76	3.50	0.12	1.0	0.70	0.15
18	2.00	10.00	0.30	2.0	0.70	0.75
28	2.00	10.00	0.30	2.0	0.70	0.75
38	2.00	10.00	0.30	2.0	0.70	0.75
48	2.00	10.00	0.30	2.0	0.70	0.75
58	2.00	10.00	0.30	2.0	0.70	0.75
68	2.00	10.00	0.30	2.0	0.70	0.75
78	2.00	10.00	0.30	2.0	0.70	0.75
88	2.00	10.00	0.30	2.0	0.70	0.75
98	2.00	10.00	0.30	2.0	0.70	0.75
108	2.00	10.00	0.30	2.0	0.70	0.75
118	2.00	10.00	0.30	2.0	0.70	0.75
128	2.00	10.00	0.30	2.0	0.70	0.75
138	2.00	10.00	0.30	2.0	0.70	0.75
148	2.00	10.00	0.30	2.0	0.70	0.75
158	2.00	10.00	0.30	2.0	0.70	0.75
168	2.00	10.00	0.30	2.0	0.70	0.75
178	2.00	10.00	0.30	2.0	0.70	0.75
188	2.00	10.00	0.30	2.0	0.70	0.75
198	2.00	10.00	0.30	2.0	0.70	0.75
208	2.00	10.00	0.30	2.0	0.70	0.75

END PWAT-PARM4

PWAT-PARM5 Defaults used \*\*\*

MON-INTERCEP

<PLS> Only required if VCSFG=1 in PWAT-PARM1 \*\*\*

# - # Interception storage capacity at start of each month \*\*\*

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
12	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
13	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
22	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
23	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
32	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
33	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
42	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
43	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
52	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
53	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
62	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
63	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
72	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
73	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
82	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50



```

83      4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
92      4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
93      4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
102     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
103     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
112     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
113     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
122     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
123     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
132     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
133     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
142     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
143     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
152     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
153     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
162     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
163     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
172     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
173     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
182     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
183     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
192     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
193     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
202     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
203     4.25 4.45 3.50 1.25 1.00 0.75 0.75 0.75 1.00 1.25 1.50 2.50
END MON-INTERCEP

```

MON-LZETPARM

```

<PLS > Only required if VLEFG=1 in PWAT-PARM1
# - # Lower zone ET parameter at start of each month
      JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC
12    0.65 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
13    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
22    0.65 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
23    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
32    0.65 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
33    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
42    0.65 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
43    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
52    0.65 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
53    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
62    0.60 0.70 0.60 0.30 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
63    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
72    0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
73    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
82    0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
83    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
92    0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
93    0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
102   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
103   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
112   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
113   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
122   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
123   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
132   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
133   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
142   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
143   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
152   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
153   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
162   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
163   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
172   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
173   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
182   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
183   0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
192   0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40

```



```

193      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
202      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
203      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
END MON-LZETPARM

```

PWAT-STATE1

<PLS > \*\*\* Initial conditions at start of simulation

```

# - # *** CEPS      SURS      UZS      IFWS      LZS      AGWS      GWVS
11 208      0.00      0.0      11.94      0.0      300.0      25.4      0.0
END PWAT-STATE1

```

END PERLND

IMPLND

ACTIVITY

<ILS > Active Sections \*\*\*

```

# - # ATMP SNOW IWAT SLD IWG IQAL ***
14 207      1      1
END ACTIVITY

```

PRINT-INFO

<ILS > Print-flags \*\*\*

```

# - # ATMP SNOW IWAT SLD IWG IQAL PIVL PYR ***
14 207      4      09
END PRINT-INFO

```

GEN-INFO

```

<ILS ><-----Name-----> Unit-systems Printer ***
# - # t-series Engl Metr ***
      in out
14    HD Urban, Indust.      2    2    0    64
15    MD Residential         2    2    0    64
17    HD Residential         2    2    0    64
24    HD Urban, Indust.      2    2    0    64
25    MD Residential         2    2    0    64
27    HD Residential         2    2    0    64
34    HD Urban, Indust.      2    2    0    64
35    MD Residential         2    2    0    64
37    HD Residential         2    2    0    64
44    HD Urban, Indust.      2    2    0    64
45    MD Residential         2    2    0    64
47    HD Residential         2    2    0    64
54    HD Urban, Indust.      2    2    0    64
55    MD Residential         2    2    0    64
57    HD Residential         2    2    0    64
64    HD Urban, Indust.      2    2    0    64
65    MD Residential         2    2    0    64
67    HD Residential         2    2    0    64
74    HD Urban, Indust.      2    2    0    64
75    MD Residential         2    2    0    64
77    HD Residential         2    2    0    64
84    HD Urban, Indust.      2    2    0    64
85    MD Residential         2    2    0    64
87    HD Residential         2    2    0    64
94    HD Urban, Indust.      2    2    0    64
95    MD Residential         2    2    0    64
97    HD Residential         2    2    0    64
104   HD Urban, Indust.      2    2    0    64
105   MD Residential         2    2    0    64
107   HD Residential         2    2    0    64
114   HD Urban, Indust.      2    2    0    64
115   MD Residential         2    2    0    64
117   HD Residential         2    2    0    64
124   HD Urban, Indust.      2    2    0    64
125   MD Residential         2    2    0    64
127   HD Residential         2    2    0    64
134   HD Urban, Indust.      2    2    0    64
135   MD Residential         2    2    0    64

```

```

193      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
202      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
203      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
END MON-LZETPARM

```

PWAT-STATE1

<PLS > \*\*\* Initial conditions at start of simulation

```

# - # *** CEPS      SURS      UZS      IFWS      LZS      AGWS      GWVS
11 208      0.00      0.0      11.94      0.0      300.0      25.4      0.0
END PWAT-STATE1

```

END PERLND

IMPLND

ACTIVITY

<ILS > Active Sections \*\*\*

```

# - # ATMP SNOW IWAT SLD IWG IQAL ***
14 207      1      1
END ACTIVITY

```

PRINT-INFO

<ILS > Print-flags \*\*\*

```

# - # ATMP SNOW IWAT SLD IWG IQAL PIVL PYR ***
14 207      4      09
END PRINT-INFO

```

GEN-INFO

<ILS ><-----Name----->

#	#	Unit-systems	Printer		Engl	Metr
			t-series			
			in	out		
14	HD Urban, Indust.	2	2	0	64	
15	MD Residential	2	2	0	64	
17	HD Residential	2	2	0	64	
24	HD Urban, Indust.	2	2	0	64	
25	MD Residential	2	2	0	64	
27	HD Residential	2	2	0	64	
34	HD Urban, Indust.	2	2	0	64	
35	MD Residential	2	2	0	64	
37	HD Residential	2	2	0	64	
44	HD Urban, Indust.	2	2	0	64	
45	MD Residential	2	2	0	64	
47	HD Residential	2	2	0	64	
54	HD Urban, Indust.	2	2	0	64	
55	MD Residential	2	2	0	64	
57	HD Residential	2	2	0	64	
64	HD Urban, Indust.	2	2	0	64	
65	MD Residential	2	2	0	64	
67	HD Residential	2	2	0	64	
74	HD Urban, Indust.	2	2	0	64	
75	MD Residential	2	2	0	64	
77	HD Residential	2	2	0	64	
84	HD Urban, Indust.	2	2	0	64	
85	MD Residential	2	2	0	64	
87	HD Residential	2	2	0	64	
94	HD Urban, Indust.	2	2	0	64	
95	MD Residential	2	2	0	64	
97	HD Residential	2	2	0	64	
104	HD Urban, Indust.	2	2	0	64	
105	MD Residential	2	2	0	64	
107	HD Residential	2	2	0	64	
114	HD Urban, Indust.	2	2	0	64	
115	MD Residential	2	2	0	64	
117	HD Residential	2	2	0	64	
124	HD Urban, Indust.	2	2	0	64	
125	MD Residential	2	2	0	64	
127	HD Residential	2	2	0	64	
134	HD Urban, Indust.	2	2	0	64	
135	MD Residential	2	2	0	64	

137	HD Residential	2	2	0	64
144	HD Urban, Indust.	2	2	0	64
145	MD Residential	2	2	0	64
147	HD Residential	2	2	0	64
154	HD Urban, Indust.	2	2	0	64
155	MD Residential	2	2	0	64
157	HD Residential	2	2	0	64
164	HD Urban, Indust.	2	2	0	64
165	MD Residential	2	2	0	64
177	HD Residential	2	2	0	64
184	HD Urban, Indust.	2	2	0	64
185	MD Residential	2	2	0	64
187	HD Residential	2	2	0	64
194	HD Urban, Indust.	2	2	0	64
195	MD Residential	2	2	0	64
197	HD Residential	2	2	0	64
204	HD Urban, Indust.	2	2	0	64
205	MD Residential	2	2	0	64
207	HD Residential	2	2	0	64

END GEN-INFO  
\*\*\* Section IWATER \*\*\*

IWAT-PARM1 defaults used \*\*\*

IWAT-PARM2

<ILS >					***
# - #	LSUR	SLSUR	NSUR	RETSC	***
14	29.0	0.117	0.050	2.54	
15	29.0	0.117	0.100	2.54	
17	29.0	0.117	0.075	2.54	
24	16.0	0.147	0.050	2.54	
25	16.0	0.147	0.100	2.54	
27	16.0	0.147	0.075	2.54	
34	40.0	0.092	0.050	2.54	
35	40.0	0.092	0.100	2.54	
37	40.0	0.092	0.075	2.54	
44	17.0	0.146	0.050	2.54	
45	17.0	0.146	0.100	2.54	
47	17.0	0.146	0.075	2.54	
54	15.0	0.150	0.050	2.54	
55	15.0	0.150	0.100	2.54	
57	15.0	0.150	0.075	2.54	
64	16.0	0.149	0.050	2.54	
65	16.0	0.149	0.100	2.54	
67	16.0	0.149	0.075	2.54	
74	15.0	0.150	0.050	2.54	
75	15.0	0.150	0.100	2.54	
77	15.0	0.150	0.075	2.54	
84	15.0	0.150	0.050	2.54	
85	15.0	0.150	0.100	2.54	
87	15.0	0.150	0.075	2.54	
94	15.0	0.150	0.050	2.54	
95	15.0	0.150	0.100	2.54	
97	15.0	0.150	0.075	2.54	
104	15.0	0.150	0.050	2.54	
105	15.0	0.150	0.100	2.54	
107	15.0	0.150	0.075	2.54	
114	15.0	0.150	0.050	2.54	
115	15.0	0.150	0.100	2.54	
117	15.0	0.150	0.075	2.54	
124	15.0	0.150	0.050	2.54	
125	15.0	0.150	0.100	2.54	
127	15.0	0.150	0.075	2.54	
134	15.0	0.150	0.050	2.54	
135	15.0	0.150	0.100	2.54	
137	15.0	0.150	0.075	2.54	
144	30.0	0.115	0.050	2.54	
145	30.0	0.115	0.100	2.54	



```

147      30.0    0.115    0.075    2.54
154      22.0    0.133    0.050    2.54
155      22.0    0.133    0.100    2.54
157      22.0    0.133    0.075    2.54
164      26.0    0.126    0.050    2.54
165      26.0    0.126    0.100    2.54
167      26.0    0.126    0.075    2.54
174      59.0    0.050    0.050    2.54
175      59.0    0.050    0.100    2.54
177      59.0    0.050    0.075    2.54
184      42.0    0.089    0.050    2.54
185      42.0    0.089    0.100    2.54
187      42.0    0.089    0.075    2.54
194      63.0    0.039    0.050    2.54
195      63.0    0.039    0.100    2.54
197      63.0    0.039    0.075    2.54
204      65.0    0.035    0.050    2.54
205      65.0    0.035    0.100    2.54
207      65.0    0.035    0.075    2.54

```

END IWAT-PARM2

IWAT-PARM3 \*\*\* defaults used

IWAT-STATE1

<ILS > IWATER state variables \*\*\*

# - # RETS SURS \*\*\*

14 207 0.00 0.00

END IWAT-STATE1

END IMPLND

RCHRES

ACTIVITY

RCHRES Active Sections (1=Active; 0=Inactive) \*\*\*

# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG \*\*\*

1 20 1

END ACTIVITY

PRINT-INFO

Print-flags

# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PH PYR \*\*\*

1 20 5 6 6 6 6 6 6 6 6 09

END PRINT-INFO

GEN-INFO

RCHRES<-----Name----->Nexit Unit Systems Printer \*\*\*

# - # User t-series Engl Metr LKFG \*\*\*

in out \*\*\*

```

1      Reach 1      1      2      2      0      62      0
2      Reach 2      1      2      2      0      62      0
3      Reach 3      1      2      2      0      62      0
4      Reach 4      1      2      2      0      62      0
5      Reach 5      1      2      2      0      62      0
6      Reach 6      1      2      2      0      62      0
7      Reach 7      1      2      2      0      62      0
8      Reach 8      1      2      2      0      62      0
9      Henley Dam   3      2      2      0      62      1
10     Reach 10     1      2      2      0      62      0
11     Reach 11     1      2      2      0      62      0
12     Reach 12     1      2      2      0      62      0
13     Reach 13     1      2      2      0      62      0
14     Reach 14     1      2      2      0      62      0
15     Reach 15     1      2      2      0      62      0
16     Reach 16     1      2      2      0      62      0
17     Reach 17     1      2      2      0      62      0
18     Reach 18     1      2      2      0      62      0
19     Reach 19     1      2      2      0      62      0

```

20 Camp's Drift weir 1 2 2 0 62 1  
END GEN-INFO

HYDR-PARM1

RCHRES Flags for HYDR section \*\*\*  
# - # VC A1 A2 A3 ODFVFG for each exit ODGTFG for each exit \*\*\* FUNCT for each exit  
FG FG FG FG possible exit possible exit \*\*\* possible exit  
1 8 0 1 0 0 4 1 2 3 4 5 \*\*\* 1 2 3 4 5  
9 0 1 0 0 4 5 1  
10 20 0 1 0 0 4  
END HYDR-PARM1

HYDR-PARM2

RCHRES \*\*\*  
# - # FTABNO LEN DELTH STCOR KS DB50 \*\*\*  
\*\*\* The values of DB50 are needed by the Colby and Toffaleti sediment  
\*\*\* transport methods. We will use the default (6.35mm)  
1 1 12.893 292.50 0.5  
2 2 13.460 195.00 0.5  
3 3 1.702 10.53 0.5  
4 4 5.355 288.12 0.5  
5 5 12.446 100.74 0.5  
6 6 0.576 0.50 0.5  
7 7 9.538 326.25 0.5  
8 8 5.237 300.00 0.5  
9 9 2.423 3.75 0.5  
10 10 9.450 172.50 0.5  
11 11 9.227 405.00 0.5  
12 12 5.692 29.06 0.5  
13 13 9.233 422.81 0.5  
14 14 4.777 29.43 0.5  
15 15 10.215 243.75 0.5  
16 16 7.869 213.75 0.5  
17 17 1.779 11.25 0.5  
18 18 1.784 0.07 0.5  
19 19 1.140 0.09 0.5  
20 20 0.918 1.08 0.5 \*\*\*  
20 20 2.900 1.72 0.5  
END HYDR-PARM2

HYDR-INIT

Reaches are assumed initially empty, except Henley Dam \*\*\*  
RCHRES VOL CAT Initial value of COLIND \*\*\* initial value of OUTDGT  
# - # Mm3 for each possible exit \*\*\* for each possible exit  
EX1 EX2 EX3 EX4 EX5 \*\*\* EX1 EX2 EX3 EX4 EX5  
1 8 0.0  
9 1.4  
10 20 0.0  
END HYDR-INIT

END RCHRES

PLTGEN

PLOTINFO  
# - # FILE NPT NMN LABL PYR PIVL \*\*\*  
1 92 1 1 1  
END PLOTINFO

GEN-LABELS

# - # <-----Title-----> \*\*\* <-----Y axis----->  
1 OBSERVED FLOW m3/s  
END GEN-LABELS

SCALING

# - # YMIN YMAX IVLIN THRESH \*\*\*  
1 0. 100. 20.  
END SCALING

```

CURV-DATA          (first curve)
# - #              <-Curve label--> Line Intg  Col Tran ***
                        type  eqv  code code ***
1              SIM FLOW                        1    1 AVER
END CURV-DATA

CURV-DATA          (second curve)
# - #              <-Curve label--> Line Intg  Col Tran ***
                        type  eqv  code code ***
1              OBS FLOW                        2    2 AVER
END CURV-DATA

```

END PLTGEN

\*\*\* FTABLES

FTABLES

```

FTABLE          1
ROWS COLS ***
20      4
DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.580      0.003      0.205      262.44
0.500      5.160      0.013      1.301      165.29
0.750      7.740      0.029      3.836      126.12
0.975      8.170      0.047      8.063      96.99
1.200      8.600      0.066      13.429     81.65
1.425      9.030      0.086      19.834     71.95
1.650      9.460      0.106      27.223     65.16
1.875      9.890      0.128      35.566     60.07
2.100     10.320      0.151      44.846     56.09
2.550     11.180      0.199      66.200     50.18
3.000     12.040      0.252      91.300     45.92
3.450     12.900      0.308     120.215     42.65
4.350     38.030      0.537     227.077     39.40
5.250     63.160      0.992     447.162     36.98
6.150     88.290      1.674     838.172     33.28
7.050    113.419      2.581    1447.822     29.72
7.950    138.549      3.715    2318.668     26.71
8.850    163.679      5.075    3489.855     24.24
9.750    188.809      6.662    4998.028     22.21
END FTABLE 1

```

```

FTABLE          2
ROWS COLS ***
20      4
DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.250      0.003      0.122      385.61
0.500      4.500      0.011      0.772      242.86
0.750      6.750      0.025      2.277      185.31
1.017      7.256      0.044      5.288      138.65
1.283      7.762      0.064      9.212      115.81
1.550      8.269      0.085     13.976     101.83
1.817      8.775      0.108     19.544      92.19
2.083      9.281      0.132     25.906      85.04
2.350      9.788      0.158     33.060      79.46
2.883     10.800      0.213     49.774      71.16
3.417     11.812      0.273     69.780      65.16
3.950     12.825      0.339     93.205      60.53
5.017     41.284      0.627    189.419      55.18
6.083     69.742      1.219    410.871      49.46
7.150     98.201      2.115    823.976      42.78
8.217    126.659      3.314   1483.643      37.23
9.283    155.118      4.817   2438.824      32.92

```



```

10.350 183.576 6.623 3734.532 29.56
11.417 212.035 8.733 5412.887 26.89
END FTABLE 2

```

```

FTABLE 3
ROWS COLS ***
20 4
DEPTH AREA VOLUME DISCH FLO-THRU ***
(m) (HA) (Mm3) (CMS) (MIN) ***
0.000 0.000 0.000 0.000 0.00
0.250 0.283 0.000 0.099 59.41
0.500 0.567 0.001 0.631 37.41
0.750 0.850 0.003 1.861 28.55
0.950 0.914 0.005 3.628 22.75
1.150 0.978 0.007 5.855 19.48
1.350 1.041 0.009 8.523 17.33
1.550 1.105 0.011 11.624 15.78
1.750 1.169 0.013 15.157 14.60
1.950 1.233 0.016 19.127 13.67
2.350 1.360 0.021 28.401 12.25
2.750 1.488 0.027 39.512 11.20
3.150 1.615 0.033 52.536 10.40
3.950 4.478 0.057 99.617 9.56
4.750 7.341 0.104 190.467 9.14
5.550 10.204 0.175 346.318 8.40
6.350 13.068 0.268 584.747 7.63
7.150 15.931 0.384 921.441 6.94
7.950 18.794 0.523 1370.844 6.35
8.750 21.657 0.684 1946.482 5.86
END FTABLE 3

```

```

FTABLE 4
ROWS COLS ***
20 4
DEPTH AREA VOLUME DISCH FLO-THRU ***
(m) (HA) (Mm3) (CMS) (MIN) ***
0.000 0.000 0.000 0.000 0.00
0.250 0.936 0.001 0.304 64.07
0.500 1.872 0.005 1.933 40.35
0.750 2.808 0.011 5.700 30.79
0.958 3.010 0.017 11.388 24.28
1.167 3.213 0.023 18.589 20.69
1.375 3.415 0.030 27.222 18.35
1.583 3.618 0.037 37.255 16.69
1.792 3.820 0.045 48.679 15.43
2.000 4.023 0.053 61.500 14.42
2.417 4.428 0.071 91.398 12.92
2.833 4.833 0.090 127.133 11.81
3.250 5.238 0.111 168.930 10.96
4.083 14.712 0.194 320.322 10.11
4.917 24.185 0.356 614.467 9.66
5.750 33.659 0.597 1121.005 8.88
6.583 43.133 0.917 1897.574 8.06
7.417 52.606 1.316 2995.624 7.32
8.250 62.080 1.794 4462.507 6.70
9.083 71.554 2.351 6342.574 6.18
END FTABLE 4

```

```

FTABLE 5
ROWS COLS ***
20 4
DEPTH AREA VOLUME DISCH FLO-THRU ***
(m) (HA) (Mm3) (CMS) (MIN) ***
0.000 0.000 0.000 0.000 0.00
0.250 5.580 0.007 0.315 368.51
0.500 11.160 0.028 2.004 232.09
0.750 16.740 0.063 5.908 177.10
0.925 17.722 0.093 10.903 142.06

```

1.100	18.703	0.125	17.146	121.31
1.275	19.685	0.158	24.592	107.35
1.450	20.667	0.194	33.219	97.18
1.625	21.648	0.231	43.020	89.39
1.800	22.630	0.269	53.998	83.17
2.150	24.593	0.352	79.522	73.80
2.500	26.557	0.442	109.900	66.97
2.850	28.520	0.538	145.280	61.72
3.550	41.894	0.784	250.661	52.16
4.250	55.269	1.125	397.011	47.21
4.950	68.643	1.558	595.612	43.60
5.650	82.018	2.086	855.894	40.61
6.350	95.392	2.706	1186.367	38.02
7.050	108.767	3.421	1594.936	35.75
7.750	122.141	4.229	2089.057	33.74
END FTABLE 5				

FTABLE 6				
ROWS COLS ***				
16 4				
DEPTH	AREA	VOLUME	DISCH	FLO-THRU ***
( m)	( HA)	(Mm3)	(CMS)	(MIN) ***
0.000	0.000	0.000	0.000	0.00
0.750	0.246	0.001	0.492	31.27
1.050	0.383	0.002	1.193	26.08
1.350	0.520	0.003	2.424	22.15
1.500	0.588	0.004	3.276	20.61
1.650	0.657	0.005	4.305	19.30
1.950	0.794	0.007	6.949	17.18
2.250	0.931	0.010	10.459	15.54
2.550	1.068	0.013	14.932	14.23
3.150	1.774	0.021	30.957	11.45
3.750	2.480	0.034	57.257	9.91
4.350	3.186	0.051	97.544	8.72
4.950	3.892	0.072	154.920	7.77
5.550	4.597	0.098	232.178	7.02
6.150	5.303	0.127	331.905	6.40
6.750	6.009	0.161	456.531	5.89
END FTABLE 6				

FTABLE 7				
ROWS COLS ***				
20 4				
DEPTH	AREA	VOLUME	DISCH	FLO-THRU ***
( m)	( HA)	(Mm3)	(CMS)	(MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	2.850	0.004	0.344	172.55
0.500	5.700	0.014	2.185	108.68
0.750	8.550	0.032	6.444	82.92
0.975	8.946	0.052	13.704	62.93
1.200	9.342	0.072	22.988	52.43
1.425	9.738	0.094	34.118	45.81
1.650	10.133	0.116	46.989	41.19
1.875	10.529	0.139	61.536	37.75
2.100	10.925	0.164	77.720	35.07
2.550	11.717	0.214	114.916	31.10
3.000	12.508	0.269	158.508	28.28
3.450	13.300	0.327	208.533	26.14
4.350	24.988	0.499	365.687	22.76
5.250	36.677	0.777	615.980	21.02
6.150	48.365	1.160	997.245	19.38
7.050	60.053	1.647	1540.859	17.82
7.950	71.742	2.240	2274.906	16.41
8.850	83.430	2.939	3225.292	15.19
9.750	95.118	3.742	4416.323	14.12
END FTABLE 7				

FTABLE 8  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.005	0.001	0.296	70.84
0.500	2.011	0.005	1.878	44.62
0.750	3.016	0.011	5.537	34.04
0.933	3.198	0.017	10.353	27.38
1.117	3.380	0.023	16.331	23.51
1.300	3.562	0.029	23.407	20.93
1.483	3.744	0.036	31.551	19.07
1.667	3.926	0.043	40.747	17.64
1.850	4.108	0.050	50.991	16.50
2.217	4.472	0.066	74.649	14.79
2.583	4.836	0.083	102.603	13.53
2.950	5.200	0.102	134.977	12.56
3.683	10.413	0.159	236.425	11.20
4.417	15.626	0.254	386.962	10.96
5.150	20.839	0.388	603.574	10.72
5.883	26.052	0.560	900.394	10.37
6.617	31.265	0.770	1290.109	9.95
7.350	36.478	1.019	1784.458	9.51
8.083	41.691	1.305	2394.474	9.08

END FTABLE 8

\*\*\* Henley Dam

FTABLE 9  
ROWS COLS \*\*\*  
19 5

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	DISCH (CMS) ***
0.000	0.000	0.000	0.000	0.00
2.000	1.100	0.001	0.558	0.00
3.000	3.900	0.020	1.000	00.00
4.000	6.300	0.061	1.000	00.00
5.000	9.100	0.125	1.000	00.00
6.000	12.200	0.216	1.000	00.00
7.000	15.100	0.343	1.000	00.00
8.000	18.300	0.508	2.000	00.00
9.000	22.200	0.714	2.000	00.00
10.000	26.100	0.964	3.000	00.00
11.000	29.700	1.257	3.000	00.00
11.800	32.100	1.522	4.000	00.00
12.000	33.000	1.590	4.000	00.00
13.000	38.000	1.971	5.000	00.00
13.200	39.000	2.053	10.000	13.00
13.400	40.000	2.134	20.000	36.00
13.600	40.800	2.219	20.000	67.00
13.800	41.600	2.306	100.000	105.00
14.000	42.300	2.392	200.000	249.00

END FTABLE 9

FTABLE 10  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.0000	0.000	0.00
0.250	1.943	0.002	0.172	235.89
0.500	3.885	0.010	1.090	148.57
0.750	5.828	0.022	3.213	113.36
0.942	6.556	0.034	6.083	92.39
1.133	7.285	0.047	9.809	79.84
1.325	8.013	0.062	14.417	71.27
1.517	8.742	0.078	19.943	64.94



1.708	9.470	0.095	26.430	60.01
1.900	10.199	0.114	33.917	56.02
2.283	11.656	0.156	52.072	49.90
2.667	13.113	0.203	74.756	45.34
3.050	14.570	0.256	102.311	41.77
3.817	28.701	0.422	196.058	35.90
4.583	42.831	0.697	348.689	33.29
5.350	56.962	1.079	582.947	30.85
6.117	71.093	1.570	917.760	28.51
6.883	85.224	2.169	1370.109	26.39
7.650	99.354	2.877	1955.685	24.52
8.417	113.485	3.693	2689.228	22.88

END FTABLE 10

FTABLE 11  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	*** ***
0.000	0.000	0.000	0.000	0.00	
0.250	1.503	0.002	0.231	135.63	
0.500	3.005	0.008	1.466	85.42	
0.750	4.508	0.017	4.323	65.18	
0.900	5.244	0.024	7.102	56.84	
1.050	5.980	0.033	10.682	50.92	
1.200	6.716	0.042	15.131	46.44	
1.350	7.452	0.053	20.514	42.88	
1.500	8.188	0.065	26.898	39.97	
1.650	8.924	0.077	34.345	37.54	
1.950	10.396	0.106	52.672	33.65	
2.250	11.868	0.140	75.969	30.65	
2.550	13.340	0.178	104.689	28.26	
3.150	24.852	0.292	204.730	23.78	
3.750	36.364	0.476	369.476	21.46	
4.350	47.876	0.728	623.969	19.46	
4.950	59.388	1.050	989.061	17.70	
5.550	70.900	1.441	1483.464	16.19	
6.150	82.412	1.901	2124.472	14.91	
6.750	93.924	2.430	2928.329	13.83	

END FTABLE 11

FTABLE 12  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	*** ***
0.000	0.000	0.000	0.000	0.00	
0.250	2.660	0.003	0.221	251.09	
0.500	5.320	0.013	1.402	158.14	
0.750	7.980	0.030	4.133	120.67	
0.967	8.327	0.048	8.654	91.66	
1.183	8.673	0.066	14.448	76.14	
1.400	9.020	0.085	21.419	66.28	
1.617	9.367	0.105	29.511	59.35	
1.833	9.714	0.126	38.688	54.18	
2.050	10.060	0.147	48.927	50.14	
2.483	10.754	0.192	72.548	44.17	
2.917	11.447	0.240	100.336	39.93	
3.350	12.141	0.291	132.314	36.72	
4.217	25.090	0.453	234.568	32.17	
5.083	38.039	0.726	412.457	29.35	
5.950	50.988	1.112	700.484	26.46	
6.817	63.937	1.610	1127.216	23.81	
7.683	76.885	2.220	1718.169	21.54	
8.550	89.834	2.943	2496.842	19.64	
9.417	102.783	3.778	3485.257	18.06	

END FTABLE 12

```

FTABLE      13
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      1.288      0.002      0.180      149.00
0.500      2.576      0.006      1.144      93.84
0.750      3.864      0.014      3.373      71.60
0.892      4.117      0.020      5.519      60.83
1.033      4.370      0.026      8.097      53.84
1.175      4.623      0.033      11.094     48.86
1.317      4.876      0.039      14.509     45.09
1.458      5.129      0.046      18.343     42.11
1.600      5.382      0.054      22.598     39.67
1.883      5.888      0.070      32.401     35.88
2.167      6.394      0.087      43.977     33.03
2.450      6.900      0.106      57.393     30.78
3.017      16.974     0.174     100.956     28.66
3.583      27.048     0.298     170.127     29.23
4.150      37.122     0.480     275.218     29.08
4.717      47.196     0.719     424.783     28.21
5.283      57.270     1.015     626.479     27.00
5.850      67.344     1.368     887.373     25.70
6.417      77.419     1.778    1214.095     24.41
END FTABLE 13
  
```

```

FTABLE      14
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      1.152      0.001      0.129     186.43
0.500      2.304      0.006      0.818     117.42
0.750      3.456      0.013      2.411      89.60
0.950      3.720      0.020      4.738      70.84
1.150      3.984      0.028      7.701      60.25
1.350      4.248      0.036     11.281      53.29
1.550      4.512      0.045     15.470      48.30
1.750      4.776      0.054     20.271      44.50
1.950      5.040      0.064     25.691      41.48
2.350      5.568      0.085     38.431      36.93
2.750      6.096      0.108     53.791      33.61
3.150      6.624      0.134     71.889      31.05
3.950     21.337      0.246    140.213      29.21
4.750     36.049      0.475    284.561      27.84
5.550     50.762      0.823    543.849      25.21
6.350     65.475      1.287    950.263      22.58
7.150     80.187      1.870   1532.510      20.34
7.950     94.900      2.570   2317.000      18.49
8.750    109.613      3.389   3328.455      16.97
END FTABLE 14
  
```

```

FTABLE      15
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      1.632      0.002      0.166     204.36
0.500      3.264      0.008      1.057     128.71
0.750      4.896      0.018      3.116      98.21
0.892      5.312      0.026      5.090      83.80
1.033      5.729      0.033      7.495      74.29
1.175      6.146      0.042     10.337      67.43
1.317      6.562      0.051     13.621      62.19
1.458      6.978      0.060     17.360      58.00
  
```

1.600	7.395	0.071	21.565	54.56
1.883	8.228	0.093	31.426	49.18
2.167	9.061	0.117	43.316	45.10
2.450	9.894	0.144	57.349	41.87
3.017	21.431	0.233	105.276	36.86
3.583	32.968	0.387	188.349	34.24
4.150	44.505	0.606	322.141	31.38
4.717	56.042	0.891	519.559	28.59
5.283	67.579	1.242	792.141	26.12
5.850	79.116	1.657	1150.521	24.01
6.417	90.652	2.138	1604.669	22.21

END FTABLE 15

FTABLE 16

ROWS COLS \*\*\*

20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.554	0.002	0.220	147.24
0.500	3.107	0.008	1.396	92.74
0.750	4.661	0.017	4.117	70.76
0.900	4.832	0.025	6.996	58.60
1.050	5.003	0.032	10.437	51.06
1.200	5.174	0.040	14.398	45.85
1.350	5.346	0.047	18.849	42.00
1.500	5.517	0.056	23.772	39.01
1.650	5.688	0.064	29.153	36.62
1.950	6.030	0.082	41.260	32.97
2.250	6.373	0.100	55.132	30.30
2.550	6.715	0.120	70.759	28.23
3.150	17.415	0.192	123.234	26.00
3.750	28.115	0.329	220.918	24.81
4.350	38.814	0.530	386.038	22.87
4.950	49.514	0.795	636.970	20.79
5.550	60.214	1.124	990.105	18.92
6.150	70.914	1.517	1460.522	17.31
6.750	81.613	1.975	2062.341	15.96

END FTABLE 16

FTABLE 17

ROWS COLS \*\*\*

20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	0.288	0.000	0.096	62.54
0.500	0.576	0.001	0.609	39.39
0.750	0.864	0.003	1.797	30.05
0.925	0.936	0.005	3.252	24.67
1.100	1.008	0.007	5.067	21.43
1.275	1.080	0.008	7.236	19.22
1.450	1.152	0.010	9.759	17.58
1.625	1.224	0.012	12.641	16.32
1.800	1.296	0.015	15.891	15.29
2.150	1.440	0.019	23.523	13.72
2.500	1.584	0.025	32.731	12.56
2.850	1.728	0.030	43.597	11.64
3.550	10.696	0.074	95.727	12.87
4.250	19.664	0.180	239.970	12.52
4.950	28.632	0.349	528.169	11.02
5.650	37.600	0.581	1003.147	9.65
6.350	46.568	0.876	1703.045	8.57
7.050	55.536	1.233	2662.914	7.72
7.750	64.504	1.653	3915.531	7.04

END FTABLE 17



```

FTABLE      18
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      3.300      0.004      0.061      1128.18
0.500      6.600      0.016      0.387      710.54
0.750      9.900      0.037      1.141      542.17
1.142      10.575      0.077      3.699      347.92
1.533      11.250      0.120      7.392      270.49
1.925      11.925      0.165      12.130      227.19
2.317      12.600      0.213      17.875      198.95
2.708      13.275      0.264      24.612      178.80
3.100      13.950      0.317      32.338      163.56
3.883      15.300      0.432      50.782      141.76
4.667      16.650      0.557      73.298      126.67
5.450      18.000      0.693      100.019      115.44
7.017      38.071      1.132      192.533      97.99
8.583      58.142      1.886      370.289      84.87
10.150      78.214      2.954      674.182      73.02
11.717      98.285      4.336      1138.079      63.50
13.283      118.356      6.033      1792.219      56.11
14.850      138.427      8.045      2664.445      50.32
16.417      158.498      10.371      3780.839      45.72
END FTABLE 18
  
```

```

FTABLE      19
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.017      0.003      0.078      538.80
0.500      4.033      0.010      0.495      339.34
0.750      6.050      0.023      1.460      258.93
1.142      6.609      0.047      4.712      167.94
1.533      7.168      0.074      9.445      131.39
1.925      7.727      0.104      15.580      110.86
2.317      8.287      0.135      23.097      97.41
2.708      8.846      0.169      32.003      87.77
3.100      9.405      0.204      42.319      80.45
3.883      10.523      0.282      67.303      69.92
4.667      11.642      0.369      98.332      62.57
5.450      12.760      0.465      135.725      57.07
7.017      25.026      0.761      266.643      47.55
8.583      37.291      1.249      510.398      40.78
10.150      49.557      1.929      918.941      34.99
11.717      61.823      2.802      1535.341      30.41
13.283      74.089      3.866      2398.096      26.87
14.850      86.354      5.123      3542.680      24.10
16.417      98.620      6.572      5002.338      21.90
END FTABLE 19
  
```

```

FTABLE      20
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      5.510      0.007      0.000      479.55
0.500      11.020      0.028      0.000      302.03
0.750      16.530      0.062      0.000      230.46
1.583      17.569      0.204      0.000      108.86
2.417      18.608      0.355      0.000      78.49
3.250      19.648      0.514      134.442      63.75
4.083      20.687      0.682      207.542      54.79
4.917      21.726      0.859      294.139      48.67
  
```

```

5.750    22.765    1.044    393.990    44.18
7.417    24.843    1.441    633.208    37.93
9.083    26.922    1.872    925.502    33.72
10.750   29.000    2.338   1271.978   30.64
14.083   97.802    4.452   2648.897   28.01
17.417  166.604    8.859   5914.768   24.96
20.750  235.406   15.559  12079.875   21.47
24.083  304.208   24.552  21979.436   18.62
27.417  373.009   35.839  36358.000   16.43
30.750  441.811   49.420  55900.180   14.73
34.083  510.613   65.293  81246.539   13.39
END FTABLE 20

```

END FTABLES

\*\*\* For an understanding of EXT SOURCES, EXT TARGETS, SCHEMATIC, MASS-LINK  
 \*\*\* and NETWORK blocks, the user should be familiar with the Time Series  
 \*\*\* Linkages and the Time Series Catalog

EXT SOURCES

```

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <Member-> ***
<Name> # <Name> # tem strg<-factor-->strg <Name> # # <Name> # # ***
WDM      1 PRCP  3 METR      0.31    PERLND  11  18  EXTNL  PREC
WDM      2 PRCP  3 METR      0.69    PERLND  11  18  EXTNL  PREC
WDM      1 PRCP  3 METR      0.31    IMPLND  14  17  EXTNL  PREC
WDM      2 PRCP  3 METR      0.69    IMPLND  14  17  EXTNL  PREC

WDM      2 PRCP  3 METR              PERLND  21  28  EXTNL  PREC
WDM      2 PRCP  3 METR              IMPLND  24  27  EXTNL  PREC

WDM      2 PRCP  3 METR              PERLND  31  38  EXTNL  PREC
WDM      2 PRCP  3 METR              IMPLND  34  37  EXTNL  PREC

WDM      1 PRCP  3 METR      0.30    PERLND  41  48  EXTNL  PREC
WDM      2 PRCP  3 METR      0.62    PERLND  41  48  EXTNL  PREC
WDM      4 PRCP  3 METR      0.08    PERLND  41  48  EXTNL  PREC
WDM      1 PRCP  3 METR      0.30    IMPLND  44  47  EXTNL  PREC
WDM      2 PRCP  3 METR      0.62    IMPLND  44  47  EXTNL  PREC
WDM      4 PRCP  3 METR      0.08    IMPLND  44  47  EXTNL  PREC

WDM      1 PRCP  3 METR      0.02    PERLND  51  58  EXTNL  PREC
WDM      2 PRCP  3 METR      0.06    PERLND  51  58  EXTNL  PREC
WDM      4 PRCP  3 METR      0.90    PERLND  51  58  EXTNL  PREC
WDM      6 PRCP  3 METR      0.02    PERLND  51  58  EXTNL  PREC
WDM      1 PRCP  3 METR      0.02    IMPLND  54  57  EXTNL  PREC
WDM      2 PRCP  3 METR      0.06    IMPLND  54  57  EXTNL  PREC
WDM      4 PRCP  3 METR      0.90    IMPLND  54  57  EXTNL  PREC
WDM      6 PRCP  3 METR      0.02    IMPLND  54  57  EXTNL  PREC

WDM      4 PRCP  3 METR              PERLND  61  68  EXTNL  PREC
WDM      4 PRCP  3 METR              IMPLND  64  67  EXTNL  PREC

WDM      1 PRCP  3 METR      0.33    PERLND  71  78  EXTNL  PREC
WDM      4 PRCP  3 METR      0.67    PERLND  71  78  EXTNL  PREC
WDM      1 PRCP  3 METR      0.33    IMPLND  74  77  EXTNL  PREC
WDM      4 PRCP  3 METR      0.67    IMPLND  74  77  EXTNL  PREC

WDM      3 PRCP  3 METR      0.03    PERLND  81  88  EXTNL  PREC
WDM      4 PRCP  3 METR      0.97    PERLND  81  88  EXTNL  PREC
WDM      3 PRCP  3 METR      0.03    IMPLND  84  87  EXTNL  PREC
WDM      4 PRCP  3 METR      0.97    IMPLND  84  87  EXTNL  PREC

WDM      4 PRCP  3 METR              PERLND  91  98  EXTNL  PREC
WDM      4 PRCP  3 METR              IMPLND  94  97  EXTNL  PREC

WDM      3 PRCP  3 METR      0.06    PERLND 101 108  EXTNL  PREC
WDM      4 PRCP  3 METR      0.94    PERLND 101 108  EXTNL  PREC

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WDM	3	PRCP	3	METR	0.06	IMPLND	104	107	EXTNL	PREC
WDM	4	PRCP	3	METR	0.94	IMPLND	104	107	EXTNL	PREC
WDM	4	PRCP	3	METR	0.89	PERLND	111	118	EXTNL	PREC
WDM	6	PRCP	3	METR	0.11	PERLND	111	118	EXTNL	PREC
WDM	4	PRCP	3	METR	0.89	IMPLND	114	117	EXTNL	PREC
WDM	6	PRCP	3	METR	0.11	IMPLND	114	117	EXTNL	PREC
WDM	4	PRCP	3	METR	0.95	PERLND	121	128	EXTNL	PREC
WDM	5	PRCP	3	METR	0.05	PERLND	121	128	EXTNL	PREC
WDM	4	PRCP	3	METR	0.95	IMPLND	124	127	EXTNL	PREC
WDM	5	PRCP	3	METR	0.05	IMPLND	124	127	EXTNL	PREC
WDM	4	PRCP	3	METR	0.72	PERLND	131	138	EXTNL	PREC
WDM	5	PRCP	3	METR	0.05	PERLND	131	138	EXTNL	PREC
WDM	6	PRCP	3	METR	0.23	PERLND	131	138	EXTNL	PREC
WDM	4	PRCP	3	METR	0.72	IMPLND	134	137	EXTNL	PREC
WDM	5	PRCP	3	METR	0.05	IMPLND	134	137	EXTNL	PREC
WDM	6	PRCP	3	METR	0.23	IMPLND	134	137	EXTNL	PREC
WDM	5	PRCP	3	METR		PERLND	141	148	EXTNL	PREC
WDM	5	PRCP	3	METR		IMPLND	144	147	EXTNL	PREC
WDM	4	PRCP	3	METR	0.13	PERLND	151	158	EXTNL	PREC
WDM	5	PRCP	3	METR	0.03	PERLND	151	158	EXTNL	PREC
WDM	6	PRCP	3	METR	0.55	PERLND	151	158	EXTNL	PREC
WDM	8	PRCP	3	METR	0.29	PERLND	151	158	EXTNL	PREC
WDM	4	PRCP	3	METR	0.13	IMPLND	154	157	EXTNL	PREC
WDM	5	PRCP	3	METR	0.03	IMPLND	154	157	EXTNL	PREC
WDM	6	PRCP	3	METR	0.55	IMPLND	154	157	EXTNL	PREC
WDM	8	PRCP	3	METR	0.29	IMPLND	154	157	EXTNL	PREC
WDM	6	PRCP	3	METR	0.08	PERLND	161	168	EXTNL	PREC
WDM	8	PRCP	3	METR	0.92	PERLND	161	168	EXTNL	PREC
WDM	6	PRCP	3	METR	0.08	IMPLND	164	167	EXTNL	PREC
WDM	8	PRCP	3	METR	0.92	IMPLND	164	167	EXTNL	PREC
WDM	5	PRCP	3	METR	0.15	PERLND	171	178	EXTNL	PREC
WDM	8	PRCP	3	METR	0.85	PERLND	171	178	EXTNL	PREC
WDM	5	PRCP	3	METR	0.15	IMPLND	174	177	EXTNL	PREC
WDM	8	PRCP	3	METR	0.85	IMPLND	174	177	EXTNL	PREC
WDM	5	PRCP	3	METR	0.82	PERLND	181	188	EXTNL	PREC
WDM	8	PRCP	3	METR	0.18	PERLND	181	188	EXTNL	PREC
WDM	5	PRCP	3	METR	0.82	IMPLND	184	187	EXTNL	PREC
WDM	8	PRCP	3	METR	0.18	IMPLND	184	187	EXTNL	PREC
WDM	5	PRCP	3	METR	0.62	PERLND	191	198	EXTNL	PREC
WDM	8	PRCP	3	METR	0.38	PERLND	191	198	EXTNL	PREC
WDM	5	PRCP	3	METR	0.62	IMPLND	194	197	EXTNL	PREC
WDM	8	PRCP	3	METR	0.38	IMPLND	194	197	EXTNL	PREC
WDM	5	PRCP	3	METR	0.87	PERLND	201	208	EXTNL	PREC
WDM	8	PRCP	3	METR	0.12	PERLND	201	208	EXTNL	PREC
WDM	9	PRCP	3	METR	0.01	PERLND	201	208	EXTNL	PREC
WDM	5	PRCP	3	METR	0.87	IMPLND	204	207	EXTNL	PREC
WDM	8	PRCP	3	METR	0.12	IMPLND	204	207	EXTNL	PREC
WDM	9	PRCP	3	METR	0.01	IMPLND	204	207	EXTNL	PREC
Evap region 1 ***										
Covers catchments 1-9 ***										
WDM	13	EVAP	3	METR	0.85	PERLND	11	98	EXTNL	PETINP
WDM	13	EVAP	3	METR	0.85	IMPLND	14	98	EXTNL	PETINP
Evap region 2 ***										
Covers catchments 10-14,17-30,34-37,41-42 ***										
WDM	14	EVAP	3	METR	0.85	PERLND	101	148	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	104	148	EXTNL	PETINP



WDM	14	EVAP	3	METR	0.85	PERLND	171	308	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	174	308	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	PERLND	341	378	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	344	378	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	PERLND	411	428	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	414	428	EXTNL	PETINP

Evap region 3 \*\*\*

Covers catchments 15-16,31-33,38-40 \*\*\*

WDM	15	EVAP	3	METR	0.85	PERLND	151	168	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	154	168	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	PERLND	311	338	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	314	338	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	PERLND	381	408	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	384	408	EXTNL	PETINP

Abstractions from Henley. Values are in m3/month.

Factor converts from m3/day totals to m3/s. HSPF automatically  
disaggregates from monthly to daily, so we don't have to take account of  
conversion from monthly to daily

\*\*\*  
\*\*\*  
\*\*\*  
\*\*\*

WDM	692	DISC	3	METR	0.000012	RCHRES	9	EXTNL	OUTDGT	1	1
-----	-----	------	---	------	----------	--------	---	-------	--------	---	---

END EXT SOURCES

SCHEMATIC

<-Source->		<--Area-->		<-Target->	<ML->	***
<Name>	#	<-factor->		<Name>	#	***
		(ha)				***
PERLND	11	235.8		RCHRES	1	1
PERLND	12	523.0		RCHRES	1	1
PERLND	13	1906.2		RCHRES	1	1
PERLND	14	***		RCHRES	1	1
PERLND	15	164.8		RCHRES	1	1
IMPLND	15	18.3		RCHRES	1	3
PERLND	16	961.1		RCHRES	1	1
PERLND	17	***		RCHRES	1	1
PERLND	18	87.5		RCHRES	1	1
PERLND	21	200.7		RCHRES	2	1
PERLND	22	652.1		RCHRES	2	1
PERLND	23	3330.8		RCHRES	2	1
PERLND	24	4.4		RCHRES	2	1
IMPLND	24	26.9		RCHRES	2	3
PERLND	25	***		RCHRES	2	1
PERLND	26	950.8		RCHRES	2	1
PERLND	27	***		RCHRES	2	1
PERLND	28	200.0		RCHRES	2	1
PERLND	31	14.2		RCHRES	3	1
PERLND	32	***		RCHRES	3	1
PERLND	33	71.2		RCHRES	3	1
PERLND	34	***		RCHRES	3	1
PERLND	35	***		RCHRES	3	1
PERLND	36	54.3		RCHRES	3	1
PERLND	37	***		RCHRES	3	1
PERLND	38	***		RCHRES	3	1
PERLND	41	597.6		RCHRES	4	1
PERLND	42	537.6		RCHRES	4	1
PERLND	43	2100.8		RCHRES	4	1
PERLND	44	***		RCHRES	4	1
PERLND	45	***		RCHRES	4	1
PERLND	46	635.4		RCHRES	4	1
PERLND	47	18.5		RCHRES	4	1
IMPLND	47	6.5		RCHRES	4	3
PERLND	48	12.5		RCHRES	4	1
PERLND	51	238.8		RCHRES	5	1
PERLND	52	507.4		RCHRES	5	1
PERLND	53	2386.4		RCHRES	5	1
PERLND	54	***		RCHRES	5	1
PERLND	55	15.6		RCHRES	5	1

IMPLND	55		1.7	RCHRES	5	3
PERLND	56		1230.7	RCHRES	5	1
PERLND	57	***		RCHRES	5	1
PERLND	58		75.0	RCHRES	5	1
PERLND	61		4.6	RCHRES	6	1
PERLND	62	***		RCHRES	6	1
PERLND	63		12.4	RCHRES	6	1
PERLND	64	***		RCHRES	6	1
PERLND	65		17.8	RCHRES	6	1
IMPLND	65		2.0	RCHRES	6	3
PERLND	66	***		RCHRES	6	1
PERLND	67	***		RCHRES	6	1
PERLND	68	***		RCHRES	6	1
PERLND	71		515.2	RCHRES	7	1
PERLND	72		430.8	RCHRES	7	1
PERLND	73		1253.5	RCHRES	7	1
PERLND	74	***		RCHRES	7	1
PERLND	75		238.5	RCHRES	7	1
IMPLND	75		26.5	RCHRES	7	3
PERLND	76		571.8	RCHRES	7	1
PERLND	77	***		RCHRES	7	1
PERLND	78	***		RCHRES	7	1
PERLND	81		370.7	RCHRES	8	1
PERLND	82		34.5	RCHRES	8	1
PERLND	83		138.8	RCHRES	8	1
PERLND	84	***		RCHRES	8	1
PERLND	85		180.1	RCHRES	8	1
IMPLND	85		20.0	RCHRES	8	3
PERLND	86	***		RCHRES	8	1
PERLND	87	***		RCHRES	8	1
PERLND	88	***		RCHRES	8	1
PERLND	91		171.7	RCHRES	9	1
PERLND	92	***		RCHRES	9	1
PERLND	93		84.7	RCHRES	9	1
PERLND	94	***		RCHRES	9	1
PERLND	95		90.9	RCHRES	9	1
IMPLND	95		10.1	RCHRES	9	3
PERLND	96	***		RCHRES	9	1
PERLND	97	***		RCHRES	9	1
PERLND	98	***		RCHRES	9	1
PERLND	101		332.1	RCHRES	10	1
PERLND	102		269.3	RCHRES	10	1
PERLND	103		760.6	RCHRES	10	1
PERLND	104		6.6	RCHRES	10	1
IMPLND	104		40.8	RCHRES	10	3
PERLND	105		1405.4	RCHRES	10	1
IMPLND	105		156.2	RCHRES	10	3
PERLND	106		0.1	RCHRES	10	1
PERLND	107		12.3	RCHRES	10	1
IMPLND	107		4.3	RCHRES	10	3
PERLND	108	***		RCHRES	10	1
PERLND	111		1023.2	RCHRES	11	1
PERLND	112	***		RCHRES	11	1
PERLND	113		2119.3	RCHRES	11	1
PERLND	114		14.1	RCHRES	11	1
IMPLND	114		86.7	RCHRES	11	3
PERLND	115		42.8	RCHRES	11	1
IMPLND	115		4.8	RCHRES	11	3
PERLND	116		283.7	RCHRES	11	1
PERLND	117		0.1	RCHRES	11	1
PERLND	118	***		RCHRES	11	1
PERLND	121		170.5	RCHRES	12	1
PERLND	122	***		RCHRES	12	1
PERLND	123		653.1	RCHRES	12	1
PERLND	124		7.4	RCHRES	12	1
IMPLND	124		45.5	RCHRES	12	3
PERLND	125		598.3	RCHRES	12	1
IMPLND	125		66.5	RCHRES	12	3

PERLND 126	***		RCHRES 12	1
PERLND 127		291.6	RCHRES 12	1
IMPLND 127		102.4	RCHRES 12	3
PERLND 128	***		RCHRES 12	1
PERLND 131		80.8	RCHRES 13	1
PERLND 132	***		RCHRES 13	1
PERLND 133		1128.7	RCHRES 13	1
PERLND 134		21.8	RCHRES 13	1
IMPLND 134		133.6	RCHRES 13	3
PERLND 135		19.3	RCHRES 13	1
IMPLND 135		2.1	RCHRES 13	3
PERLND 136		103.6	RCHRES 13	1
PERLND 137		9.0	RCHRES 13	1
IMPLND 137		3.2	RCHRES 13	3
PERLND 138	***		RCHRES 13	1
PERLND 141		46.0	RCHRES 14	1
PERLND 142	***		RCHRES 14	1
PERLND 143		618.3	RCHRES 14	1
PERLND 144		30.2	RCHRES 14	1
IMPLND 144		185.8	RCHRES 14	3
PERLND 145		1.4	RCHRES 14	1
PERLND 146	***		RCHRES 14	1
PERLND 147	***		RCHRES 14	1
PERLND 148	***		RCHRES 14	1
PERLND 151		223.5	RCHRES 15	1
PERLND 152		174.0	RCHRES 15	1
PERLND 153		1646.5	RCHRES 15	1
PERLND 154		52.9	RCHRES 15	1
IMPLND 154		325.1	RCHRES 15	3
PERLND 155		14.5	RCHRES 15	1
IMPLND 155		1.6	RCHRES 15	3
PERLND 156		220.4	RCHRES 15	1
PERLND 157	***		RCHRES 15	1
PERLND 158	***		RCHRES 15	1
PERLND 161		105.4	RCHRES 16	1
PERLND 162		417.1	RCHRES 16	1
PERLND 163		1081.4	RCHRES 16	1
PERLND 164		19.2	RCHRES 16	1
IMPLND 164		117.8	RCHRES 16	3
PERLND 165	***		RCHRES 16	1
PERLND 166		49.6	RCHRES 16	1
PERLND 167	***		RCHRES 16	1
PERLND 168	***		RCHRES 16	1
PERLND 171	***		RCHRES 17	1
PERLND 172	***		RCHRES 17	1
PERLND 173		181.1	RCHRES 17	1
PERLND 174		2.3	RCHRES 17	1
IMPLND 174		14.4	RCHRES 17	3
PERLND 175		200.7	RCHRES 17	1
IMPLND 175		22.3	RCHRES 17	3
PERLND 176	***		RCHRES 17	1
PERLND 177	***		RCHRES 17	1
PERLND 178	***		RCHRES 17	1
PERLND 181		44.9	RCHRES 20	1
PERLND 182	***		RCHRES 20	1
PERLND 183		218.7	RCHRES 20	1
PERLND 184		2.9	RCHRES 20	1
IMPLND 184		17.9	RCHRES 20	3
PERLND 185		48.9	RCHRES 20	1
IMPLND 185		5.4	RCHRES 20	3
PERLND 186	***		RCHRES 20	1
PERLND 187		16.7	RCHRES 20	1
IMPLND 187		5.9	RCHRES 20	3
PERLND 188	***		RCHRES 20	1
PERLND 191		0.2	RCHRES 20	1
PERLND 192		75.8	RCHRES 20	1
PERLND 193		66.7	RCHRES 20	1
PERLND 194	***		RCHRES 20	1



```

PERLND 195          110.0      RCHRES 20      1
IMPLND 195          12.2      RCHRES 20      3
PERLND 196          ***      RCHRES 20      1
PERLND 197          ***      RCHRES 20      1
PERLND 198          ***      RCHRES 20      1
PERLND 201          5.1       RCHRES 20      1
PERLND 202          147.6     RCHRES 20      1
PERLND 203          1.5       RCHRES 20      1
PERLND 204          ***      RCHRES 20      1
PERLND 205          42.5      RCHRES 20      1
IMPLND 205          4.7       RCHRES 20      3
PERLND 206          ***      RCHRES 20      1
PERLND 207          1.4       RCHRES 20      1
IMPLND 207          0.5       RCHRES 20      3
PERLND 208          ***      RCHRES 20      1
RCHRES 1            RCHRES 3      2
RCHRES 2            RCHRES 3      2
RCHRES 3            RCHRES 4      2
RCHRES 4            RCHRES 5      2
RCHRES 5            RCHRES 6      2
RCHRES 6            RCHRES 9      2
RCHRES 7            RCHRES 9      2
RCHRES 8            RCHRES 9      2
RCHRES 9            RCHRES 10     4
RCHRES 10           RCHRES 12     2
RCHRES 11           RCHRES 12     2
RCHRES 12           RCHRES 14     2
RCHRES 13           RCHRES 14     2
RCHRES 14           RCHRES 20     2
RCHRES 15           RCHRES 17     2
RCHRES 16           RCHRES 17     2
RCHRES 17           RCHRES 20     2
RCHRES 18           RCHRES 20     2***
RCHRES 19           RCHRES 20     2***
END SCHEMATIC

```

#### MASS-LINK

```

MASS-LINK 1
<Srce>      <-Grp> <-Member-><--Mult--> <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor-> <Name>      <Name> <Name> # # ***
Factor converts (mm.ha) to (million m3) ***
PERLND PWATER PERO 0.00001 RCHRES INFLOW IVOL
END MASS-LINK 1

```

```

MASS-LINK 2
<Srce>      <-Grp> <-Member-><--Mult--> <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor-> <Name>      <Name> <Name> # # ***
RCHRES ROFLOW RCHRES INFLOW
END MASS-LINK 2

```

```

MASS-LINK 3
<Srce>      <-Grp> <-Member-><--Mult--> <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor-> <Name>      <Name> <Name> # # ***
Factor converts (mm.ha) to (million m3) ***
IMPLND IWATER SURO 0.00001 RCHRES INFLOW IVOL
END MASS-LINK 3

```

```

MASS-LINK 4
<Srce>      <-Grp> <-Member-><--Mult--> <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor-> <Name>      <Name> <Name> # # ***
For dams, only spillway and flow-thru goes to next reach ***
RCHRES OFLOW OVOL 2 1 RCHRES INFLOW IVOL
RCHRES OFLOW OVOL 3 1 RCHRES INFLOW IVOL
END MASS-LINK 4

```

END MASS-LINK

```

NETWORK
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # #<-factor->strg <Name> # # <Name> # # ***
RCHRES 5 HYDR RO PLTGEN 1 INPUT POINT 1
RCHRES 9 HYDR O 2 COPY 1 INPUT POINT 1
RCHRES 9 HYDR O 3 COPY 1 INPUT POINT 1
END NETWORK

EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Aggr Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name>qf tem strg strg***

*** For selected reaches, we output the following simulated time series:
*** Total outflow rate (m3/s)

RCHRES 5 HYDR RO WDM 805 HYDR 1 METR AGGR REPL
COPY 1 OUTPUT POINT 1 WDM 809 HYDR 1 METR AGGR REPL
RCHRES 14 HYDR RO WDM 814 HYDR 1 METR AGGR REPL
RCHRES 20 HYDR ROVOL WDM 820 HYDR 1 METR AGGR REPL
END EXT TARGETS

END RUN

```

## LOWER SUB-CATCHMENTS (21-42)

RUN

GLOBAL

**HSPF DEMONSTRATION RUN UPSTREAM OF MGENI CONFLUENCE**

**START 1992/01/01 END 2000/06/01**

RUN INTERP OUTPUT LEVEL 4

RESUME 0 RUN 1 TSSFL 0 WDMSFL 0 UNITS 2

END GLOBAL

FILES

<FILE> <UN#>\*\*\*<---FILE NAME----->

WDM 21 msunduzi.wdm

MESSU 22 msunduzi.ech

61 msunduzi.p61

62 msunduzi.p62

63 msunduzi.p63

64 msunduzi.p64

92 msunduzi.plt

END FILES

OPN SEQUENCE

INGRP INDELT 24:00

Catchment 21 \*\*\*\*\*

PERLND 211

PERLND 212

PERLND 213

PERLND 214

IMPLND 214

PERLND 215

IMPLND 215

PERLND 216\*\*\*

PERLND 217

IMPLND 217

PERLND 218\*\*\*

RCHRES 21

Catchment 22 \*\*\*\*\*

PERLND 221

PERLND 222

PERLND 223

PERLND 224\*\*\*

PERLND 225

IMPLND 225

PERLND 226\*\*\*

PERLND 227

IMPLND 227

PERLND 228\*\*\*

RCHRES 22

Catchment 23 \*\*\*\*\*

PERLND 231

PERLND 232

PERLND 233

PERLND 234

IMPLND 234

PERLND 235

IMPLND 235

PERLND 236\*\*\*

PERLND 237

IMPLND 237

PERLND 238\*\*\*

RCHRES 23

Catchment 24 \*\*\*\*\*

PERLND 241

PERLND 242

PERLND 243

PERLND 244



---

IMPLND	244
PERLND	245
IMPLND	245
PERLND	246***
PERLND	247
IMPLND	247
PERLND	248***
RCHRES	24
Catchment	25 *****
PERLND	251
PERLND	252
PERLND	253
PERLND	254
IMPLND	254
PERLND	255
IMPLND	255
PERLND	256***
PERLND	257
IMPLND	257
PERLND	258***
RCHRES	25
Catchment	26 *****
PERLND	261
PERLND	262
PERLND	263
PERLND	264
IMPLND	264
PERLND	265
IMPLND	265
PERLND	266***
PERLND	267***
PERLND	268***
RCHRES	26
Catchment	27 *****
PERLND	271
PERLND	272
PERLND	273
PERLND	274
IMPLND	274
PERLND	275***
PERLND	276***
PERLND	277
IMPLND	277
PERLND	278***
RCHRES	27
Catchment	28 *****
PERLND	281
PERLND	282
PERLND	283
PERLND	284
IMPLND	284
PERLND	285***
PERLND	286***
PERLND	287***
PERLND	288***
RCHRES	28
Catchment	29 *****
PERLND	291
PERLND	292
PERLND	293
PERLND	294
IMPLND	294
PERLND	295***
PERLND	296***
PERLND	297
IMPLND	297
PERLND	298***
RCHRES	29

---

---

```
Catchment 30 *****
PERLND 301
PERLND 302
PERLND 303
PERLND 304
PERLND 305***
PERLND 306***
PERLND 307
IMPLND 307
PERLND 308***
RCHRES 30
Catchment 31 *****
PERLND 311
PERLND 312
PERLND 313
PERLND 314
IMPLND 314
PERLND 315***
PERLND 316***
PERLND 317***
PERLND 318***
RCHRES 31
Catchment 32 *****
PERLND 321
PERLND 322
PERLND 323
PERLND 324***
PERLND 325***
PERLND 326***
PERLND 327***
PERLND 328***
RCHRES 32
Catchment 33 *****
PERLND 331
PERLND 332
PERLND 333
PERLND 334
IMPLND 334
PERLND 335***
PERLND 336***
PERLND 337***
PERLND 338***
RCHRES 33
Catchment 34 *****
PERLND 341
PERLND 342
PERLND 343
PERLND 344
IMPLND 344
PERLND 345***
PERLND 346***
PERLND 347
IMPLND 347
PERLND 348***
RCHRES 34
Catchment 35 *****
PERLND 351
PERLND 352
PERLND 353
PERLND 354
IMPLND 354
PERLND 355***
PERLND 356
PERLND 357***
PERLND 358***
RCHRES 35
Catchment 36 *****
PERLND 361
```

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```

PERLND      362
PERLND      363
PERLND      364***
PERLND      365***
PERLND      366
PERLND      367***
PERLND      368***
RCHRES       36
Catchment    37 *****
PERLND      371
PERLND      372
PERLND      373
PERLND      374***
PERLND      375
IMPLND      375
PERLND      376
PERLND      377***
PERLND      378***
RCHRES       37
Catchment    38 *****
PERLND      381
PERLND      382
PERLND      383
PERLND      384***
PERLND      385
IMPLND      385
PERLND      386***
PERLND      387***
PERLND      388***
RCHRES       38
Catchment    39 *****
PERLND      391
PERLND      392
PERLND      393
PERLND      394
IMPLND      394
PERLND      395
IMPLND      395
PERLND      396***
PERLND      397***
PERLND      398***
RCHRES       39
Catchment    40 *****
PERLND      401
PERLND      402
PERLND      403
PERLND      404
IMPLND      404
PERLND      405
IMPLND      405
PERLND      406
PERLND      407***
PERLND      408***
RCHRES       40
Catchment    41 *****
PERLND      411
PERLND      412
PERLND      413***
PERLND      414***
PERLND      415
IMPLND      415
PERLND      416
PERLND      417***
PERLND      418***
RCHRES       41
Catchment    42 *****
PERLND      421
PERLND      422

```

---



```

PERLND      423
PERLND      424
IMPLND      424
PERLND      425
IMPLND      425
PERLND      426
PERLND      427***
PERLND      428***
RCHRES      42
PLTGEN      1***
END INGRP
END OPN SEQUENCE

```

PERLND

```

ACTIVITY
<PLS >      Active Sections (1=Active; 0=Inactive)      ***
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
11 428              1
END ACTIVITY

```

```

PRINT-INFO
<PLS >      Print-flags      *** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
11 428      5              5      09
END PRINT-INFO

```

```

GEN-INFO
<PLS ><-----Name----->      Unit-systems      Printer ***
# - #      t-series Engl Metr ***
      in out      ***
211 Catch 21, Forest      2      2      0      61
212 Catch 21, Crops      2      2      0      61
213 Catch 21, Grassland      2      2      0      61
214 Catch 21, CBD      2      2      0      61
215 Catch 21, MD Res.      2      2      0      61
216 Catch 21, LD Res.      2      2      0      61
217 Catch 21, HD Res.      2      2      0      61
218 Catch 21, Wetland      2      2      0      61
221 Catch 22, Forest      2      2      0      61
222 Catch 22, Crops      2      2      0      61
223 Catch 22, Grassland      2      2      0      61
224 Catch 22, CBD      2      2      0      61
225 Catch 22, MD Res.      2      2      0      61
226 Catch 22, LD Res.      2      2      0      61
227 Catch 22, HD Res.      2      2      0      61
228 Catch 22, Wetland      2      2      0      61
231 Catch 23, Forest      2      2      0      61
232 Catch 23, Crops      2      2      0      61
233 Catch 23, Grassland      2      2      0      61
234 Catch 23, CBD      2      2      0      61
235 Catch 23, MD Res.      2      2      0      61
236 Catch 23, LD Res.      2      2      0      61
237 Catch 23, HD Res.      2      2      0      61
238 Catch 23, Wetland      2      2      0      61
241 Catch 24, Forest      2      2      0      61
242 Catch 24, Crops      2      2      0      61
243 Catch 24, Grassland      2      2      0      61
244 Catch 24, CBD      2      2      0      61
245 Catch 24, MD Res.      2      2      0      61
246 Catch 24, LD Res.      2      2      0      61
247 Catch 24, HD Res.      2      2      0      61
248 Catch 24, Wetland      2      2      0      61
251 Catch 25, Forest      2      2      0      61
252 Catch 25, Crops      2      2      0      61
253 Catch 25, Grassland      2      2      0      61
254 Catch 25, CBD      2      2      0      61

```

255	Catch 25, MD Res.	2	2	0	61
256	Catch 25, LD Res.	2	2	0	61
257	Catch 25, HD Res.	2	2	0	61
258	Catch 25, Wetland	2	2	0	61
261	Catch 26, Forest	2	2	0	61
262	Catch 26, Crops	2	2	0	61
263	Catch 26, Grassland	2	2	0	61
264	Catch 26, CBD	2	2	0	61
265	Catch 26, MD Res.	2	2	0	61
266	Catch 26, LD Res.	2	2	0	61
267	Catch 26, HD Res.	2	2	0	61
268	Catch 26, Wetland	2	2	0	61
271	Catch 27, Forest	2	2	0	61
272	Catch 27, Crops	2	2	0	61
273	Catch 27, Grassland	2	2	0	61
274	Catch 27, CBD	2	2	0	61
275	Catch 27, MD Res.	2	2	0	61
276	Catch 27, LD Res.	2	2	0	61
277	Catch 27, HD Res.	2	2	0	61
278	Catch 27, Wetland	2	2	0	61
281	Catch 28, Forest	2	2	0	61
282	Catch 28, Crops	2	2	0	61
283	Catch 28, Grassland	2	2	0	61
284	Catch 28, CBD	2	2	0	61
285	Catch 28, MD Res.	2	2	0	61
286	Catch 28, LD Res.	2	2	0	61
287	Catch 28, HD Res.	2	2	0	61
288	Catch 28, Wetland	2	2	0	61
291	Catch 29, Forest	2	2	0	61
292	Catch 29, Crops	2	2	0	61
293	Catch 29, Grassland	2	2	0	61
294	Catch 29, CBD	2	2	0	61
295	Catch 29, MD Res.	2	2	0	61
296	Catch 29, LD Res.	2	2	0	61
297	Catch 29, HD Res.	2	2	0	61
298	Catch 29, Wetland	2	2	0	61
301	Catch 30, Forest	2	2	0	61
302	Catch 30, Crops	2	2	0	61
303	Catch 30, Grassland	2	2	0	61
304	Catch 30, CBD	2	2	0	61
305	Catch 30, MD Res.	2	2	0	61
306	Catch 30, LD Res.	2	2	0	61
307	Catch 30, HD Res.	2	2	0	61
308	Catch 30, Wetland	2	2	0	61
311	Catch 31, Forest	2	2	0	61
312	Catch 31, Crops	2	2	0	61
313	Catch 31, Grassland	2	2	0	61
314	Catch 31, CBD	2	2	0	61
315	Catch 31, MD Res.	2	2	0	61
316	Catch 31, LD Res.	2	2	0	61
317	Catch 31, HD Res.	2	2	0	61
318	Catch 31, Wetland	2	2	0	61
321	Catch 32, Forest	2	2	0	61
322	Catch 32, Crops	2	2	0	61
323	Catch 32, Grassland	2	2	0	61
324	Catch 32, CBD	2	2	0	61
325	Catch 32, MD Res.	2	2	0	61
326	Catch 32, LD Res.	2	2	0	61
327	Catch 32, HD Res.	2	2	0	61
328	Catch 32, Wetland	2	2	0	61
331	Catch 33, Forest	2	2	0	61
332	Catch 33, Crops	2	2	0	61
333	Catch 33, Grassland	2	2	0	61
334	Catch 33, CBD	2	2	0	61
335	Catch 33, MD Res.	2	2	0	61
336	Catch 33, LD Res.	2	2	0	61
337	Catch 33, HD Res.	2	2	0	61
338	Catch 33, Wetland	2	2	0	61

341	Catch 34, Forest	2	2	0	61
342	Catch 34, Crops	2	2	0	61
343	Catch 34, Grassland	2	2	0	61
344	Catch 34, CBD	2	2	0	61
345	Catch 34, MD Res.	2	2	0	61
346	Catch 34, LD Res.	2	2	0	61
347	Catch 34, HD Res.	2	2	0	61
348	Catch 34, Wetland	2	2	0	61
351	Catch 35, Forest	2	2	0	61
352	Catch 35, Crops	2	2	0	61
353	Catch 35, Grassland	2	2	0	61
354	Catch 35, CBD	2	2	0	61
355	Catch 35, MD Res.	2	2	0	61
356	Catch 35, LD Res.	2	2	0	61
357	Catch 35, HD Res.	2	2	0	61
358	Catch 35, Wetland	2	2	0	61
361	Catch 36, Forest	2	2	0	61
362	Catch 36, Crops	2	2	0	61
363	Catch 36, Grassland	2	2	0	61
364	Catch 36, CBD	2	2	0	61
365	Catch 36, MD Res.	2	2	0	61
366	Catch 36, LD Res.	2	2	0	61
367	Catch 36, HD Res.	2	2	0	61
368	Catch 36, Wetland	2	2	0	61
371	Catch 37, Forest	2	2	0	61
372	Catch 37, Crops	2	2	0	61
373	Catch 37, Grassland	2	2	0	61
374	Catch 37, CBD	2	2	0	61
375	Catch 37, MD Res.	2	2	0	61
376	Catch 37, LD Res.	2	2	0	61
377	Catch 37, HD Res.	2	2	0	61
378	Catch 37, Wetland	2	2	0	61
381	Catch 38, Forest	2	2	0	61
382	Catch 38, Crops	2	2	0	61
383	Catch 38, Grassland	2	2	0	61
384	Catch 38, CBD	2	2	0	61
385	Catch 38, MD Res.	2	2	0	61
386	Catch 38, LD Res.	2	2	0	61
387	Catch 38, HD Res.	2	2	0	61
388	Catch 38, Wetland	2	2	0	61
391	Catch 39, Forest	2	2	0	61
392	Catch 39, Crops	2	2	0	61
393	Catch 39, Grassland	2	2	0	61
394	Catch 39, CBD	2	2	0	61
395	Catch 39, MD Res.	2	2	0	61
396	Catch 39, LD Res.	2	2	0	61
397	Catch 39, HD Res.	2	2	0	61
398	Catch 39, Wetland	2	2	0	61
401	Catch 40, Forest	2	2	0	61
402	Catch 40, Crops	2	2	0	61
403	Catch 40, Grassland	2	2	0	61
404	Catch 40, CBD	2	2	0	61
405	Catch 40, MD Res.	2	2	0	61
406	Catch 40, LD Res.	2	2	0	61
407	Catch 40, HD Res.	2	2	0	61
408	Catch 40, Wetland	2	2	0	61
411	Catch 41, Forest	2	2	0	61
412	Catch 41, Crops	2	2	0	61
413	Catch 41, Grassland	2	2	0	61
414	Catch 41, CBD	2	2	0	61
415	Catch 41, MD Res.	2	2	0	61
416	Catch 41, LD Res.	2	2	0	61
417	Catch 41, HD Res.	2	2	0	61
418	Catch 41, Wetland	2	2	0	61
421	Catch 42, Forest	2	2	0	61
422	Catch 42, Crops	2	2	0	61
423	Catch 42, Grassland	2	2	0	61
424	Catch 42, CBD	2	2	0	61



425	Catch 42, MD Res.	2	2	0	61
426	Catch 42, LD Res.	2	2	0	61
427	Catch 42, HD Res.	2	2	0	61
428	Catch 42, Wetland	2	2	0	61

END GEN-INFO

PWAT-PARM1

```
<PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE IFFC ***
```

212	0	0	0	1	0	0	0	0	1
213	0	0	0	1	0	0	0	0	1
222	0	0	0	1	0	0	0	0	1
223	0	0	0	1	0	0	0	0	1
232	0	0	0	1	0	0	0	0	1
233	0	0	0	1	0	0	0	0	1
242	0	0	0	1	0	0	0	0	1
243	0	0	0	1	0	0	0	0	1
252	0	0	0	1	0	0	0	0	1
253	0	0	0	1	0	0	0	0	1
262	0	0	0	1	0	0	0	0	1
263	0	0	0	1	0	0	0	0	1
272	0	0	0	1	0	0	0	0	1
273	0	0	0	1	0	0	0	0	1
282	0	0	0	1	0	0	0	0	1
283	0	0	0	1	0	0	0	0	1
292	0	0	0	1	0	0	0	0	1
293	0	0	0	1	0	0	0	0	1
302	0	0	0	1	0	0	0	0	1
303	0	0	0	1	0	0	0	0	1
312	0	0	0	1	0	0	0	0	1
313	0	0	0	1	0	0	0	0	1
322	0	0	0	1	0	0	0	0	1
323	0	0	0	1	0	0	0	0	1
332	0	0	0	1	0	0	0	0	1
333	0	0	0	1	0	0	0	0	1
342	0	0	0	1	0	0	0	0	1
343	0	0	0	1	0	0	0	0	1
352	0	0	0	1	0	0	0	0	1
353	0	0	0	1	0	0	0	0	1
362	0	0	0	1	0	0	0	0	1
363	0	0	0	1	0	0	0	0	1
372	0	0	0	1	0	0	0	0	1
373	0	0	0	1	0	0	0	0	1
382	0	0	0	1	0	0	0	0	1
383	0	0	0	1	0	0	0	0	1
392	0	0	0	1	0	0	0	0	1
393	0	0	0	1	0	0	0	0	1
402	0	0	0	1	0	0	0	0	1
403	0	0	0	1	0	0	0	0	1
412	0	0	0	1	0	0	0	0	1
413	0	0	0	1	0	0	0	0	1
422	0	0	0	1	0	0	0	0	1
423	0	0	0	1	0	0	0	0	1

END PWAT-PARM1

PWAT-PARM2

```
<PLS > *** PWATER input info: Part 2
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARV AGWRC
```

211	0.000	300.0	1.00	125.0	0.052	0.0	0.93
212	0.000	300.0	1.00	125.0	0.052	0.0	0.93
213	0.000	300.0	1.00	125.0	0.052	0.0	0.93
214	0.000	300.0	1.00	125.0	0.052	0.0	0.93
215	0.000	300.0	1.00	125.0	0.052	0.0	0.93
216	0.000	300.0	1.00	125.0	0.052	0.0	0.93
217	0.000	300.0	1.00	125.0	0.052	0.0	0.93
218	0.000	300.0	1.00	125.0	0.052	0.0	0.93
221	0.000	300.0	1.00	48.0	0.216	0.0	0.93
222	0.000	300.0	1.00	48.0	0.216	0.0	0.93

223	0.000	300.0	1.00	48.0	0.216	0.0	0.93
224	0.000	300.0	1.00	48.0	0.216	0.0	0.93
225	0.000	300.0	1.00	48.0	0.216	0.0	0.93
226	0.000	300.0	1.00	48.0	0.216	0.0	0.93
227	0.000	300.0	1.00	48.0	0.216	0.0	0.93
228	0.000	300.0	1.00	48.0	0.216	0.0	0.93
231	0.000	300.0	1.00	45.0	0.230	0.0	0.93
232	0.000	300.0	1.00	45.0	0.230	0.0	0.93
233	0.000	300.0	1.00	45.0	0.230	0.0	0.93
234	0.000	300.0	1.00	45.0	0.230	0.0	0.93
235	0.000	300.0	1.00	45.0	0.230	0.0	0.93
236	0.000	300.0	1.00	45.0	0.230	0.0	0.93
237	0.000	300.0	1.00	45.0	0.230	0.0	0.93
238	0.000	300.0	1.00	45.0	0.230	0.0	0.93
241	0.000	300.0	1.00	89.0	0.107	0.0	0.93
242	0.000	300.0	1.00	89.0	0.107	0.0	0.93
243	0.000	300.0	1.00	89.0	0.107	0.0	0.93
244	0.000	300.0	1.00	89.0	0.107	0.0	0.93
245	0.000	300.0	1.00	89.0	0.107	0.0	0.93
246	0.000	300.0	1.00	89.0	0.107	0.0	0.93
247	0.000	300.0	1.00	89.0	0.107	0.0	0.93
248	0.000	300.0	1.00	89.0	0.107	0.0	0.93
251	0.000	300.0	1.00	143.0	0.024	0.0	0.93
252	0.000	300.0	1.00	143.0	0.024	0.0	0.93
253	0.000	300.0	1.00	143.0	0.024	0.0	0.93
254	0.000	300.0	1.00	143.0	0.024	0.0	0.93
255	0.000	300.0	1.00	143.0	0.024	0.0	0.93
256	0.000	300.0	1.00	143.0	0.024	0.0	0.93
257	0.000	300.0	1.00	143.0	0.024	0.0	0.93
258	0.000	300.0	1.00	143.0	0.024	0.0	0.93
261	0.000	300.0	1.00	143.0	0.024	0.0	0.93
262	0.000	300.0	1.00	143.0	0.024	0.0	0.93
263	0.000	300.0	1.00	143.0	0.024	0.0	0.93
264	0.000	300.0	1.00	143.0	0.024	0.0	0.93
265	0.000	300.0	1.00	143.0	0.024	0.0	0.93
266	0.000	300.0	1.00	143.0	0.024	0.0	0.93
267	0.000	300.0	1.00	143.0	0.024	0.0	0.93
268	0.000	300.0	1.00	143.0	0.024	0.0	0.93
271	0.000	300.0	1.00	122.0	0.056	0.0	0.93
272	0.000	300.0	1.00	122.0	0.056	0.0	0.93
273	0.000	300.0	1.00	122.0	0.056	0.0	0.93
274	0.000	300.0	1.00	122.0	0.056	0.0	0.93
275	0.000	300.0	1.00	122.0	0.056	0.0	0.93
276	0.000	300.0	1.00	122.0	0.056	0.0	0.93
277	0.000	300.0	1.00	122.0	0.056	0.0	0.93
278	0.000	300.0	1.00	122.0	0.056	0.0	0.93
281	0.000	300.0	1.00	150.0	0.013	0.0	0.93
282	0.000	300.0	1.00	150.0	0.013	0.0	0.93
283	0.000	300.0	1.00	150.0	0.013	0.0	0.93
284	0.000	300.0	1.00	150.0	0.013	0.0	0.93
285	0.000	300.0	1.00	150.0	0.013	0.0	0.93
286	0.000	300.0	1.00	150.0	0.013	0.0	0.93
287	0.000	300.0	1.00	150.0	0.013	0.0	0.93
288	0.000	300.0	1.00	150.0	0.013	0.0	0.93
291	0.000	300.0	1.00	84.0	0.114	0.0	0.93
292	0.000	300.0	1.00	84.0	0.114	0.0	0.93
293	0.000	300.0	1.00	84.0	0.114	0.0	0.93
294	0.000	300.0	1.00	84.0	0.114	0.0	0.93
295	0.000	300.0	1.00	84.0	0.114	0.0	0.93
296	0.000	300.0	1.00	84.0	0.114	0.0	0.93
297	0.000	300.0	1.00	84.0	0.114	0.0	0.93
298	0.000	300.0	1.00	84.0	0.114	0.0	0.93
301	0.000	300.0	1.00	96.0	0.096	0.0	0.93
302	0.000	300.0	1.00	96.0	0.096	0.0	0.93
303	0.000	300.0	1.00	96.0	0.096	0.0	0.93
304	0.000	300.0	1.00	96.0	0.096	0.0	0.93
305	0.000	300.0	1.00	96.0	0.096	0.0	0.93
306	0.000	300.0	1.00	96.0	0.096	0.0	0.93

307	0.000	300.0	1.00	96.0	0.096	0.0	0.93
308	0.000	300.0	1.00	96.0	0.096	0.0	0.93
311	0.000	320.0	1.00	108.0	0.078	0.0	0.93
312	0.000	320.0	1.00	108.0	0.078	0.0	0.93
313	0.000	320.0	1.00	108.0	0.078	0.0	0.93
314	0.000	320.0	1.00	108.0	0.078	0.0	0.93
315	0.000	320.0	1.00	108.0	0.078	0.0	0.93
316	0.000	320.0	1.00	108.0	0.078	0.0	0.93
317	0.000	320.0	1.00	108.0	0.078	0.0	0.93
318	0.000	320.0	1.00	108.0	0.078	0.0	0.93
321	0.000	320.0	1.00	108.0	0.078	0.0	0.93
322	0.000	320.0	1.00	108.0	0.078	0.0	0.93
323	0.000	320.0	1.00	108.0	0.078	0.0	0.93
324	0.000	320.0	1.00	108.0	0.078	0.0	0.93
325	0.000	320.0	1.00	108.0	0.078	0.0	0.93
326	0.000	320.0	1.00	108.0	0.078	0.0	0.93
327	0.000	320.0	1.00	108.0	0.078	0.0	0.93
328	0.000	320.0	1.00	108.0	0.078	0.0	0.93
331	0.000	320.0	1.00	131.0	0.043	0.0	0.93
332	0.000	320.0	1.00	131.0	0.043	0.0	0.93
333	0.000	320.0	1.00	131.0	0.043	0.0	0.93
334	0.000	320.0	1.00	131.0	0.043	0.0	0.93
335	0.000	320.0	1.00	131.0	0.043	0.0	0.93
336	0.000	320.0	1.00	131.0	0.043	0.0	0.93
337	0.000	320.0	1.00	131.0	0.043	0.0	0.93
338	0.000	320.0	1.00	131.0	0.043	0.0	0.93
341	0.000	320.0	1.00	97.0	0.094	0.0	0.93
342	0.000	320.0	1.00	97.0	0.094	0.0	0.93
343	0.000	320.0	1.00	97.0	0.094	0.0	0.93
344	0.000	320.0	1.00	97.0	0.094	0.0	0.93
345	0.000	320.0	1.00	97.0	0.094	0.0	0.93
346	0.000	320.0	1.00	97.0	0.094	0.0	0.93
347	0.000	320.0	1.00	97.0	0.094	0.0	0.93
348	0.000	320.0	1.00	97.0	0.094	0.0	0.93
351	0.000	320.0	1.00	58.0	0.164	0.0	0.93
352	0.000	320.0	1.00	58.0	0.164	0.0	0.93
353	0.000	320.0	1.00	58.0	0.164	0.0	0.93
354	0.000	320.0	1.00	58.0	0.164	0.0	0.93
355	0.000	320.0	1.00	58.0	0.164	0.0	0.93
356	0.000	320.0	1.00	58.0	0.164	0.0	0.93
357	0.000	320.0	1.00	58.0	0.164	0.0	0.93
358	0.000	320.0	1.00	58.0	0.164	0.0	0.93
361	0.000	320.0	1.00	59.0	0.159	0.0	0.93
362	0.000	320.0	1.00	59.0	0.159	0.0	0.93
363	0.000	320.0	1.00	59.0	0.159	0.0	0.93
364	0.000	320.0	1.00	59.0	0.159	0.0	0.93
365	0.000	320.0	1.00	59.0	0.159	0.0	0.93
366	0.000	320.0	1.00	59.0	0.159	0.0	0.93
367	0.000	320.0	1.00	59.0	0.159	0.0	0.93
368	0.000	320.0	1.00	59.0	0.159	0.0	0.93
371	0.000	320.0	1.00	38.0	0.266	0.0	0.93
372	0.000	320.0	1.00	38.0	0.266	0.0	0.93
373	0.000	320.0	1.00	38.0	0.266	0.0	0.93
374	0.000	320.0	1.00	38.0	0.266	0.0	0.93
375	0.000	320.0	1.00	38.0	0.266	0.0	0.93
376	0.000	320.0	1.00	38.0	0.266	0.0	0.93
377	0.000	320.0	1.00	38.0	0.266	0.0	0.93
378	0.000	320.0	1.00	38.0	0.266	0.0	0.93
381	0.000	320.0	1.00	78.0	0.124	0.0	0.93
382	0.000	320.0	1.00	78.0	0.124	0.0	0.93
383	0.000	320.0	1.00	78.0	0.124	0.0	0.93
384	0.000	320.0	1.00	78.0	0.124	0.0	0.93
385	0.000	320.0	1.00	78.0	0.124	0.0	0.93
386	0.000	320.0	1.00	78.0	0.124	0.0	0.93
387	0.000	320.0	1.00	78.0	0.124	0.0	0.93
388	0.000	320.0	1.00	78.0	0.124	0.0	0.93
391	0.000	320.0	1.00	80.0	0.121	0.0	0.93
392	0.000	320.0	1.00	80.0	0.121	0.0	0.93



END PWAT-PARM3

PWAT-PARM4

<PLS > PWATER input info: Part 4							***
# - #	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP	***
211	4.45	29.40	0.40	2.5	0.70	0.80	
221	4.45	29.40	0.40	2.5	0.70	0.80	
231	4.45	29.40	0.40	2.5	0.70	0.80	
241	4.45	29.40	0.40	2.5	0.70	0.80	
251	4.45	29.40	0.40	2.5	0.70	0.80	
261	4.45	29.40	0.40	2.5	0.70	0.80	
271	4.45	29.40	0.40	2.5	0.70	0.80	
281	4.45	29.40	0.40	2.5	0.70	0.80	
291	4.45	29.40	0.40	2.5	0.70	0.80	
301	4.45	29.40	0.40	2.5	0.70	0.80	
311	4.45	29.40	0.40	2.5	0.70	0.80	
321	4.45	29.40	0.40	2.5	0.70	0.80	
331	4.45	29.40	0.40	2.5	0.70	0.80	
341	4.45	29.40	0.40	2.5	0.70	0.80	
351	4.45	29.40	0.40	2.5	0.70	0.80	
361	4.45	29.40	0.40	2.5	0.70	0.80	
371	4.45	29.40	0.40	2.5	0.70	0.80	
381	4.45	29.40	0.40	2.5	0.70	0.80	
391	4.45	29.40	0.40	2.5	0.70	0.80	
401	4.45	29.40	0.40	2.5	0.70	0.80	
411	4.45	29.40	0.40	2.5	0.70	0.80	
421	4.45	29.40	0.40	2.5	0.70	0.80	
212	3.50	16.80	0.25	3.0	0.70	0.60	
222	3.50	16.80	0.25	3.0	0.70	0.60	
232	3.50	16.80	0.25	3.0	0.70	0.60	
242	3.50	16.80	0.25	3.0	0.70	0.60	
252	3.50	16.80	0.25	3.0	0.70	0.60	
262	3.50	16.80	0.25	3.0	0.70	0.60	
272	3.50	16.80	0.25	3.0	0.70	0.60	
282	3.50	16.80	0.25	3.0	0.70	0.60	
292	3.50	16.80	0.25	3.0	0.70	0.60	
302	3.50	16.80	0.25	3.0	0.70	0.60	
312	3.50	16.80	0.25	3.0	0.70	0.60	
322	3.50	16.80	0.25	3.0	0.70	0.60	
332	3.50	16.80	0.25	3.0	0.70	0.60	
342	3.50	16.80	0.25	3.0	0.70	0.60	
352	3.50	16.80	0.25	3.0	0.70	0.60	
362	3.50	16.80	0.25	3.0	0.70	0.60	
372	3.50	16.80	0.25	3.0	0.70	0.60	
382	3.50	16.80	0.25	3.0	0.70	0.60	
392	3.50	16.80	0.25	3.0	0.70	0.60	
402	3.50	16.80	0.25	3.0	0.70	0.60	
412	3.50	16.80	0.25	3.0	0.70	0.60	
422	3.50	16.80	0.25	3.0	0.70	0.60	
213	2.54	16.80	0.20	2.0	0.70	0.50	
223	2.54	16.80	0.20	2.0	0.70	0.50	
233	2.54	16.80	0.20	2.0	0.70	0.50	
243	2.54	16.80	0.20	2.0	0.70	0.50	
253	2.54	16.80	0.20	2.0	0.70	0.50	
263	2.54	16.80	0.20	2.0	0.70	0.50	
273	2.54	16.80	0.20	2.0	0.70	0.50	
283	2.54	16.80	0.20	2.0	0.70	0.50	
293	2.54	16.80	0.20	2.0	0.70	0.50	
303	2.54	16.80	0.20	2.0	0.70	0.50	
313	2.54	16.80	0.20	2.0	0.70	0.50	
323	2.54	16.80	0.20	2.0	0.70	0.50	
333	2.54	16.80	0.20	2.0	0.70	0.50	
343	2.54	16.80	0.20	2.0	0.70	0.50	
353	2.54	16.80	0.20	2.0	0.70	0.50	
363	2.54	16.80	0.20	2.0	0.70	0.50	
373	2.54	16.80	0.20	2.0	0.70	0.50	
383	2.54	16.80	0.20	2.0	0.70	0.50	
393	2.54	16.80	0.20	2.0	0.70	0.50	

403	2.54	16.80	0.20	2.0	0.70	0.50
413	2.54	16.80	0.20	2.0	0.70	0.50
423	2.54	16.80	0.20	2.0	0.70	0.50
214	0.25	2.54	0.10	1.0	0.70	0.10
224	0.25	2.54	0.10	1.0	0.70	0.10
234	0.25	2.54	0.10	1.0	0.70	0.10
244	0.25	2.54	0.10	1.0	0.70	0.10
254	0.25	2.54	0.10	1.0	0.70	0.10
264	0.25	2.54	0.10	1.0	0.70	0.10
274	0.25	2.54	0.10	1.0	0.70	0.10
284	0.25	2.54	0.10	1.0	0.70	0.10
294	0.25	2.54	0.10	1.0	0.70	0.10
304	0.25	2.54	0.10	1.0	0.70	0.10
314	0.25	2.54	0.10	1.0	0.70	0.10
324	0.25	2.54	0.10	1.0	0.70	0.10
334	0.25	2.54	0.10	1.0	0.70	0.10
344	0.25	2.54	0.10	1.0	0.70	0.10
354	0.25	2.54	0.10	1.0	0.70	0.10
364	0.25	2.54	0.10	1.0	0.70	0.10
374	0.25	2.54	0.10	1.0	0.70	0.10
384	0.25	2.54	0.10	1.0	0.70	0.10
394	0.25	2.54	0.10	1.0	0.70	0.10
404	0.25	2.54	0.10	1.0	0.70	0.10
414	0.25	2.54	0.10	1.0	0.70	0.10
424	0.25	2.54	0.10	1.0	0.70	0.10
215	1.05	6.50	0.14	1.5	0.70	0.20
225	1.05	6.50	0.14	1.5	0.70	0.20
235	1.05	6.50	0.14	1.5	0.70	0.20
245	1.05	6.50	0.14	1.5	0.70	0.20
255	1.05	6.50	0.14	1.5	0.70	0.20
265	1.05	6.50	0.14	1.5	0.70	0.20
275	1.05	6.50	0.14	1.5	0.70	0.20
285	1.05	6.50	0.14	1.5	0.70	0.20
295	1.05	6.50	0.14	1.5	0.70	0.20
305	1.05	6.50	0.14	1.5	0.70	0.20
315	1.05	6.50	0.14	1.5	0.70	0.20
325	1.05	6.50	0.14	1.5	0.70	0.20
335	1.05	6.50	0.14	1.5	0.70	0.20
345	1.05	6.50	0.14	1.5	0.70	0.20
355	1.05	6.50	0.14	1.5	0.70	0.20
365	1.05	6.50	0.14	1.5	0.70	0.20
375	1.05	6.50	0.14	1.5	0.70	0.20
385	1.05	6.50	0.14	1.5	0.70	0.20
395	1.05	6.50	0.14	1.5	0.70	0.20
405	1.05	6.50	0.14	1.5	0.70	0.20
415	1.05	6.50	0.14	1.5	0.70	0.20
425	1.05	6.50	0.14	1.5	0.70	0.20
216	1.85	10.00	0.15	1.8	0.70	0.40
226	1.85	10.00	0.15	1.8	0.70	0.40
236	1.85	10.00	0.15	1.8	0.70	0.40
246	1.85	10.00	0.15	1.8	0.70	0.40
256	1.85	10.00	0.15	1.8	0.70	0.40
266	1.85	10.00	0.15	1.8	0.70	0.40
276	1.85	10.00	0.15	1.8	0.70	0.40
286	1.85	10.00	0.15	1.8	0.70	0.40
296	1.85	10.00	0.15	1.8	0.70	0.40
306	1.85	10.00	0.15	1.8	0.70	0.40
316	1.85	10.00	0.15	1.8	0.70	0.40
326	1.85	10.00	0.15	1.8	0.70	0.40
336	1.85	10.00	0.15	1.8	0.70	0.40
346	1.85	10.00	0.15	1.8	0.70	0.40
356	1.85	10.00	0.15	1.8	0.70	0.40
366	1.85	10.00	0.15	1.8	0.70	0.40
376	1.85	10.00	0.15	1.8	0.70	0.40
386	1.85	10.00	0.15	1.8	0.70	0.40
396	1.85	10.00	0.15	1.8	0.70	0.40
406	1.85	10.00	0.15	1.8	0.70	0.40
416	1.85	10.00	0.15	1.8	0.70	0.40

426	1.85	10.00	0.15	1.8	0.70	0.40
217	0.76	3.50	0.12	1.0	0.70	0.15
227	0.76	3.50	0.12	1.0	0.70	0.15
237	0.76	3.50	0.12	1.0	0.70	0.15
247	0.76	3.50	0.12	1.0	0.70	0.15
257	0.76	3.50	0.12	1.0	0.70	0.15
267	0.76	3.50	0.12	1.0	0.70	0.15
277	0.76	3.50	0.12	1.0	0.70	0.15
287	0.76	3.50	0.12	1.0	0.70	0.15
297	0.76	3.50	0.12	1.0	0.70	0.15
307	0.76	3.50	0.12	1.0	0.70	0.15
317	0.76	3.50	0.12	1.0	0.70	0.15
327	0.76	3.50	0.12	1.0	0.70	0.15
337	0.76	3.50	0.12	1.0	0.70	0.15
347	0.76	3.50	0.12	1.0	0.70	0.15
357	0.76	3.50	0.12	1.0	0.70	0.15
367	0.76	3.50	0.12	1.0	0.70	0.15
377	0.76	3.50	0.12	1.0	0.70	0.15
387	0.76	3.50	0.12	1.0	0.70	0.15
397	0.76	3.50	0.12	1.0	0.70	0.15
407	0.76	3.50	0.12	1.0	0.70	0.15
417	0.76	3.50	0.12	1.0	0.70	0.15
427	0.76	3.50	0.12	1.0	0.70	0.15
218	2.00	10.00	0.30	2.0	0.70	0.75
228	2.00	10.00	0.30	2.0	0.70	0.75
238	2.00	10.00	0.30	2.0	0.70	0.75
248	2.00	10.00	0.30	2.0	0.70	0.75
258	2.00	10.00	0.30	2.0	0.70	0.75
268	2.00	10.00	0.30	2.0	0.70	0.75
278	2.00	10.00	0.30	2.0	0.70	0.75
288	2.00	10.00	0.30	2.0	0.70	0.75
298	2.00	10.00	0.30	2.0	0.70	0.75
308	2.00	10.00	0.30	2.0	0.70	0.75
318	2.00	10.00	0.30	2.0	0.70	0.75
328	2.00	10.00	0.30	2.0	0.70	0.75
338	2.00	10.00	0.30	2.0	0.70	0.75
348	2.00	10.00	0.30	2.0	0.70	0.75
358	2.00	10.00	0.30	2.0	0.70	0.75
368	2.00	10.00	0.30	2.0	0.70	0.75
378	2.00	10.00	0.30	2.0	0.70	0.75
388	2.00	10.00	0.30	2.0	0.70	0.75
398	2.00	10.00	0.30	2.0	0.70	0.75
408	2.00	10.00	0.30	2.0	0.70	0.75
418	2.00	10.00	0.30	2.0	0.70	0.75
428	2.00	10.00	0.30	2.0	0.70	0.75

END PWAT-PARM4

PWAT-PARM5 Defaults used \*\*\*

MON-INTERCEP

<PLS> Only required if VCSFG=1 in PWAT-PARM1 \*\*\*

# - # Interception storage capacity at start of each month \*\*\*

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
212	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
213	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
222	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
223	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
232	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
233	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
242	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
243	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
252	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
253	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
262	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
263	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
272	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
273	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
282	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50



283	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
292	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
293	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
302	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
303	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
312	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
313	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
322	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
323	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
332	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
333	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
342	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
343	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
352	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
353	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
362	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
363	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
372	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
373	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
382	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
383	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
392	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
393	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
402	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
403	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
412	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
413	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
422	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50
423	4.25	4.45	3.50	1.25	1.00	0.75	0.75	0.75	1.00	1.25	1.50	2.50

END MON-INTERCEP

MON-LZETPARM

<PLS > Only required if VLEFG=1 in PWAT-PARM1 \*\*\*

# - # Lower zone ET parameter at start of each month \*\*\*

	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
212	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
213	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
222	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
223	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
232	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
233	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
242	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
243	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
252	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
253	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
262	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
263	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
272	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
273	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
282	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
283	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
292	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
293	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
302	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
303	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
312	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
313	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
322	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
323	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
332	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
333	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
342	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
343	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
352	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
353	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
362	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40
363	0.55	0.60	0.50	0.20	0.20	0.20	0.20	0.20	0.25	0.30	0.35	0.40
372	0.60	0.70	0.60	0.20	0.20	0.15	0.15	0.15	0.20	0.25	0.30	0.40

```

373      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
382      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
383      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
392      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
393      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
402      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
403      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
412      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
413      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
422      0.60 0.70 0.60 0.20 0.20 0.15 0.15 0.15 0.20 0.25 0.30 0.40
423      0.55 0.60 0.50 0.20 0.20 0.20 0.20 0.20 0.25 0.30 0.35 0.40
END MON-LZETPARM

```

#### PWAT-STATE1

```

<PLS > *** Initial conditions at start of simulation
# - # *** CEPS      SURS      UZS      IFWS      LZS      AGWS      GWVS
11  248      0.00      0.0      11.94      0.0      210.0      25.4      00.0
251 428      0.00      0.0      11.94      0.0      210.0      25.4      00.0
END PWAT-STATE1

```

#### END PERLND

#### IMPLND

##### ACTIVITY

```

<ILS > Active Sections      ***
# - # ATMP SNOW IWAT  SLD  IWG IQAL ***
214 427      1
END ACTIVITY

```

##### PRINT-INFO

```

<ILS > Print-flags      ***
# - # ATMP SNOW IWAT  SLD  IWG IQAL PIVL  PYR ***
214 427      4      09
END PRINT-INFO

```

##### GEN-INFO

```

<ILS > <-----Name----->  Unit-systems  Printer ***
# - #      t-series  Engl Metr ***
      in  out      ***
214      HD Urban, Indust.      2      2      0      64
215      MD Residential      2      2      0      64
217      HD Residential      2      2      0      64
224      HD Urban, Indust.      2      2      0      64
225      MD Residential      2      2      0      64
227      HD Residential      2      2      0      64
234      HD Urban, Indust.      2      2      0      64
235      MD Residential      2      2      0      64
237      HD Residential      2      2      0      64
244      HD Urban, Indust.      2      2      0      64
245      MD Residential      2      2      0      64
247      HD Residential      2      2      0      64
254      HD Urban, Indust.      2      2      0      64
255      MD Residential      2      2      0      64
257      HD Residential      2      2      0      64
264      HD Urban, Indust.      2      2      0      64
265      MD Residential      2      2      0      64
267      HD Residential      2      2      0      64
274      HD Urban, Indust.      2      2      0      64
275      MD Residential      2      2      0      64
277      HD Residential      2      2      0      64
284      HD Urban, Indust.      2      2      0      64
285      MD Residential      2      2      0      64
287      HD Residential      2      2      0      64
294      HD Urban, Indust.      2      2      0      64
295      MD Residential      2      2      0      64
297      HD Residential      2      2      0      64
304      HD Urban, Indust.      2      2      0      64
305      MD Residential      2      2      0      64

```



307	HD Residential	2	2	0	64
314	HD Urban, Indust.	2	2	0	64
315	MD Residential	2	2	0	64
317	HD Residential	2	2	0	64
324	HD Urban, Indust.	2	2	0	64
325	MD Residential	2	2	0	64
327	HD Residential	2	2	0	64
334	HD Urban, Indust.	2	2	0	64
335	MD Residential	2	2	0	64
337	HD Residential	2	2	0	64
344	HD Urban, Indust.	2	2	0	64
345	MD Residential	2	2	0	64
347	HD Residential	2	2	0	64
354	HD Urban, Indust.	2	2	0	64
355	MD Residential	2	2	0	64
357	HD Residential	2	2	0	64
364	HD Urban, Indust.	2	2	0	64
365	MD Residential	2	2	0	64
367	HD Residential	2	2	0	64
374	HD Urban, Indust.	2	2	0	64
375	MD Residential	2	2	0	64
377	HD Residential	2	2	0	64
384	HD Urban, Indust.	2	2	0	64
385	MD Residential	2	2	0	64
387	HD Residential	2	2	0	64
394	HD Urban, Indust.	2	2	0	64
395	MD Residential	2	2	0	64
397	HD Residential	2	2	0	64
404	HD Urban, Indust.	2	2	0	64
405	MD Residential	2	2	0	64
407	HD Residential	2	2	0	64
414	HD Urban, Indust.	2	2	0	64
415	MD Residential	2	2	0	64
417	HD Residential	2	2	0	64
424	HD Urban, Indust.	2	2	0	64
425	MD Residential	2	2	0	64
427	HD Residential	2	2	0	64

END GEN-INFO

\*\*\* Section IWATER \*\*\*

IWAT-PARM1 defaults used \*\*\*

IWAT-PARM2

<ILS >		***			
#	- #	LSUR	SLSUR	NSUR	RETSC ***
214		58.0	0.052	0.050	2.54
215		58.0	0.052	0.100	2.54
217		58.0	0.052	0.075	2.54
224		15.0	0.150	0.050	2.54
225		15.0	0.150	0.100	2.54
227		15.0	0.150	0.075	2.54
234		15.0	0.150	0.050	2.54
235		15.0	0.150	0.100	2.54
237		15.0	0.150	0.075	2.54
244		34.0	0.107	0.050	2.54
245		34.0	0.107	0.100	2.54
247		34.0	0.107	0.075	2.54
254		70.0	0.024	0.050	2.54
255		70.0	0.024	0.100	2.54
257		70.0	0.024	0.075	2.54
264		70.0	0.024	0.050	2.54
265		70.0	0.024	0.100	2.54
267		70.0	0.024	0.075	2.54
274		56.0	0.056	0.050	2.54
275		56.0	0.056	0.100	2.54
277		56.0	0.056	0.075	2.54
284		75.0	0.013	0.050	2.14
285		75.0	0.013	0.100	2.54



```

287      75.0    0.013    0.075    2.54
294      31.0    0.114    0.050    2.54
295      31.0    0.114    0.100    2.54
297      31.0    0.114    0.075    2.54
304      39.0    0.096    0.050    2.54
305      39.0    0.096    0.100    2.54
307      39.0    0.096    0.075    2.54
314      46.0    0.078    0.050    2.54
315      46.0    0.078    0.100    2.54
317      46.0    0.078    0.075    2.54
324      46.0    0.078    0.050    2.54
325      46.0    0.078    0.100    2.54
327      46.0    0.078    0.075    2.54
334      62.0    0.043    0.050    2.54
335      62.0    0.043    0.100    2.54
337      62.0    0.043    0.075    2.54
344      39.0    0.094    0.050    2.54
345      39.0    0.094    0.100    2.54
347      39.0    0.094    0.075    2.54
354      15.0    0.150    0.050    2.54
355      15.0    0.150    0.100    2.54
357      15.0    0.150    0.075    2.54
364      15.0    0.150    0.050    2.54
365      15.0    0.150    0.100    2.54
367      15.0    0.150    0.075    2.54
374      15.0    0.150    0.050    2.54
375      15.0    0.150    0.100    2.54
377      15.0    0.150    0.075    2.54
384      26.0    0.124    0.050    2.54
385      26.0    0.124    0.100    2.54
387      26.0    0.124    0.075    2.54
394      28.0    0.121    0.050    2.54
395      28.0    0.121    0.100    2.54
397      28.0    0.121    0.075    2.54
404      15.0    0.150    0.050    2.54
405      15.0    0.150    0.100    2.54
407      15.0    0.150    0.075    2.54
414      35.0    0.104    0.050    2.54
415      35.0    0.104    0.100    2.54
417      35.0    0.104    0.075    2.54
424      25.0    0.128    0.050    2.54
425      25.0    0.128    0.100    2.54
427      25.0    0.128    0.075    2.54
END IWAT-PARM2

IWAT-PARM3 *** defaults used

IWAT-STATE1
  <ILS > IWATER state variables ***
  # - #      RETS      SURS      ***
214 427      0.00      0.00
END IWAT-STATE1

END IMPLND

RCHRES

ACTIVITY
  RCHRES Active Sections (1=Active; 0=Inactive) ***
  # - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG ***
21 42 1
END ACTIVITY

PRINT-INFO
  Print-flags
  # - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PH PYR ***
1 42 5 6 6 6 6 6 6 6 6 6 09
END PRINT-INFO

```

```

287      75.0    0.013    0.075    2.54
294      31.0    0.114    0.050    2.54
295      31.0    0.114    0.100    2.54
297      31.0    0.114    0.075    2.54
304      39.0    0.096    0.050    2.54
305      39.0    0.096    0.100    2.54
307      39.0    0.096    0.075    2.54
314      46.0    0.078    0.050    2.54
315      46.0    0.078    0.100    2.54
317      46.0    0.078    0.075    2.54
324      46.0    0.078    0.050    2.54
325      46.0    0.078    0.100    2.54
327      46.0    0.078    0.075    2.54
334      62.0    0.043    0.050    2.54
335      62.0    0.043    0.100    2.54
337      62.0    0.043    0.075    2.54
344      39.0    0.094    0.050    2.54
345      39.0    0.094    0.100    2.54
347      39.0    0.094    0.075    2.54
354      15.0    0.150    0.050    2.54
355      15.0    0.150    0.100    2.54
357      15.0    0.150    0.075    2.54
364      15.0    0.150    0.050    2.54
365      15.0    0.150    0.100    2.54
367      15.0    0.150    0.075    2.54
374      15.0    0.150    0.050    2.54
375      15.0    0.150    0.100    2.54
377      15.0    0.150    0.075    2.54
384      26.0    0.124    0.050    2.54
385      26.0    0.124    0.100    2.54
387      26.0    0.124    0.075    2.54
394      28.0    0.121    0.050    2.54
395      28.0    0.121    0.100    2.54
397      28.0    0.121    0.075    2.54
404      15.0    0.150    0.050    2.54
405      15.0    0.150    0.100    2.54
407      15.0    0.150    0.075    2.54
414      35.0    0.104    0.050    2.54
415      35.0    0.104    0.100    2.54
417      35.0    0.104    0.075    2.54
424      25.0    0.128    0.050    2.54
425      25.0    0.128    0.100    2.54
427      25.0    0.128    0.075    2.54
END IWAT-PARM2

IWAT-PARM3 *** defaults used

IWAT-STATE1
<ILS > IWATER state variables ***
# - #      RETS      SURS      ***
214 427      0.00      0.00
END IWAT-STATE1

END IMPLND

RCHRES

ACTIVITY
RCHRES Active Sections (1=Active; 0=Inactive) ***
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG ***
21 42      1
END ACTIVITY

PRINT-INFO
Print-flags ***
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PH PYR ***
1 42 5 6 6 6 6 6 6 6 6 6 09
END PRINT-INFO

```

GEN-INFO

RCHRES<-----Name----->Nexit		Unit Systems		Printer		***	
#	- #	User	t-series	Engl	Metr	LKFG	***
		in out				***	
21	Reach 11	1	2 2	0	62	0	
22	Reach 12	1	2 2	0	62	0	
23	Reach 13	1	2 2	0	62	0	
24	Reach 14	1	2 2	0	62	0	
25	Reach 15	1	2 2	0	62	0	
26	Reach 16	1	2 2	0	62	0	
27	Reach 17	1	2 2	0	62	0	
28	Reach 27	1	2 2	0	62	0	
29	Reach 28	1	2 2	0	62	0	
30	Reach 29	1	2 2	0	62	0	
31	Reach 30	1	2 2	0	62	0	
32	Reach 31	1	2 2	0	62	0	
33	Reach 32	1	2 2	0	62	0	
34	Reach 33	1	2 2	0	62	0	
35	Reach 35	1	2 2	0	62	0	
36	Reach 36	1	2 2	0	62	0	
37	Reach 37	1	2 2	0	62	0	
38	Reach 38	1	2 2	0	62	0	
39	Reach 39	1	2 2	0	62	0	
40	Reach 40	1	2 2	0	62	0	
41	Reach 41	1	2 2	0	62	0	
42	Reach 42	1	2 2	0	62	0	

END GEN-INFO

HYDR-PARM1

RCHRES Flags for HYDR section										***											
#	- #	VC	A1	A2	A3	ODFVFG for each					ODGTFG for each					*** FUNCT for each					
		FG	FG	FG	FG	possible exit					possible exit					*** possible exit					
		1	2	3		1	2	3	4	5	1	2	3	4	5	***	1	2	3	4	5
21	42	0	1	1	1	4															

END HYDR-PARM1

HYDR-PARM2

#	- #	FTABNO	LEN	DELTH	STCOR	KS	DB50	***
*** The values of DB50 are needed by the Colby and Toffaleti sediment transport methods. We will use the default (6.35mm)								
21		21	2.888	1.1		0.5		
22		22	5.663	235.1		0.5		
23		23	4.571	234.6		0.5		
24		24	10.695	81.3		0.5		
25		25	3.351	6.6		0.5		
26		26	6.437	122.3		0.5		
27		27	2.618	6.0		0.5		
28		28	1.832	1.5		0.5		
29		29	6.054	150.7		0.5		
30		30	5.767	30.1		0.5		
31		31	8.237	128.0		0.5		
32		32	11.891	195.5		0.5		
33		33	4.187	30.4		0.5		
34		34	18.140	65.4		0.5		
35		35	7.510	42.6		0.5		
36		36	4.208	9.1		0.5		
37		37	12.403	40.1		0.5		
38		38	11.583	184.6		0.5		
39		39	11.460	210.5		0.5		
40		40	10.972	71.0		0.5		
41		41	6.120	13.1		0.5		
42		42	15.075	56.4		0.5		

END HYDR-PARM2



```

HYDR-INIT
  Reaches are assumed initially empty ***
RCHRES      VOL CAT Initial value of COLIND *** initial value of OUTDGT
  # - #      Mm3      for each possible exit *** for each possible exit
                  EX1 EX2 EX3 EX4 EX5 *** EX1 EX2 EX3 EX4 EX5
    21  42      0.0
END HYDR-INIT

END RCHRES

PLTGEN
PLOTINFO
  # - # FILE  NPT  NMN LABL  PYR PIVL ***
    1    92      2      1
END PLOTINFO

GEN-LABELS
  # - #<-----Title-----> *** <-----Y axis----->
    1  OBSERVED ECOLI Count
END GEN-LABELS

SCALING
  # - #      YMIN      YMAX      IVLIN      THRESH ***
    1      0.      100.      20.
END SCALING

CURV-DATA      (first curve)
  <-Curve label--> Line Intg Col Tran ***
  # - #      type eqv code code ***
    1  OBS ECOLI      1  1 AVER
END CURV-DATA

CURV-DATA      (second curve)
  <-Curve label--> Line Intg Col Tran ***
  # - #      type eqv code code ***
    1  SIM ECOLI      2  2 AVER
END CURV-DATA

END PLTGEN

*** FTABLES

FTABLES

FTABLE      21
ROWS COLS ***
  20  4
    DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
    ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
    0.000      0.000      0.000      0.000      0.00
    0.250      1.943      0.002      0.063      646.89
    0.500      3.886      0.010      0.397      407.42
    0.750      5.829      0.022      1.172      310.88
    1.100      6.092      0.043      3.451      206.31
    1.450      6.356      0.065      6.624      162.31
    1.800      6.619      0.087      10.593     137.22
    2.150      6.883      0.111      15.305     120.70
    2.500      7.146      0.135      20.728     108.86
    2.850      7.410      0.161      26.842     99.88
    3.550      7.936      0.215      41.096     87.02
    4.250      8.463      0.272      58.024     78.12
    4.950      8.990      0.333      77.627     71.51
    6.350      18.249     0.524      141.016    61.90
    7.750      27.508     0.844      252.741    55.66
    9.150      36.767     1.294      434.810    49.60
    10.550     46.025     1.873      705.454    44.26
    11.950     55.284     2.583     1080.959    39.82
    13.350     64.543     3.421     1576.327    36.18

```

14.750	73.802	4.390	2205.617	33.17
END FTABLE 21				
FTABLE 22				
ROWS COLS ***				
20	4			
DEPTH	AREA	VOLUME	DISCH	FLO-THRU ***
( m)	( HA)	(Mm3)	(CMS)	(MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.311	0.002	0.319	85.57
0.500	2.622	0.007	2.027	53.89
0.750	3.933	0.015	5.978	41.12
0.942	4.194	0.023	11.481	32.72
1.133	4.455	0.031	18.407	27.91
1.325	4.717	0.040	26.691	24.74
1.517	4.978	0.049	36.306	22.45
1.708	5.239	0.059	47.243	20.71
1.900	5.500	0.069	59.507	19.32
2.283	6.023	0.091	88.069	17.24
2.667	6.545	0.115	122.142	15.72
3.050	7.068	0.141	161.921	14.54
3.817	15.512	0.228	292.643	12.97
4.583	23.957	0.379	510.020	12.39
5.350	32.401	0.595	850.764	11.66
6.117	40.846	0.876	1345.325	10.85
6.883	49.290	1.221	2020.939	10.07
7.650	57.735	1.632	2902.726	9.37
8.417	66.179	2.107	4014.241	8.75
END FTABLE 22				
FTABLE 23				
ROWS COLS ***				
20	4			
DEPTH	AREA	VOLUME	DISCH	FLO-THRU ***
( m)	( HA)	(Mm3)	(CMS)	(MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.104	0.001	0.417	55.12
0.500	2.208	0.006	2.650	34.71
0.750	3.312	0.012	7.815	26.49
0.950	3.527	0.019	15.390	20.86
1.150	3.741	0.027	24.975	17.70
1.350	3.956	0.034	36.470	15.64
1.550	4.171	0.042	49.824	14.17
1.750	4.385	0.051	65.022	13.05
1.950	4.600	0.060	82.064	12.16
2.350	5.029	0.079	121.738	10.84
2.750	5.459	0.100	169.030	9.87
3.150	5.888	0.123	224.190	9.13
3.950	11.519	0.192	397.096	8.08
4.750	17.150	0.307	654.950	7.82
5.550	22.782	0.467	1027.060	7.58
6.350	28.413	0.672	1537.829	7.28
7.150	34.044	0.921	2209.170	6.95
7.950	39.675	1.216	3061.356	6.62
8.750	45.307	1.556	4113.454	6.31
END FTABLE 23				
FTABLE 24				
ROWS COLS ***				
20	4			
DEPTH	AREA	VOLUME	DISCH	FLO-THRU ***
( m)	( HA)	(Mm3)	(CMS)	(MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	2.675	0.003	0.168	331.88
0.500	5.350	0.013	1.066	209.02
0.750	8.025	0.030	3.145	159.49
0.958	8.560	0.047	6.344	124.45
1.167	9.095	0.066	10.423	105.15
1.375	9.630	0.085	15.336	92.67

1.583	10.165	0.106	21.062	83.79
1.792	10.700	0.128	27.595	77.08
2.000	11.235	0.150	34.937	71.78
2.417	12.305	0.200	52.071	63.86
2.833	13.375	0.253	72.551	58.12
3.250	14.445	0.311	96.492	53.71
4.083	67.679	0.653	195.269	55.75
4.917	120.913	1.439	433.736	55.29
5.750	174.146	2.668	887.714	50.10
6.583	227.380	4.341	1619.854	44.67
7.417	280.614	6.458	2685.990	40.07
8.250	333.848	9.018	4137.443	36.33
9.083	387.082	12.022	6022.229	33.27

END FTABLE 24

FTABLE 25  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.417	0.002	0.114	258.64
0.500	2.833	0.007	0.725	162.90
0.750	4.250	0.016	2.137	124.30
1.050	4.403	0.029	5.551	86.83
1.350	4.556	0.042	10.112	69.81
1.650	4.709	0.056	15.676	59.81
1.950	4.862	0.071	22.157	53.11
2.250	5.015	0.085	29.499	48.26
2.550	5.168	0.101	37.662	44.56
3.150	5.474	0.133	56.351	39.23
3.750	5.780	0.166	78.093	35.51
4.350	6.086	0.202	102.828	32.74
5.550	20.503	0.362	191.922	31.39
6.750	34.920	0.694	381.171	30.35
7.950	49.337	1.200	722.790	27.66
9.150	63.754	1.878	1259.939	24.84
10.350	78.171	2.730	2031.092	22.40
11.550	92.588	3.754	3071.630	20.37
12.750	107.005	4.952	4414.662	18.69

END FTABLE 25

FTABLE 26  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	0.832	0.001	0.153	113.19
0.500	1.664	0.004	0.973	71.29
0.750	2.496	0.009	2.868	54.40
0.883	2.805	0.013	4.503	47.72
1.017	3.115	0.017	6.529	42.99
1.150	3.424	0.021	8.969	39.40
1.283	3.733	0.026	11.844	36.55
1.417	4.043	0.031	15.179	34.21
1.550	4.352	0.037	18.996	32.24
1.817	4.971	0.049	28.169	29.10
2.083	5.589	0.063	39.539	26.67
2.350	6.208	0.079	53.279	24.71
2.883	14.230	0.133	101.859	21.84
3.417	22.252	0.231	187.417	20.52
3.950	30.274	0.371	326.191	18.95
4.483	38.296	0.554	531.652	17.36
5.017	46.318	0.779	815.850	15.92
5.550	54.340	1.048	1189.894	14.68
6.083	62.362	1.359	1664.203	13.61

END FTABLE 26



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FTABLE      27
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.167      0.003      0.250      180.75
0.500      4.333      0.011      1.586      113.84
0.750      6.500      0.024      4.677      86.86
1.075      6.717      0.046      13.044     58.58
1.400      6.933      0.068      24.524     46.24
1.725      7.150      0.091      38.767     39.09
2.050      7.367      0.115      55.566     34.35
2.375      7.583      0.139      74.788     30.93
2.700      7.800      0.164      96.338     28.34
3.350      8.233      0.216     146.178     24.62
4.000      8.667      0.271     204.760     22.04
4.650      9.100      0.329     271.931     20.14
5.950     20.385      0.520     476.605     18.19
7.250     31.671      0.859     814.956     17.56
8.550     42.956      1.344    1344.642     16.65
9.850     54.242      1.975    2113.438     15.58
11.150     65.527      2.754    3164.052     14.51
12.450     76.813      3.679    4535.846     13.52
13.750     88.098      4.751    6265.725     12.64
END FTABLE 27
  
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FTABLE      28
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      0.810      0.001      0.101     167.07
0.500      1.620      0.004      0.641     105.23
0.750      2.430      0.009      1.892      80.29
1.008      2.572      0.016      4.419      58.74
1.267      2.715      0.022      7.768      48.07
1.525      2.857      0.030     11.881      41.53
1.783      3.000      0.037     16.727      37.03
2.042      3.142      0.045     22.291      33.72
2.300      3.285      0.053     28.568      31.16
2.817      3.570      0.071     43.259      27.40
3.333      3.855      0.090     60.836      24.74
3.850      4.140      0.111     81.365      22.73
4.883     11.132      0.190    149.450      21.17
5.917     18.125      0.341    270.170      21.04
6.950     25.117      0.564    467.477      20.12
7.983     32.110      0.860    761.213      18.83
9.017     39.102      1.228   1169.106      17.51
10.050     46.095      1.668   1707.491      16.28
11.083     53.087      2.181   2391.672      15.20
END FTABLE 28
  
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FTABLE      29
ROWS COLS ***
20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      1.403      0.002      0.247     118.32
0.500      2.806      0.007      1.569      74.52
0.750      4.209      0.016      4.626      56.86
0.917      4.494      0.023      8.248      46.55
1.083      4.778      0.031     12.728      40.28
1.250      5.063      0.039     18.043      35.99
  
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1.417	5.348	0.048	24.183	32.83
1.583	5.632	0.057	31.151	30.38
1.750	5.917	0.066	38.954	28.42
2.083	6.486	0.087	57.110	25.41
2.417	7.056	0.110	78.768	23.20
2.750	7.625	0.134	104.064	21.48
3.417	25.306	0.244	196.042	20.73
4.083	42.987	0.472	382.328	20.56
4.750	60.668	0.817	710.503	19.17
5.417	78.350	1.280	1219.921	17.49
6.083	96.031	1.862	1945.687	15.95
6.750	113.712	2.561	2920.096	14.62
7.417	131.393	3.378	4173.383	13.49

END FTABLE 29

FTABLE 30  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	2.397	0.003	0.154	323.30
0.500	4.795	0.012	0.981	203.62
0.750	7.192	0.027	2.893	155.37
1.117	7.390	0.054	8.746	102.34
1.483	7.588	0.081	16.738	80.82
1.850	7.786	0.109	26.508	68.75
2.217	7.985	0.138	37.837	60.90
2.583	8.183	0.168	50.583	55.32
2.950	8.381	0.198	64.643	51.12
3.683	8.777	0.261	96.428	45.14
4.417	9.174	0.327	132.799	41.04
5.150	9.570	0.396	173.530	38.01
6.617	30.192	0.687	321.759	35.60
8.083	50.814	1.281	639.572	33.39
9.550	71.437	2.178	1216.028	29.85
11.017	92.059	3.377	2124.724	26.49
12.483	112.681	4.878	3431.263	23.70
13.950	133.303	6.682	5195.961	21.43
15.417	153.926	8.789	7475.262	19.59

END FTABLE 30

FTABLE 31  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN) ***
0.000	0.000	0.000	0.000	0.00
0.250	1.039	0.001	0.100	215.89
0.500	2.077	0.005	0.637	135.97
0.750	3.116	0.012	1.877	103.75
0.892	3.403	0.016	3.050	89.09
1.033	3.690	0.021	4.481	79.33
1.175	3.977	0.027	6.174	72.23
1.317	4.264	0.033	8.135	66.78
1.458	4.551	0.039	10.373	62.40
1.600	4.838	0.045	12.896	58.79
1.883	5.412	0.060	18.833	53.11
2.167	5.986	0.076	26.022	48.78
2.450	6.560	0.094	34.539	45.33
3.017	15.947	0.158	65.055	40.40
3.583	25.334	0.275	122.029	37.51
4.150	34.722	0.445	217.970	34.01
4.717	44.109	0.668	363.235	30.66
5.283	53.496	0.945	567.075	27.77
5.850	62.883	1.274	838.006	25.35
6.417	72.270	1.657	1184.010	23.33

END FTABLE 31

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FTABLE      32
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.063      0.003      0.169      254.44
0.500      4.125      0.010      1.073      160.25
0.750      6.188      0.023      3.163      122.28
0.917      6.644      0.034      5.604      100.82
1.083      7.100      0.045      8.617      87.72
1.250      7.556      0.058      12.189     78.71
1.417      8.013      0.071      16.318     72.05
1.583      8.469      0.084      21.005     66.87
1.750      8.925      0.099      26.260     62.69
2.083      9.837      0.130      38.509     56.28
2.417      10.750     0.164      53.161     51.53
2.750      11.662     0.202      70.320     47.81
3.417      29.041     0.337     131.058     42.90
4.083      46.419     0.589     245.617     39.96
4.750      63.798     0.956     439.822     36.24
5.417      81.176     1.440     735.051     32.64
6.083      98.555     2.039    1150.387     29.54
6.750     115.934     2.754    1703.400     26.94
7.417     133.312     3.584    2410.546     24.78
END FTABLE 32
  
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FTABLE      33
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      0.896      0.001      0.124     150.87
0.500      1.792      0.004      0.786      95.02
0.750      2.688      0.010      2.317      72.50
0.925      2.814      0.015      4.246      58.47
1.100      2.940      0.020      6.613      50.23
1.275      3.066      0.025      9.387      44.71
1.450      3.192      0.031     12.551      40.71
1.625      3.318      0.036     16.091      37.66
1.800      3.444      0.042     20.003      35.22
2.150      3.696      0.055     28.927      31.56
2.500      3.948      0.068     39.317      28.89
2.850      4.200      0.082     51.190      26.83
3.550     13.069      0.143     94.036      25.32
4.250     21.938      0.265    182.194      24.28
4.950     30.806      0.450    338.987      22.12
5.650     39.675      0.697     583.692      19.89
6.350     48.544      1.005     933.499      17.95
7.050     57.413      1.376    1404.213      16.34
7.750     66.281      1.809    2010.631      15.00
END FTABLE 33
  
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FTABLE      34
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      (  HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      8.447      0.011      0.145    1209.88
0.500     16.893      0.042      0.924     762.00
0.750     25.340      0.095      2.724     581.44
1.050     26.999      0.174      7.071     409.03
1.350     28.658      0.257     12.991     329.74
1.650     30.318      0.345     20.366     282.72
  
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1.950	31.977	0.439	29.141	251.04
2.250	33.636	0.537	39.288	227.95
2.550	35.295	0.641	50.802	210.21
3.150	38.613	0.862	77.951	184.40
3.750	41.932	1.104	110.688	166.25
4.350	45.250	1.366	149.180	152.57
5.550	84.105	2.142	271.627	131.42
6.750	122.960	3.384	465.325	121.21
7.950	161.815	5.093	757.699	112.02
9.150	200.670	7.268	1171.545	103.39
10.350	239.525	9.909	1727.296	95.61
11.550	278.381	13.016	2443.824	88.77
12.750	317.236	16.590	3338.857	82.81

END FTABLE 34

FTABLE 35  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	*** ***
0.000	0.000	0.000	0.000	0.00	
0.250	3.900	0.005	0.222	366.26	
0.500	7.800	0.020	1.409	230.68	
0.750	11.700	0.044	4.154	176.02	
1.013	13.050	0.076	9.704	131.15	
1.275	14.400	0.112	17.272	108.45	
1.538	15.750	0.152	26.856	94.31	
1.800	17.100	0.195	38.496	84.46	
2.062	18.450	0.242	52.256	77.10	
2.325	19.800	0.292	68.209	71.33	
2.850	22.500	0.403	107.011	62.76	
3.375	25.200	0.528	155.562	56.59	
3.900	27.900	0.668	214.537	51.86	
4.950	42.403	1.037	404.482	42.72	
6.000	56.906	1.558	683.210	38.01	
7.050	71.408	2.232	1076.674	34.55	
8.100	85.911	3.058	1606.642	31.72	
9.150	100.414	4.036	2292.781	29.34	
10.200	114.917	5.166	3153.366	27.31	
11.250	129.419	6.449	4205.640	25.56	

END FTABLE 35

FTABLE 36  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	*** ***
0.000	0.000	0.000	0.000	0.00	
0.250	1.932	0.002	0.112	360.61	
0.500	3.864	0.010	0.709	227.12	
0.750	5.796	0.022	2.090	173.30	
1.008	6.188	0.037	4.876	127.20	
1.267	6.580	0.054	8.584	104.27	
1.525	6.972	0.071	13.160	90.19	
1.783	7.364	0.090	18.580	80.49	
2.042	7.756	0.109	24.835	73.32	
2.300	8.148	0.130	31.927	67.76	
2.817	8.932	0.174	48.643	59.59	
3.333	9.716	0.222	68.817	53.79	
3.850	10.500	0.274	92.565	49.39	
4.883	23.732	0.451	172.215	43.67	
5.917	36.963	0.765	313.668	40.64	
6.950	50.195	1.215	544.859	37.17	
7.983	63.427	1.802	888.939	33.79	
9.017	76.659	2.526	1366.597	30.81	
10.050	89.890	3.386	1996.897	28.26	
11.083	103.122	4.384	2797.709	26.11	

END FTABLE 36

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FTABLE      37
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      7.440      0.009      0.177      874.83
0.500      14.880      0.037      1.125      550.98
0.750      22.320      0.084      3.318      420.42
1.017      23.818      0.145      7.931      305.15
1.283      25.317      0.211      14.120      248.74
1.550      26.815      0.280      21.791      214.33
1.817      28.313      0.354      30.906      190.76
2.083      29.812      0.431      41.450      173.40
2.350      31.310      0.513      53.423      159.96
2.883      34.307      0.688      81.706      140.28
3.417      37.303      0.879      115.901      126.35
3.950      40.300      1.086      156.209      115.83
5.017      73.785      1.694      285.837      98.78
6.083      107.270      2.660      498.366      88.95
7.150      140.756      3.983      827.747      80.19
8.217      174.241      5.662      1302.168      72.48
9.283      207.726      7.700      1946.864      65.91
10.350      241.211      10.094      2785.120      60.40
11.417      274.697      12.845      3838.789      55.77
END FTABLE 37

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FTABLE      38
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.088      0.003      0.173      251.47
0.500      4.176      0.010      1.099      158.38
0.750      6.264      0.023      3.240      120.85
0.892      6.602      0.033      5.340      101.76
1.033      6.941      0.042      7.858      89.50
1.175      7.279      0.052      10.777      80.84
1.317      7.617      0.063      14.088      74.32
1.458      7.956      0.074      17.787      69.20
1.600      8.294      0.085      21.875      65.04
1.883      8.971      0.110      31.221      58.62
2.167      9.647      0.136      42.154      53.85
2.450      10.324      0.164      54.714      50.11
3.017      21.298      0.254      95.012      44.57
3.583      32.272      0.406      160.327      42.19
4.150      43.246      0.620      261.223      39.55
4.717      54.219      0.896      406.455      36.74
5.283      65.193      1.234      603.851      34.07
5.850      76.167      1.635      860.628      31.66
6.417      87.141      2.098      1183.554      29.54
END FTABLE 38

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FTABLE      39
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.415      0.003      0.194      259.85
0.500      4.830      0.012      1.230      163.66
0.750      7.245      0.027      3.626      124.88
0.900      7.600      0.038      6.158      103.66
1.050      7.954      0.050      9.213      90.39
1.200      8.309      0.062      12.765      81.16

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FTABLE      37
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      7.440      0.009      0.177      874.83
0.500      14.880      0.037      1.125      550.98
0.750      22.320      0.084      3.318      420.42
1.017      23.818      0.145      7.931      305.15
1.283      25.317      0.211      14.120      248.74
1.550      26.815      0.280      21.791      214.33
1.817      28.313      0.354      30.906      190.76
2.083      29.812      0.431      41.450      173.40
2.350      31.310      0.513      53.423      159.96
2.883      34.307      0.688      81.706      140.28
3.417      37.303      0.879      115.901      126.35
3.950      40.300      1.086      156.209      115.83
5.017      73.785      1.694      285.837      98.78
6.083      107.270      2.660      498.366      88.95
7.150      140.756      3.983      827.747      80.19
8.217      174.241      5.662      1302.168      72.48
9.283      207.726      7.700      1946.864      65.91
10.350      241.211      10.094      2785.120      60.40
11.417      274.697      12.845      3838.789      55.77
END FTABLE 37

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FTABLE      38
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.088      0.003      0.173      251.47
0.500      4.176      0.010      1.099      158.38
0.750      6.264      0.023      3.240      120.85
0.892      6.602      0.033      5.340      101.76
1.033      6.941      0.042      7.858      89.50
1.175      7.279      0.052      10.777      80.84
1.317      7.617      0.063      14.088      74.32
1.458      7.956      0.074      17.787      69.20
1.600      8.294      0.085      21.875      65.04
1.883      8.971      0.110      31.221      58.62
2.167      9.647      0.136      42.154      53.85
2.450      10.324      0.164      54.714      50.11
3.017      21.298      0.254      95.012      44.57
3.583      32.272      0.406      160.327      42.19
4.150      43.246      0.620      261.223      39.55
4.717      54.219      0.896      406.455      36.74
5.283      65.193      1.234      603.851      34.07
5.850      76.167      1.635      860.628      31.66
6.417      87.141      2.098      1183.554      29.54
END FTABLE 38

```

```

FTABLE      39
ROWS COLS ***
  20      4
  DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
  ( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      2.415      0.003      0.194      259.85
0.500      4.830      0.012      1.230      163.66
0.750      7.245      0.027      3.626      124.88
0.900      7.600      0.038      6.158      103.66
1.050      7.954      0.050      9.213      90.39
1.200      8.309      0.062      12.765      81.16

```



1.350	8.663	0.075	16.799	74.30
1.500	9.018	0.088	21.305	68.96
1.650	9.372	0.102	26.280	64.65
1.950	10.082	0.131	37.634	58.07
2.250	10.791	0.162	50.874	53.22
2.550	11.500	0.196	66.032	49.44
3.150	21.845	0.296	113.850	43.32
3.750	32.190	0.458	190.148	40.15
4.350	42.534	0.682	306.731	37.07
4.950	52.879	0.968	473.386	34.10
5.550	63.224	1.317	698.864	31.40
6.150	73.569	1.727	991.237	29.04
6.750	83.914	2.200	1358.074	26.99

END FTABLE 39

FTABLE 40  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	***
0.000	0.000	0.000	0.000	0.00	
0.250	2.603	0.003	0.117	463.41	
0.500	5.207	0.013	0.743	291.86	
0.750	7.810	0.029	2.192	222.70	
0.917	8.259	0.043	3.918	181.54	
1.083	8.708	0.057	6.043	156.69	
1.250	9.157	0.072	8.550	139.78	
1.417	9.607	0.087	11.428	127.38	
1.583	10.056	0.104	14.676	117.80	
1.750	10.505	0.121	18.291	110.13	
2.083	11.403	0.157	26.634	98.48	
2.417	12.302	0.197	36.486	89.94	
2.750	13.200	0.239	47.889	83.31	
3.417	27.750	0.376	83.786	74.77	
4.083	42.301	0.609	139.606	72.75	
4.750	56.851	0.940	223.098	70.22	
5.417	71.401	1.367	340.691	66.89	
6.083	85.951	1.892	498.148	63.30	
6.750	100.502	2.513	700.785	59.78	
7.417	115.052	3.232	953.595	56.49	

END FTABLE 40

FTABLE 41  
ROWS COLS \*\*\*  
20 4

DEPTH ( m)	AREA ( HA)	VOLUME (Mm3)	DISCH (CMS)	FLO-THRU (MIN)	***
0.000	0.000	0.000	0.000	0.00	
0.250	2.562	0.003	0.101	529.37	
0.500	5.124	0.013	0.640	333.40	
0.750	7.686	0.029	1.888	254.40	
0.987	8.799	0.048	4.075	197.66	
1.223	9.912	0.070	7.046	166.69	
1.460	11.026	0.095	10.831	146.57	
1.697	12.139	0.123	15.468	132.16	
1.933	13.252	0.153	21.002	121.19	
2.170	14.365	0.185	27.476	112.46	
2.643	16.592	0.259	43.430	99.26	
3.117	18.819	0.342	63.694	89.61	
3.590	21.045	0.437	88.624	82.14	
4.537	32.560	0.691	171.372	67.16	
5.483	44.075	1.053	296.504	59.20	
6.430	55.589	1.525	477.311	53.25	
7.377	67.104	2.106	724.932	48.41	
8.323	78.619	2.796	1049.413	44.40	
9.270	90.134	3.594	1460.080	41.03	
10.217	101.649	4.502	1965.723	38.17	

END FTABLE 41

```

FTABLE      42
ROWS COLS ***
20      4
DEPTH      AREA      VOLUME      DISCH      FLO-THRU ***
( m)      ( HA)      (Mm3)      (CMS)      (MIN) ***
0.000      0.000      0.000      0.000      0.00
0.250      20.133      0.025      0.364      1153.33
0.500      40.267      0.101      2.310      726.38
0.750      60.400      0.227      6.811      554.26
1.042      63.923      0.408      17.459      389.29
1.333      67.447      0.599      31.979      312.38
1.625      70.970      0.801      50.104      266.53
1.917      74.493      1.013      71.702      235.55
2.208      78.017      1.236      96.712      212.97
2.500      81.540      1.468      125.110      195.62
3.083      88.587      1.965      192.098      170.46
3.667      95.633      2.502      272.861      152.82
4.250      102.680      3.080      367.741      139.61
5.417      135.888      4.472      646.784      115.24
6.583      169.095      6.251      1028.910      101.26
7.750      202.303      8.418      1538.860      91.17
8.917      235.511      10.972      2197.427      83.21
10.083      268.718      13.913      3023.461      76.69
11.250      301.926      17.242      4034.537      71.23
12.417      335.134      20.958      5247.278      66.57
END FTABLE 42

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END FTABLES

\*\*\* For an understanding of EXT SOURCES, EXT TARGETS, SCHEMATIC, MASS-LINK  
 \*\*\* and NETWORK blocks, the user should be familiar with the Time Series  
 \*\*\* Linkages and the Time Series Catalog

EXT SOURCES

```

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # ***
WDM      5 PRCP  3 METR      0.02      PERLND 211 218 EXTNL PREC
WDM      7 PRCP  3 METR      0.14      PERLND 211 218 EXTNL PREC
WDM      8 PRCP  3 METR      0.69      PERLND 211 218 EXTNL PREC
WDM      9 PRCP  3 METR      0.15      PERLND 211 218 EXTNL PREC
WDM      5 PRCP  3 METR      0.02      IMPLND 214 217 EXTNL PREC
WDM      7 PRCP  3 METR      0.14      IMPLND 214 217 EXTNL PREC
WDM      8 PRCP  3 METR      0.69      IMPLND 214 217 EXTNL PREC
WDM      9 PRCP  3 METR      0.15      IMPLND 214 217 EXTNL PREC

WDM      3 PRCP  3 METR      0.17      PERLND 221 228 EXTNL PREC
WDM      4 PRCP  3 METR      0.08      PERLND 221 228 EXTNL PREC
WDM      5 PRCP  3 METR      0.43      PERLND 221 228 EXTNL PREC
WDM      7 PRCP  3 METR      0.32      PERLND 221 228 EXTNL PREC
WDM      3 PRCP  3 METR      0.17      IMPLND 224 227 EXTNL PREC
WDM      4 PRCP  3 METR      0.08      IMPLND 224 227 EXTNL PREC
WDM      5 PRCP  3 METR      0.43      IMPLND 224 227 EXTNL PREC
WDM      7 PRCP  3 METR      0.32      IMPLND 224 227 EXTNL PREC

WDM      3 PRCP  3 METR      0.18      PERLND 231 238 EXTNL PREC
WDM      7 PRCP  3 METR      0.82      PERLND 231 238 EXTNL PREC
WDM      3 PRCP  3 METR      0.18      IMPLND 234 237 EXTNL PREC
WDM      7 PRCP  3 METR      0.82      IMPLND 234 237 EXTNL PREC

WDM      5 PRCP  3 METR      0.37      PERLND 241 248 EXTNL PREC
WDM      7 PRCP  3 METR      0.60      PERLND 241 248 EXTNL PREC
WDM      9 PRCP  3 METR      0.03      PERLND 241 248 EXTNL PREC
WDM      5 PRCP  3 METR      0.37      IMPLND 244 247 EXTNL PREC
WDM      7 PRCP  3 METR      0.60      IMPLND 244 247 EXTNL PREC
WDM      9 PRCP  3 METR      0.03      IMPLND 244 247 EXTNL PREC

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WDM	7	PRCP	3	METR	0.08	PERLND	251	258	EXTNL	PREC
WDM	9	PRCP	3	METR	0.92	PERLND	251	258	EXTNL	PREC
WDM	7	PRCP	3	METR	0.08	IMPLND	254	257	EXTNL	PREC
WDM	9	PRCP	3	METR	0.92	IMPLND	254	257	EXTNL	PREC
WDM	8	PRCP	3	METR	0.70	PERLND	261	268	EXTNL	PREC
WDM	9	PRCP	3	METR	0.30	PERLND	261	268	EXTNL	PREC
WDM	8	PRCP	3	METR	0.70	IMPLND	264	267	EXTNL	PREC
WDM	9	PRCP	3	METR	0.30	IMPLND	264	267	EXTNL	PREC
WDM	9	PRCP	3	METR		PERLND	271	278	EXTNL	PREC
WDM	9	PRCP	3	METR		IMPLND	274	277	EXTNL	PREC
WDM	9	PRCP	3	METR		PERLND	281	288	EXTNL	PREC
WDM	9	PRCP	3	METR		IMPLND	284	287	EXTNL	PREC
WDM	7	PRCP	3	METR	0.32	PERLND	291	298	EXTNL	PREC
WDM	9	PRCP	3	METR	0.68	PERLND	291	298	EXTNL	PREC
WDM	7	PRCP	3	METR	0.32	IMPLND	294	297	EXTNL	PREC
WDM	9	PRCP	3	METR	0.68	IMPLND	294	297	EXTNL	PREC
WDM	9	PRCP	3	METR		PERLND	301	308	EXTNL	PREC
WDM	9	PRCP	3	METR		IMPLND	304	307	EXTNL	PREC
WDM	8	PRCP	3	METR		PERLND	311	318	EXTNL	PREC
WDM	8	PRCP	3	METR		IMPLND	314	317	EXTNL	PREC
WDM	6	PRCP	3	METR	0.09	PERLND	321	328	EXTNL	PREC
WDM	8	PRCP	3	METR	0.76	PERLND	321	328	EXTNL	PREC
WDM	10	PRCP	3	METR	0.15	PERLND	321	328	EXTNL	PREC
WDM	6	PRCP	3	METR	0.09	IMPLND	324	327	EXTNL	PREC
WDM	8	PRCP	3	METR	0.76	IMPLND	324	327	EXTNL	PREC
WDM	10	PRCP	3	METR	0.15	IMPLND	324	327	EXTNL	PREC
WDM	8	PRCP	3	METR	0.73	PERLND	331	338	EXTNL	PREC
WDM	9	PRCP	3	METR	0.05	PERLND	331	338	EXTNL	PREC
WDM	10	PRCP	3	METR	0.22	PERLND	331	338	EXTNL	PREC
WDM	8	PRCP	3	METR	0.73	IMPLND	334	337	EXTNL	PREC
WDM	9	PRCP	3	METR	0.05	IMPLND	334	337	EXTNL	PREC
WDM	10	PRCP	3	METR	0.22	IMPLND	334	337	EXTNL	PREC
WDM	8	PRCP	3	METR	0.15	PERLND	341	348	EXTNL	PREC
WDM	9	PRCP	3	METR	0.73	PERLND	341	348	EXTNL	PREC
WDM	10	PRCP	3	METR	0.12	PERLND	341	348	EXTNL	PREC
WDM	8	PRCP	3	METR	0.15	IMPLND	344	347	EXTNL	PREC
WDM	9	PRCP	3	METR	0.73	IMPLND	344	347	EXTNL	PREC
WDM	10	PRCP	3	METR	0.12	IMPLND	344	347	EXTNL	PREC
WDM	9	PRCP	3	METR	0.56	PERLND	351	358	EXTNL	PREC
WDM	10	PRCP	3	METR	0.24	PERLND	351	358	EXTNL	PREC
WDM	11	PRCP	3	METR	0.20	PERLND	351	358	EXTNL	PREC
WDM	9	PRCP	3	METR	0.56	IMPLND	354	357	EXTNL	PREC
WDM	10	PRCP	3	METR	0.24	IMPLND	354	357	EXTNL	PREC
WDM	11	PRCP	3	METR	0.20	IMPLND	354	357	EXTNL	PREC
WDM	11	PRCP	3	METR		PERLND	361	368	EXTNL	PREC
WDM	11	PRCP	3	METR		IMPLND	364	367	EXTNL	PREC
WDM	10	PRCP	3	METR	0.18	PERLND	371	378	EXTNL	PREC
WDM	11	PRCP	3	METR	0.82	PERLND	371	378	EXTNL	PREC
WDM	10	PRCP	3	METR	0.18	IMPLND	374	377	EXTNL	PREC
WDM	11	PRCP	3	METR	0.82	IMPLND	374	377	EXTNL	PREC
WDM	10	PRCP	3	METR		PERLND	381	388	EXTNL	PREC
WDM	10	PRCP	3	METR		IMPLND	384	387	EXTNL	PREC
WDM	10	PRCP	3	METR		PERLND	391	398	EXTNL	PREC
WDM	10	PRCP	3	METR		IMPLND	394	397	EXTNL	PREC



WDM	10	PRCP	3	METR	0.78	PERLND	401	408	EXTNL	PREC
WDM	11	PRCP	3	METR	0.02	PERLND	401	408	EXTNL	PREC
WDM	12	PRCP	3	METR	0.20	PERLND	401	408	EXTNL	PREC
WDM	10	PRCP	3	METR	0.78	IMPLND	404	407	EXTNL	PREC
WDM	11	PRCP	3	METR	0.02	IMPLND	404	407	EXTNL	PREC
WDM	12	PRCP	3	METR	0.20	IMPLND	404	407	EXTNL	PREC
WDM	11	PRCP	3	METR	0.85	PERLND	411	418	EXTNL	PREC
WDM	12	PRCP	3	METR	0.15	PERLND	411	418	EXTNL	PREC
WDM	11	PRCP	3	METR	0.85	IMPLND	414	417	EXTNL	PREC
WDM	12	PRCP	3	METR	0.15	IMPLND	414	417	EXTNL	PREC
WDM	11	PRCP	3	METR	0.57	PERLND	421	428	EXTNL	PREC
WDM	12	PRCP	3	METR	0.43	PERLND	421	428	EXTNL	PREC
WDM	11	PRCP	3	METR	0.57	IMPLND	424	427	EXTNL	PREC
WDM	12	PRCP	3	METR	0.43	IMPLND	424	427	EXTNL	PREC

Evap region 1 \*\*\*

Covers catchments 1-9 \*\*\*

WDM	13	EVAP	3	METR	0.85	PERLND	11	98	EXTNL	PETINP
WDM	13	EVAP	3	METR	0.85	IMPLND	14	98	EXTNL	PETINP

Evap region 2 \*\*\*

Covers catchments 10-14,17-30,34-37,41-42 \*\*\*

WDM	14	EVAP	3	METR	0.85	PERLND	101	148	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	104	148	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	PERLND	171	308	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	174	308	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	PERLND	341	378	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	344	378	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	PERLND	411	428	EXTNL	PETINP
WDM	14	EVAP	3	METR	0.85	IMPLND	414	428	EXTNL	PETINP

Evap region 3 \*\*\*

Covers catchments 15-16,31-33,38-40 \*\*\*

WDM	15	EVAP	3	METR	0.85	PERLND	151	168	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	154	168	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	PERLND	311	338	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	314	338	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	PERLND	381	408	EXTNL	PETINP
WDM	15	EVAP	3	METR	0.85	IMPLND	384	408	EXTNL	PETINP

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> \*\*\*  
 <Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # \*\*\*

Darvill sewage treatment plant inflow \*\*\*  
 Values are in megalitres per day, so divide by 1 000 \*\*\*  
 The RCHRES module expects IVOL to be in Mm3 per interval of the data \*\*\*  

WDM	600	FLOW	3	METR	1E-3	RCHRES	30	EXTNL	IVOL
-----	-----	------	---	------	------	--------	----	-------	------

Upstream simulations feed into downstream simulation \*\*\*  

WDM	820	HYDR	3	METR		RCHRES	21	EXTNL	IVOL
-----	-----	------	---	------	--	--------	----	-------	------

END EXT SOURCES

SCHEMATIC

<-Source->	<--Area-->	<-Target->	<ML->	***
<Name> #	<-factor->	<Name> #	#	***
	(ha)			***
PERLND 211	132.8	RCHRES 21	1	
PERLND 212	254.5	RCHRES 21	1	
PERLND 213	317.9	RCHRES 21	1	
PERLND 214	13.3	RCHRES 21	1	
IMPLND 214	81.4	RCHRES 21	3	
PERLND 215	463.0	RCHRES 21	1	

IMPLND 215		51.4	RCHRES 21	3
PERLND 216	***		RCHRES 21	1
PERLND 217		4.2	RCHRES 21	1
IMPLND 217		1.5	RCHRES 21	3
PERLND 218	***		RCHRES 21	1
PERLND 221		1037.8	RCHRES 22	1
PERLND 222		396.3	RCHRES 22	1
PERLND 223		240.9	RCHRES 22	1
PERLND 224	***		RCHRES 22	1
PERLND 225		168.6	RCHRES 22	1
IMPLND 225		18.7	RCHRES 22	3
PERLND 226	***		RCHRES 22	1
PERLND 227		38.5	RCHRES 22	1
IMPLND 227		13.6	RCHRES 22	3
PERLND 228	***		RCHRES 22	1
PERLND 231		1747.4	RCHRES 23	1
PERLND 232		421.7	RCHRES 23	1
PERLND 233		229.6	RCHRES 23	1
PERLND 234		0.2	RCHRES 23	1
IMPLND 234		1.1	RCHRES 23	3
PERLND 235		495.4	RCHRES 23	1
IMPLND 235		55.0	RCHRES 23	3
PERLND 236	***		RCHRES 23	1
PERLND 237		77.3	RCHRES 23	1
IMPLND 237		27.2	RCHRES 23	3
PERLND 238	***		RCHRES 23	1
PERLND 241		103.6	RCHRES 24	1
PERLND 242		78.2	RCHRES 24	1
PERLND 243		304.2	RCHRES 24	1
PERLND 244		74.2	RCHRES 24	1
IMPLND 244		455.5	RCHRES 24	3
PERLND 245		973.1	RCHRES 24	1
IMPLND 245		108.1	RCHRES 24	3
PERLND 246	***		RCHRES 24	1
PERLND 247		244.5	RCHRES 24	1
IMPLND 247		85.9	RCHRES 24	3
PERLND 248	***		RCHRES 24	1
PERLND 251		6.1	RCHRES 25	1
PERLND 252		41.6	RCHRES 25	1
PERLND 253		160.9	RCHRES 25	1
PERLND 254		16.4	RCHRES 25	1
IMPLND 254		101.1	RCHRES 25	3
PERLND 255		115.2	RCHRES 25	1
IMPLND 255		12.8	RCHRES 25	3
PERLND 256	***		RCHRES 25	1
PERLND 257		101.3	RCHRES 25	1
IMPLND 257		35.6	RCHRES 25	3
PERLND 258	***		RCHRES 25	1
PERLND 261		22.6	RCHRES 26	1
PERLND 262		169.7	RCHRES 26	1
PERLND 263		399.4	RCHRES 26	1
PERLND 264		28.1	RCHRES 26	1
IMPLND 264		172.8	RCHRES 26	3
PERLND 265		607.2	RCHRES 26	1
IMPLND 265		67.5	RCHRES 26	3
PERLND 266	***		RCHRES 26	1
PERLND 267	***		RCHRES 26	1
PERLND 268	***		RCHRES 26	1
PERLND 271		163.6	RCHRES 27	1
PERLND 272		70.6	RCHRES 27	1
PERLND 273		146.9	RCHRES 27	1
PERLND 274		5.3	RCHRES 27	1
IMPLND 274		32.8	RCHRES 27	3
PERLND 275	***		RCHRES 27	1
PERLND 276	***		RCHRES 27	1
PERLND 277		12.0	RCHRES 27	1
IMPLND 277		4.2	RCHRES 27	3
PERLND 278	***		RCHRES 27	1

PERLND 281		54.9	RCHRES 28	1
PERLND 282		8.4	RCHRES 28	1
PERLND 283		45.4	RCHRES 28	1
PERLND 284		3.8	RCHRES 28	1
IMPLND 284		23.6	RCHRES 28	3
PERLND 285	***		RCHRES 28	1
PERLND 286	***		RCHRES 28	1
PERLND 287	***		RCHRES 28	1
PERLND 288	***		RCHRES 28	1
PERLND 291		443.8	RCHRES 29	1
PERLND 292		1010.7	RCHRES 29	1
PERLND 293		263.3	RCHRES 29	1
PERLND 294		37.4	RCHRES 29	1
IMPLND 294		229.8	RCHRES 29	3
PERLND 295	***		RCHRES 29	1
PERLND 296	***		RCHRES 29	1
PERLND 297		617.0	RCHRES 29	1
IMPLND 297		216.8	RCHRES 29	3
PERLND 298	***		RCHRES 29	1
PERLND 301		71.2	RCHRES 30	1
PERLND 302		415.3	RCHRES 30	1
PERLND 303		545.1	RCHRES 30	1
PERLND 304		0.2	RCHRES 30	1
PERLND 305	***		RCHRES 30	1
PERLND 306	***		RCHRES 30	1
PERLND 307		56.8	RCHRES 30	1
IMPLND 307		20.0	RCHRES 30	3
PERLND 308	***		RCHRES 30	1
PERLND 311		324.5	RCHRES 31	1
PERLND 312		401.4	RCHRES 31	1
PERLND 313		746.4	RCHRES 31	1
PERLND 314		2.6	RCHRES 31	1
IMPLND 314		16.2	RCHRES 31	3
PERLND 315	***		RCHRES 31	1
PERLND 316	***		RCHRES 31	1
PERLND 317	***		RCHRES 31	1
PERLND 318	***		RCHRES 31	1
PERLND 321		745.8	RCHRES 32	1
PERLND 322		4175.8	RCHRES 32	1
PERLND 323		646.1	RCHRES 32	1
PERLND 324	***		RCHRES 32	1
PERLND 325	***		RCHRES 32	1
PERLND 326	***		RCHRES 32	1
PERLND 327	***		RCHRES 32	1
PERLND 328	***		RCHRES 32	1
PERLND 331		80.1	RCHRES 33	1
PERLND 332		876.0	RCHRES 33	1
PERLND 333		358.4	RCHRES 33	1
PERLND 334		1.8	RCHRES 33	1
IMPLND 334		10.8	RCHRES 33	3
PERLND 335	***		RCHRES 33	1
PERLND 336	***		RCHRES 33	1
PERLND 337	***		RCHRES 33	1
PERLND 338	***		RCHRES 33	1
PERLND 341		1340.9	RCHRES 34	1
PERLND 342		3366.2	RCHRES 34	1
PERLND 343		1697.9	RCHRES 34	1
PERLND 344		9.4	RCHRES 34	1
IMPLND 344		58.1	RCHRES 34	3
PERLND 345	***		RCHRES 34	1
PERLND 346	***		RCHRES 34	1
PERLND 347		20.4	RCHRES 34	1
IMPLND 347		7.2	RCHRES 34	3
PERLND 348	***		RCHRES 34	1
PERLND 351		1907.6	RCHRES 35	1
PERLND 352		1807.1	RCHRES 35	1
PERLND 353		53.1	RCHRES 35	1
PERLND 354		0.8	RCHRES 35	1



IMPLND 354		4.7	RCHRES 35	3
PERLND 355	***		RCHRES 35	1
PERLND 356		277.2	RCHRES 35	1
PERLND 357	***		RCHRES 35	1
PERLND 358	***		RCHRES 35	1
PERLND 361		341.4	RCHRES 36	1
PERLND 362		101.7	RCHRES 36	1
PERLND 363		9.4	RCHRES 36	1
PERLND 364	***		RCHRES 36	1
IMPLND 364	***		RCHRES 36	1
PERLND 365	***	674.7	RCHRES 36	1
PERLND 366		214.4	RCHRES 36	1
PERLND 367	***		RCHRES 36	1
PERLND 368	***		RCHRES 36	1
PERLND 371		1470.1	RCHRES 37	1
PERLND 372		1616.3	RCHRES 37	1
PERLND 373		593.8	RCHRES 37	1
PERLND 374	***		RCHRES 37	1
PERLND 375		206.5	RCHRES 37	1
IMPLND 375		23.0	RCHRES 37	3
PERLND 376		733.9	RCHRES 37	1
PERLND 377	***		RCHRES 37	1
PERLND 378	***		RCHRES 37	1
PERLND 381		2058.6	RCHRES 38	1
PERLND 382		148.8	RCHRES 38	1
PERLND 383		16.4	RCHRES 38	1
PERLND 384	***		RCHRES 38	1
PERLND 385		2.2	RCHRES 38	1
IMPLND 385		0.3	RCHRES 38	3
PERLND 386	***		RCHRES 38	1
PERLND 387	***		RCHRES 38	1
PERLND 388	***		RCHRES 38	1
PERLND 391		2334.3	RCHRES 39	1
PERLND 392		624.3	RCHRES 39	1
PERLND 393		172.2	RCHRES 39	1
PERLND 394		4.3	RCHRES 39	1
IMPLND 394		26.8	RCHRES 39	3
PERLND 395		135.0	RCHRES 39	1
IMPLND 395		15.0	RCHRES 39	3
PERLND 396	***		RCHRES 39	1
PERLND 397	***		RCHRES 39	1
PERLND 398	***		RCHRES 39	1
PERLND 401		929.8	RCHRES 40	1
PERLND 402		1345.2	RCHRES 40	1
PERLND 403		796.1	RCHRES 40	1
PERLND 404		0.5	RCHRES 40	1
IMPLND 404		3.0	RCHRES 40	3
PERLND 405		84.3	RCHRES 40	1
IMPLND 405		9.4	RCHRES 40	3
PERLND 406		464.6	RCHRES 40	1
PERLND 407	***		RCHRES 40	1
PERLND 408	***		RCHRES 40	1
PERLND 411		274.2	RCHRES 41	1
PERLND 412		466.3	RCHRES 41	1
PERLND 413	***		RCHRES 41	1
PERLND 414	***		RCHRES 41	1
PERLND 415		1.4	RCHRES 41	1
IMPLND 415		0.2	RCHRES 41	3
PERLND 416		112.1	RCHRES 41	1
PERLND 417	***		RCHRES 41	1
PERLND 418	***		RCHRES 41	1
PERLND 421		774.7	RCHRES 42	1
PERLND 422		698.5	RCHRES 42	1
PERLND 423		65.5	RCHRES 42	1
PERLND 424		1.1	RCHRES 42	1
IMPLND 424		6.9	RCHRES 42	3
PERLND 425		112.9	RCHRES 42	1
IMPLND 425		12.5	RCHRES 42	3

```

PERLND 426          144.6      RCHRES 42      1
PERLND 427          ***      RCHRES 42      1
PERLND 428          ***      RCHRES 42      1
RCHRES 21          RCHRES 25      2
RCHRES 22          RCHRES 24      2
RCHRES 23          RCHRES 24      2
RCHRES 24          RCHRES 25      2
RCHRES 25          RCHRES 27      2
RCHRES 26          RCHRES 27      2
RCHRES 27          RCHRES 28      2
RCHRES 28          RCHRES 30      2
RCHRES 29          RCHRES 30      2
RCHRES 30          RCHRES 34      2
RCHRES 31          RCHRES 34      2
RCHRES 32          RCHRES 33      2
RCHRES 33          RCHRES 34      2
RCHRES 34          RCHRES 35      2
RCHRES 35          RCHRES 36      2
RCHRES 36          RCHRES 37      2
RCHRES 38          RCHRES 40      2
RCHRES 39          RCHRES 40      2
RCHRES 37          RCHRES 41      2
RCHRES 40          RCHRES 41      2
RCHRES 41          RCHRES 42      2
END SCHEMATIC

```

#### MASS-LINK

```

MASS-LINK          1
<Src>      <-Grp> <-Member-><--Mult-->      <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor->      <Name>      <Name> <Name> # # ***
Factor converts (mm.ha) to (million m3)      ***
PERLND      PWATER PERO      0.00001      RCHRES      INFLOW IVOL
END MASS-LINK      1

```

```

MASS-LINK          2
<Src>      <-Grp> <-Member-><--Mult-->      <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor->      <Name>      <Name> <Name> # # ***
RCHRES      ROFLOW      RCHRES      INFLOW
END MASS-LINK      2

```

```

MASS-LINK          3
<Src>      <-Grp> <-Member-><--Mult-->      <Targ>      <-Grp> <-Member-> ***
<Name>      <Name> <Name> # #<-factor->      <Name>      <Name> <Name> # # ***
Factor converts (mm.ha) to (million m3)      ***
IMPLND      IWATER SURO      0.00001      RCHRES      INFLOW IVOL
END MASS-LINK      3

```

#### END MASS-LINK

#### EXT TARGETS

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Aggr Amd ***
<Name>      #      <Name> # #<-factor->strg <Name>      # <Name>qf tem strg strg***

```

\*\*\* For selected reaches, we output the following simulated time series:  
 \*\*\* Total outflow rate (m3/s)

```

RCHRES 30 HYDR      RO      WDM      830 HYDR      1 METR AGGR REPL
RCHRES 41 HYDR      RO      WDM      841 HYDR      1 METR AGGR REPL
END EXT TARGETS

```

#### END RUN